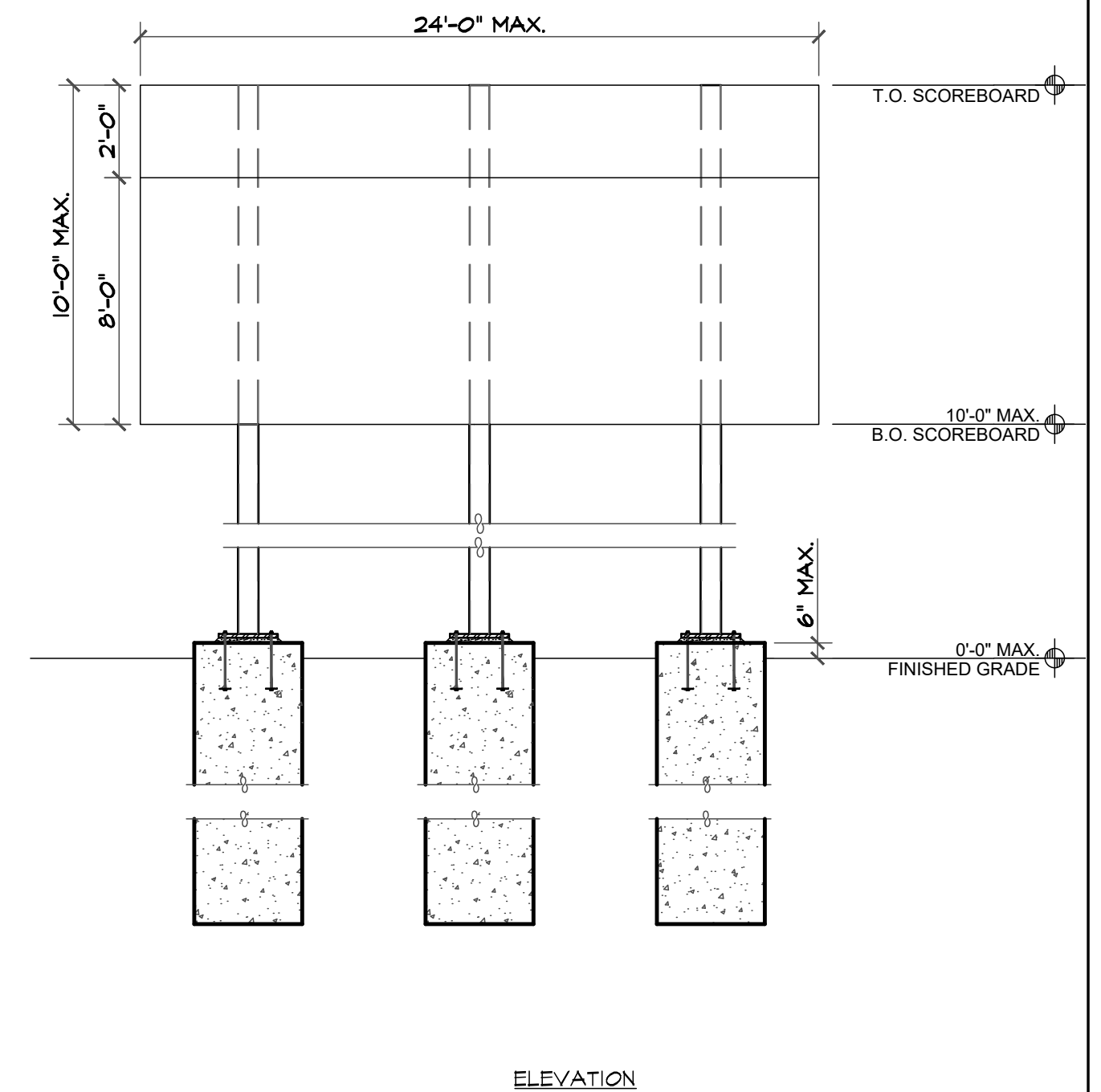


SEE DSA P.C. 04-122317 FOR NEVCO 1606-PC SCOREBOARD

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10 DETAIL

SCALE: N.T.



APPROVALS:
APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

DATE: 12/05/2023

PARTIAL SITE PLAN
MADERA HIGH SCHOOL
BASEBALL SCOREBOARD
MADERA, CA 93637

REVISIONS	
1	
2	
3	
4	
5	

BrooksRansom
ASSOCIATES
7415 N. PALM AVE. STE. 100 FRESNO, CA 93711
(559) 449-8444 OFFICE | (559) 449-8404 FAX


SHEET:

SP-1

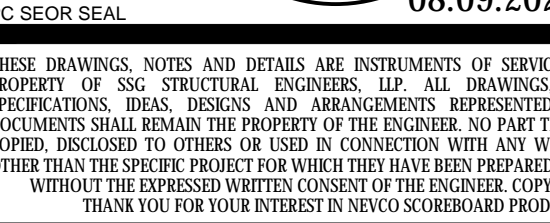
PROJECT 23311



DSA P.C. 04-122317



SSG
structural engineers



SHEET INFORMATION	
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SG JOB #	S23109
SHEET	CD 1

TABLE C - SITE SPECIFIC SEISMIC AND WIND VALUES				TABLE B - STRUCTURAL DESIGN VALUES				
EARTHQUAKE DESIGN DATA		MAXIMUM	SITE SPECIFIC	All values reported are unfactored and strength level, unless noted otherwise				
		$S_m = 3.73 \text{ g}$	\geq	S ₁ = 0.71 g		Gravity Design Data		Value
Mapped Spectral Response Accelerations (Maximum)		$S_m = 1.0 \text{ g}$	\geq	S ₁ = 0.24 g		Dead Loads:		PER SCHEDULE
Site Class		D	\geq	D		Sign Dead Load		
		$S_m = 2.49 \text{ g}$	\geq	S ₁ = 0.67 g		Snow Loads:		
Spectral Response Coefficients (Maximum)		$S_m = 1.0 \text{ g}$	\geq	S ₁ = 0.42 g		Ground Snow Load, P _g (Maximum)		30 psf
Wind Design Data		Value		Value		Deflection Criteria:		H/240
Design Wind Speed (3-sec gust), V _{ULT}		100 mph	\geq	94 mph		Sign, Wind Load		Value
Exposure Category		C		C		Design Wind Speed (3-sec gust), V _{ULT}		100 mph
TABLE D - SITE FLOOD ZONE						Design Wind Speed (3-sec gust), V _{ASD}		77 mph
THIS SECTION NOT REQUIRED IF SITE IS IN FLOOD ZONE X						Risk Category		II
Geotechnical Engineer:						Exposure Category		C
Letter Dated:						Applicable Internal Pressure Coefficient		± 0.18
						Design Wind Pressure(s) for Components & Cladding (Not specifically designed by the Registered Design Professional, and to be modified by applicable factors per ASCE 7)		q = 21.8xk, psf k VARIES
						Earthquake Design Data		Value
						Risk Category		II
						Importance Factor, I _p		1.0
						Mapped Spectral Response Accelerations (Maximum)		S _m = 3.73 g S _m = 1.0 g
						Site Class		A through E
						Spectral Response Coefficients (Maximum)		S _m = 2.49 g S _m = 1.0 g
						Seismic Design Category		E
						Analysis Procedure Used		Equivalent Lateral Force Procedure (ASCE 7, 12.8)
						Basic Seismic Force Resisting System		Non-Building Structure, ASCE 7-16 Chapter 15
						Response Modification Factor, Signs and Billboards Table 15.4.2		R = 3.0
						Seismic Response Coefficient		C = 0.83
						Design Base Shear		V = C _s w ₀
						Flood Design		
						When the scoreboard is located in a flood zone other than Zone X, a letter stamped and signed from a Geotechnical Engineer is needed to validate allowable soil values specified in the PC are still applicable.		
						Geotechnical Design Data		Value
						Geotechnical Design Based on: 2022 California Building Code, Chapter 18A, Table 1806.A.2 (Class 5 Material)		
						Allowable Soil Bearing Pressure (DL + LL)		1,500 psf
						Design Passive Pressure, P _p (Tabular value has been increased per CBC Section 1806A.3.4 for pier design)		100 pcF
						Design Skin Friction, f _s		100 psf

TABLE A - SCOREBOARD ASSEMBLY WORKSHEET ⁽¹⁾			
Item	Part	Qty	Notes
1	Scoreboard	1	
2	Scoreboard Assembly	1	
3	Scoreboard Assembly	1	
4	Scoreboard Assembly	1	
5	Scoreboard Assembly	1	
6	Scoreboard Assembly	1	
7	Scoreboard Assembly	1	
8	Scoreboard Assembly	1	
9	Scoreboard Assembly	1	
10	Scoreboard Assembly	1	
11	Scoreboard Assembly	1	
12	Scoreboard Assembly	1	
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96	Scoreboard Assembly	1	
97	Scoreboard Assembly	1	
98	Scoreboard Assembly	1	
99	Scoreboard Assembly	1	
100	Scoreboard Assembly	1	

1. Verify part number, dimensions, and weight with Nevco
2. See Step 3 of Scoreboard Assembly Worksheet Instructions

GENERAL NOTES AND MATERIAL SPECIFICATIONS

1. THE ARCHITECT OR PROFESSIONAL ENGINEER IN GENERAL RESPONSIBLE CHARGE SHALL SIGN AND SEAL ALL DRAWINGS AND SPECIFICATIONS PER TITLE 24, PART 1, SECTIONS 4-316(E) AND 4-317 (H).
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA, OR CONSTRUCTION CHANGE DOCUMENTS APPROVED BY THE DIVISION OF THE STATE ARCHITECT (DSA), AS REQUIRED BY TITLE 24, PART 1, SECTION 4-338.
3. THE DISTRICT SHALL EMPLOY A CLASS 2 PROJECT INSPECTOR WHEN OVERALL STRUCTURE HEIGHT IS 35 FEET OR GREATER. OTHERWISE A CLASS 3 PROJECT INSPECTOR MAY BE USED. THE PROJECT INSPECTOR SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK , AND SHALL SUBMIT VERIFIED REPORTS ON A DSA-6 FORM. THE DUTIES OF THE PROJECT INSPECTION ARE DEFINED IN TITLE 24, PART 1, SECTION 4-342.
4. ALL SCOREBOARD CONTROLS SHALL BE FULLY ACCESSIBLE VIA WIRELESS CONTROL OR COMPLETE DESIGN SHALL BE DEMONSTRATED IN THE SITE-SPECIFIC APPLICATION.
5. ALL ASSEMBLIES SHALL HAVE ELECTRICAL DISCONNECT PER CEC 600.6 AND BE ELECTRICALLY GROUNDED PER CEC 600.7. SEE DETAIL B/SB5.1
6. IN FLOOD ZONES, LOCATION OF ELECTRICAL ELEMENTS SHALL CONFORM TO ASCE 24, SECTION 7.2 PER DSA PR-14-01 SECTION 1.2.1.
7. SEE PAGE, SB0.2, FOR ALL MATERIAL SPECIFICATIONS AND NOTES.
8. PROJECT DESIGN PROFESSIONAL OF RECORD IS RESPONSIBLE FOR PREPARATION OF THE PROJECT SPECIFIC DSA 103 AND IS RESPONSIBLE FOR ALL SHOP DRAWING AND SUBMITTAL REVIEWS. SEE SB0.3 FOR EXAMPLE DSA 103

SB0.1

GENERAL NOTES

1. The following notes, typical details and schedules shall apply to all phases of this project unless otherwise shown or noted.
2. Specific notes and details shall take precedence over general notes and typical details.
3. All materials and workmanship shall conform to the minimum standards of the 2022 edition of Title 24 of the California Building Code (CBC) and any other regulating agencies exercising authority over any portion of the work. The Contractor shall have a current copy of the CBC on the job site.
4. The "Contract or Construction Documents" shall consist of these notes, details, schedules, plans, and drawings.
5. All specifications, including but not limited to materials and products, shall be those put forth in the "Contract or Construction Documents". The Contractor shall verify and be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.
6. The Contractor shall examine the "Contract or Construction Documents" and shall notify the Architect or Structural Engineer of Record of any discrepancies he may find before proceeding with the work.
7. All information on existing conditions shown on drawings are based on best present information. The Contractor, but not the Architect or Structural Engineer, shall be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.
8. The Contractor shall immediately notify the Architect or Structural Engineer of Record of any condition which in his opinion might endanger the stability of the structure or cause distress to the structure.
9. All work shall conform to the best practice prevailing in the various trades comprising work. The Contractor shall be responsible for coordinating the work of all trades.
10. These "Contract or Construction Documents" represent the finished structure, and do not indicate the method of construction. The Contractor shall supervise and direct the work and shall be solely responsible for construction means, methods, techniques, sequences and procedures.
11. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 170A4.2.5, 170A3.1, and 170A3.3.
12. Evaluation and follow-up inspection services (as required or specified), shall conform to CBC Section 170A3.1.
13. The Contractor shall provide temporary bracing and shoring for all structural members as required for structural stability of the structure during all phases of construction.
14. The Contractor shall take all steps necessary to ensure proper alignment of any structure during the installation of all structural and finish materials. This shall include any necessary preloading of the structure to determine final position of the completed work.
15. Observation visits to the project site by field representatives of Architect and/or Structural Engineer of Record (support services) shall not include inspections of safety or protective measures, nor construction procedures, techniques or methods. Any support services personnel performing safety or protective measures shall be permitted to observe construction but shall be distinguished from continuous and detailed inspection services (as required by any regulating governmental agency, e.g. the Authority Having Jurisdiction) provided by others. The Contractor shall be responsible for the safety of all personnel employed for the purpose or assisting in quality control and in achieving conformance with contract documents, but do not guarantee Contractor's performance and shall not be construed as supervision or construction.
16. These notes, details, drawings and specifications (Contract or Construction Documents) do not constitute provisions for necessary provisions for construction safety. These documents and all phases of construction hereby contemplated are to be governed, at all times, by applicable provisions of the current California Occupational Safety and Health Act.
17. Where any conflict occurs between the requirements of federal, state and local laws, codes, ordinances, rules and regulations, the most stringent shall govern.
18. Written dimensions shall have precedence over scaled dimensions.
19. Drawings (notes, schedules, details and plans) shall have precedence over Structural Calculations.
20. In the event that certain features of the construction are not fully shown on the drawings or called for in the General Notes or Specifications, then their construction shall be of the same character as for similar conditions that are shown or called for.
21. ASTM designation and all standards refer to the latest amendments.
22. These structural "Contract or Construction Documents" shall not be modified without prior written approval of the Structural Engineer of Record.
23. Only structural working drawings approved by the Division of the State Architect are permitted to be used for construction on this project. All other drawings or documents are obsolete and are not permitted on the job site, nor shall they be used for any construction purposes. Contractors using unapproved drawings or documents are solely responsible for any work not performed in accordance with the "approved" drawings.
24. A Division of the State Architect certified project inspector employed by the District (Owner) and approved by the Division of the State Architect shall provide continuous inspection of the work. The duties of the inspector are defined in Section 4-342, Part 1, Title 24 California Code of Regulations.

FOUNDATION NOTES

1. Basic: See Structural Design Values Chart, Sheet S80.1 Table B
 2. Unexpected soil conditions: Allowable values and foundation design are based upon the minimum values provided in Table 1806A.2 of the 2022 California Building Code. See S80.1 for values
 3. Excavate to required depths and dimensions (as indicated in drawings), cut square and smooth with firm level bottoms. Care shall be taken not to over-excavate foundation a lower elevation and prevent disturbing of soils around higher elevation.
 4. Footings shall be poured in neat excavations, without side forms whenever possible.
 5. Carry all foundations to required depths into compacted fill or natural soil (as per Structural Plans and Details).
 6. All foundation excavations shall be inspected and approved by the Inspector of Record or Geotechnical Engineer prior to forming and placement of reinforcing or concrete.
 7. Foundations shall not be poured until all required reinforcing steel, sleeves, inserts, conduits, pipes, etc., and formwork is properly placed and inspected by the Authority having jurisdiction.
 8. The sides and bottoms of excavations which are to have concrete contact must be moistened several times just prior to pouring upon them.
 9. De-water footings, as required, to maintain dry working conditions.
- REINFORCING STEEL**
1. All reinforcing steel shall be deformed intermediate grade bars conforming to ASTM A615 Grade 60, or 60 ksi unless noted otherwise.
 2. Reinforcing steel shall not be welded, unless specifically noted otherwise.
 3. To hold reinforcing bars in their true position and prevent displacement, standard tie and anchorage devices must be provided. Flacing of reinforcement shall conform to ACI 318-19 Section 26.6.2.
 4. Shop drawings for fabrication of any reinforcing steel shall be approved by Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record, for their review, prior to fabrication.
 5. Refer to typical details for minimum splice length and minimum radius of bend of reinforcing steel.
 6. All reinforcing steel splices shall be staggered 24", unless specifically noted or detailed otherwise.
 7. All reinforcing bars bends shall be made cold.
 8. Fabrication, erection and placement of reinforcing steel shall conform to Concrete Reinforcing Steel Institute of Standard Practice.
 9. Reinforcing steel shall be clean of rust, grease or other material likely to impair bond.

CONCRETE

1. All concrete shall have a minimum ultimate compressive strength (f'_c) as outlined below at 28 days. All concrete shall be regular weight (unless specifically noted otherwise).
 - A. Concrete for footings: 4,500 psi w/c = 0.45 max.
2. Maximum Fly Ash content shall be 15%, by weight, of total cementitious materials and shall conform to ASTM C618.
3. All concrete work shall comply with CBC Chapter 19A and ACI 318-19 and latest edition of ACI Manual of Concrete Practice.
4. Special Inspection (as required or specified) shall conform to CBC Chapter 17A.
5. Cement shall be portland cement Type V and shall conform to ASTM C150.

A.B.
ARV

ADI	American Concrete Institute	HSS	Hollow Steel Section
ADJ	Adjacent	HT	Height
AEU	Division of the State Architect	ICC	International Building Code
AISC	American Institute of Steel Construction	ICC	International Code Council
ASCE	Architectural Record	ID	Inside Diameter
APPROX.	Approximately	IN.	Inch, Inches
ASCE	American Society of Civil Engineers	INT.	Interior
ARCH.	Architect, Architecture	ksi	Kips per Square Inch
ASTM	American Society of Testing and Materials	LL	Live Load
ATR	All Thread Rod	MAX	Maximum
AWB	American Welding Society	MB	Machine Bolt
B.O.	Bottom of _____	MFR.	Manufactured, Manufacturer
b/d	Between _____	MIN	Minimum
		NPH	Miles per Hour
CAC	California Administrative Code	N/R	Not Required
CBC	California Building Code	N.T.S.	Not To Scale
CIP	Cast-in-place	OC	On Center
CIP	Complete Joint Penetration	o/o	Over
CL	Centerline	OD	Outside Diameter
CLR	Clear	PEN.	Penetration
COL.	Column	PI.	Plate
CONC.	Concrete	PJ	Partial Joint Penetration
CONN.	Connection	psf	Pounds per Square Inch
CONST.	Construction	PSF	Pounds per Square Foot
CONT.	Continue, Continuous		
Ø	Diameter	REBAR	Reinforcing Bar
DBL	Double	REINF.	Reinforcement
DET.	Detail	REQ'D	Required
DL	Dead Load		
DSA	Division of State Architect	S.F.	Square Feet
DWG/S	Drawings	SHT.	Sheet
EA.	Each	SHM.	Similar
E.F.	Each Face	SMS	Sheet Metal Screw
ELEC.	Electric, Electrical	SQ.	Square
ELEV.	Elevation	STAG'd	Staggered
EMBED.	Embedded, Embedment	STD	Standard
ENG.	Engineer of Record	STL	Structural Engineer of Record
EQU.	Equal		
EQUIP.	Equipment	T&B	Top and bottom
E.S.	Each Side	TH'D	Thru
E.W.	Each Way	T.O.	Top of _____
EXT.	Exterior	TYP.	Typical
FAB.	Fabricated		
FDN.	Foundation	N.O.	Not Needed Otherwise
F.G.	Finish Grade		
F.O.	Face of	VERT.	Vertical
FRMG.	Framing	VF	Verily In Field
FT.	Foot, Feet	w/	With
GA.	Footing	w/c	Water/Cement Ratio
G/G	Gauge	WSS	Welded Steel Stud
GAZ.	Galvanized	WT.	Weight
GEOR.	Geotechnical Engineer of Record		

POST INSTALLED ANCHOR & TESTING

- All post-installed anchors are to be tension tested with the exception that torque testing is allowed if the anchors are specifically designed as torque controlled
- Test quantity of post-installed anchors as noted below:
- | Application | Quantity |
|--|----------|
| Non-structural (Equipment Anchorage, etc.) | 50% |
| Structural | 100% |
- Apply proof test loads to anchors without removing the nut if possible. If not, remove nut and install a threaded coupler to the same tightness of the original nut using a torque wrench and apply load.
- All tests shall be performed in the presence of the inspector.
- Reaction loads from test fixtures may be applied close to the anchor being tested, provided the anchor is not restrained from withdrawing or restricted from a concrete clear one type failure mechanism.
- Test equipment is to be calibrated by an approved testing laboratory in accordance with standard recognized procedures.
- The following criteria apply for the acceptance of installed anchors:
- A. Hydraulic ram method: anchors tested with a hydraulic jack or spring loaded devices shall maintain the test load for a minimum of 15 seconds and shall exhibit no discernible movement during the tension test, e.g. as evidenced by loosening of the washer under the nut.
 - B. Torque wrench method: anchors tested with a calibrated torque wrench must attain the manufacturer recommended torque within $\frac{1}{2}$ turn of the nut.
Exceptions:
 - Wedge or sleeve type: one-quarter turn of the nut from 3/8" sleeve anchor only.
 - Threaded type: one-quarter turn of the screw after initial seating of the screw head.
- If any anchor fails testing, test all anchors of the same type not previously tested within two consecutive anchors pass, then resume the initial test frequency. If the anchors are used for the support and bracing of non-structural components (pipe, duct or conduit), the twenty shall be only those anchors installed by the same trade.
- Test loads per ACI ESR, LAPMO, or UES report
- When installing drilled-in anchors and/or powder driven pits in existing non-prestressed reinforced concrete, use care and caution to avoid cutting or damaging the existing reinforcement. When installing them into existing prestressed concrete (pre or post-tensioned) locate the prestressed tendons by using a non-destructive method prior to installation. Exercise extreme care and caution to avoid cutting or damaging the tendons during installation. Maintain a minimum clearance of one inch between the reinforcement and the drilled-in anchor or/and pin.

ANCHOR TORQUE TEST VALUES

Anchor Diameter	CONCRETE		MASONRY	
	HILTI KB TZ 2	SIMPSON STRONG BOLT 2	HILTI KB TZ 2	SIMPSON STRONG BOLT 2
	ESR-4266	ESR-3037	ESR-4561	ER-240
3/8"	30 ft-lb	30 ft-lb	15 ft-lb	20 ft-lb
1/2"	50 ft-lb	60 ft-lb	25 ft-lb	35 ft-lb
5/8"	40 ft-lb	90 ft-lb	30 ft-lb	55 ft-lb
3/4"	110 ft-lb	150 ft-lb	50 ft-lb	100 ft-lb

If the manufacturer's recommended installation torque is less than the test torque noted in the table, the manufacturer's recommended installation torque should be used in lieu of the tabulated values.

See manufacturer's ESR report for Maximum Impact Wrench Torque Rating.

~~EXAMPLE DSA 103 - TESTING AND INSPECTIONS~~

OSA 103-22 LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC						
License Number: School Number: OSA File Number:	Nexus County/County PC Increment Number:	School District: Nexus County/County PC Date Created: 2022-02-10 08:55:36				
2022 CBC						
<p>IMPORTANT: This form is only a summary list of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Department of Engineering of Record, Laboratory of Record, or Special Inspector. The actual complete test and inspection program must be determined as detailed on the OSA approved drawings. It depends on the bottom of this form identifies work NOT subject to special inspection.</p> <p>Special Inspection: The Special Inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections and testing of materials, construction methods, and construction quality control, including but not limited to, framing, anchorage, and non-structural components, etc. per Title 24, Part 2, Chapter 7.3A (2022 CBC).</p>						
<p>**NOTE: Underlined section and table references found in this document are from the CBC, or California Building Code.</p>						
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="text-align: left; padding: 5px;">KEY TO COLUMNS</th> </tr> </thead> <tbody> <tr> <td style="padding: 5px;"> 1. TYPE Continuous - Indicates that a continuous special inspection is required Periodic - Indicates that a periodic special inspection is required Test - Indicates that a test is required </td> </tr> <tr> <td style="padding: 5px;"> 2. PERFORMED BY Structural Engineering - Indicates that the special inspection will be performed by a registered geotechnical engineer or by an authorized engineer. LOR (Laboratory of Record) - Indicates that the test or special inspection shall be performed by the Laboratory of Record or the Laboratory Evaluation and Accredited (LEA) firm. See CBC Section 3.35. If Project Inspector - Indicates that the special inspection may be performed by a project inspector when specifically approved by OSE. Special Inspector - Indicates that the special inspection shall be performed by an appropriately qualified/supervised special inspector. </td> </tr> </tbody> </table>				KEY TO COLUMNS	1. TYPE Continuous - Indicates that a continuous special inspection is required Periodic - Indicates that a periodic special inspection is required Test - Indicates that a test is required	2. PERFORMED BY Structural Engineering - Indicates that the special inspection will be performed by a registered geotechnical engineer or by an authorized engineer. LOR (Laboratory of Record) - Indicates that the test or special inspection shall be performed by the Laboratory of Record or the Laboratory Evaluation and Accredited (LEA) firm. See CBC Section 3.35. If Project Inspector - Indicates that the special inspection may be performed by a project inspector when specifically approved by OSE. Special Inspector - Indicates that the special inspection shall be performed by an appropriately qualified/supervised special inspector.
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D5A 10-22-22 LISTS OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC								
Table 1705A-1, Table 1718-B-19 Section 26.1, 26.13			School Districts					
606 1228-0000			School Name		School District			
606 419-0000			Nevco Schoolboard PC		District Code/ID			
			Inspection Number		Code Reference and Notes			
Test or Special Inspection			Type	Performed by	Code Reference and Notes			
a	Verify the insitu concrete strength prior to placing of post-tensioning tendons	Periodic	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10. Special inspection to verify specified concrete strength test prior to installing.			
					Table 1705A-1, Section 10.13			
b	Inspection application of post-tensioning and grouting of post-tensioning ducts and grouting of post-tensioning tendons	Continuous	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13, Table 1718-B-19 Section 26.13			
					Table 1705A-1, Section 10.13, Table 1718-B-19 Section 26.13			
C1 PRECAST CONCRETE (IN ADDITION TO SECTION C1)								
Test or Special Inspection			Type	Performed by	Code Reference and Notes			
a	Inspection installation of precast concrete members	Periodic	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13			
					Table 1705A-1, Section 10.13			
b	Inspection erection of precast concrete members	Periodic	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13			
					Table 1705A-1, Section 10.13			
c	For precast concrete diaphragms connected to reinforcement at joints designed as moment or high deformable elements (RCC or RSC or RSC in situations assigned to joints design Category 2 or 3) inspect such connections and reinforcement in the field and/or	Continuous	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13, Table 1718-B-19 Section 26.13			
					Table 1705A-1, Section 10.13, Table 1718-B-19 Section 26.13			
d	Inspection installation of the embedded parts	Periodic	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13			
					Table 1705A-1, Section 10.13			
e	Inspection installation of the embedded parts	Periodic	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13			
					Table 1705A-1, Section 10.13			
f	Inspection installation of the embedded parts	Periodic	SI	Table 1705A-1, Section 10	Table 1705A-1, Section 10.13			
					Table 1705A-1, Section 10.13			

QSA 103-22- LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC												
1. Project Information Project Name: 1745 602-16, Tables 3 and 4 School Name: Nevvo Scoreboards PC QSA 1227-23 QSA File #: School District: Nevvo Scoreboards PC Date Created: 02/20/2023		2. Test Results <table border="1"> <thead> <tr> <th>Test or Special Inspection</th> <th>Type</th> <th>Performed By</th> <th>Code References and Notes</th> </tr> </thead> <tbody> <tr> <td> <input checked="" type="checkbox"/> a. Test post installed anchors <input type="checkbox"/> b. Test post installed anchors </td> <td>Test</td> <td>LOH</td> <td>17054.4, 1910d.3, See Appendix (end of this form) for exemptions.</td> </tr> </tbody> </table>			Test or Special Inspection	Type	Performed By	Code References and Notes	<input checked="" type="checkbox"/> a. Test post installed anchors <input type="checkbox"/> b. Test post installed anchors	Test	LOH	17054.4, 1910d.3, See Appendix (end of this form) for exemptions.
Test or Special Inspection	Type	Performed By	Code References and Notes									
<input checked="" type="checkbox"/> a. Test post installed anchors <input type="checkbox"/> b. Test post installed anchors	Test	LOH	17054.4, 1910d.3, See Appendix (end of this form) for exemptions.									
3. Other Information <table border="1"> <thead> <tr> <th>Test or Special Inspection</th> <th>Type</th> <th>Performed By</th> <th>Code References and Notes</th> </tr> </thead> <tbody> <tr> <td> <input type="checkbox"/> a. </td> <td></td> <td></td> <td></td> </tr> </tbody> </table>					Test or Special Inspection	Type	Performed By	Code References and Notes	<input type="checkbox"/> a.			
Test or Special Inspection	Type	Performed By	Code References and Notes									
<input type="checkbox"/> a.												

[illegible]

ASA 103-22-LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS(SIGNATURE), 2022 CBC			
Application Number:	School Name:	School District:	
AS-1759	Newco-Soundebays PC	Newco Soundebays PC	
DSA File Number:	Increment Number:	Date Created:	
		2023-05-23 08:00:36	
Name of Architect or Engineering Consultant responsible charge			
Name of Structural Engineer (When structural engineer has been designated)			
Signature of Architect or Structural Engineer			
Note: To facilitate DSA electronic mark-ups and identification stamp applications, DSA recommends scanning, saving secured electronic or digital signatures.			
		<div style="border: 1px solid black; padding: 5px;"> DSA STAMP </div>	

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DASA 103-22-2 LISTS OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC					
Form ID: TSDA-22-18-119 Sections 26.12.9, 26.12.9.1			School District:		
Section Number:			School Name:		
On 1/28/2022			News-Schools/DC Public		
Form ID: TSDA-22-18-119			Document Number:		
Form ID: TSDA-22-18-119			Date: 2022-01-28 09:38:39		
C.3. SATISFACTORY ADDITION TO SECTION C(1):					
<input type="checkbox"/>	Test or Special Inspection:	Type	Performed By	Check References and Notes	
<input type="checkbox"/>	a. Inspect electronic placement and proper application techniques	Continuous	SI	TSDA-22-19, Table 1702A.3 Item 7, 1702A.3.1, 1702A-22-19, 1702A-22-19.1, 1702A-22-19.2, 1702A-22-19.3, 1702A-22-19.4, 1702A-22-19.5, 1702A-22-19.6, 1702A-22-19.7, 1702A-22-19.8, 1702A-22-19.9, 1702A-22-19.10, 1702A-22-19.11, 1702A-22-19.12, 1702A-22-19.13, 1702A-22-19.14, 1702A-22-19.15, 1702A-22-19.16, 1702A-22-19.17, 1702A-22-19.18, 1702A-22-19.19, 1702A-22-19.20, 1702A-22-19.21, 1702A-22-19.22, 1702A-22-19.23, 1702A-22-19.24, 1702A-22-19.25, 1702A-22-19.26, 1702A-22-19.27, 1702A-22-19.28, 1702A-22-19.29, 1702A-22-19.30, 1702A-22-19.31, 1702A-22-19.32, 1702A-22-19.33, 1702A-22-19.34, 1702A-22-19.35, 1702A-22-19.36, 1702A-22-19.37, 1702A-22-19.38, 1702A-22-19.39, 1702A-22-19.40, 1702A-22-19.41, 1702A-22-19.42, 1702A-22-19.43, 1702A-22-19.44, 1702A-22-19.45, 1702A-22-19.46, 1702A-22-19.47, 1702A-22-19.48, 1702A-22-19.49, 1702A-22-19.50, 1702A-22-19.51, 1702A-22-19.52, 1702A-22-19.53, 1702A-22-19.54, 1702A-22-19.55, 1702A-22-19.56, 1702A-22-19.57, 1702A-22-19.58, 1702A-22-19.59, 1702A-22-19.60, 1702A-22-19.61, 1702A-22-19.62, 1702A-22-19.63, 1702A-22-19.64, 1702A-22-19.65, 1702A-22-19.66, 1702A-22-19.67, 1702A-22-19.68, 1702A-22-19.69, 1702A-22-19.70, 1702A-22-19.71, 1702A-22-19.72, 1702A-22-19.73, 1702A-22-19.74, 1702A-22-19.75, 1702A-22-19.76, 1702A-22-19.77, 1702A-22-19.78, 1702A-22-19.79, 1702A-22-19.80, 1702A-22-19.81, 1702A-22-19.82, 1702A-22-19.83, 1702A-22-19.84, 1702A-22-19.85, 1702A-22-19.86, 1702A-22-19.87, 1702A-22-19.88, 1702A-22-19.89, 1702A-22-19.90, 1702A-22-19.91, 1702A-22-19.92, 1702A-22-19.93, 1702A-22-19.94, 1702A-22-19.95, 1702A-22-19.96, 1702A-22-19.97, 1702A-22-19.98, 1702A-22-19.99, 1702A-22-19.100, 1702A-22-19.101, 1702A-22-19.102, 1702A-22-19.103, 1702A-22-19.104, 1702A-22-19.105, 1702A-22-19.106, 1702A-22-19.107, 1702A-22-19.108, 1702A-22-19.109, 1702A-22-19.110, 1702A-22-19.111, 1702A-22-19.112, 1702A-22-19.113, 1702A-22-19.114, 1702A-22-19.115, 1702A-22-19.116, 1702A-22-19.117, 1702A-22-19.118, 1702A-22-19.119, 1702A-22-19.120, 1702A-22-19.121, 1702A-22-19.122, 1702A-22-19.123, 1702A-22-19.124, 1702A-22-19.125, 1702A-22-19.126, 1702A-22-19.127, 1702A-22-19.128, 1702A-22-19.129, 1702A-22-19.130, 1702A-22-19.131, 1702A-22-19.132, 1702A-22-19.133, 1702A-22-19.134, 1702A-22-19.135, 1702A-22-19.136, 1702A-22-19.137, 1702A-22-19.138, 1702A-22-19.139, 1702A-22-19.140, 1702A-22-19.141, 1702A-22-19.142, 1702A-22-19.143, 1702A-22-19.144, 1702A-22-19.145, 1702A-22-19.146, 1702A-22-19.147, 1702A-22-19.148, 1702A-22-19.149, 1702A-22-19.150, 1702A-22-19.151, 1702A-22-19.152, 1702A-22-19.153, 1702A-22-19.154, 1702A-22-19.155, 1702A-22-19.156, 1702A-22-19.157, 1702A-22-19.158, 1702A-22-19.159, 1702A-22-19.160, 1702A-22-19.161, 1702A-22-19.162, 1702A-22-19.163, 1702A-22-19.164, 1702A-22-19.165, 1702A-22-19.166, 1702A-22-19.167, 1702A-22-19.168, 1702A-22-19.169, 1702A-22-19.170, 1702A-22-19.171, 1702A-22-19.172, 1702A-22-19.173, 1702A-22-19.174, 1702A-22-19.175, 1702A-22-19.176, 1702A-22-19.177, 1702A-22-19.178, 1702A-22-19.179, 1702A-22-19.180, 1702A-22-19.181, 1702A-22-19.182, 1702A-22-19.183, 1702A-22-19.184, 1702A-22-19.185, 1702A-22-19.186, 1702A-22-19.187, 1702A-22-19.188, 1702A-22-19.189, 1702A-22-19.190, 1702A-22-19.191, 1702A-22-19.192, 1702A-22-19.193, 1702A-22-19.194, 1702A-22-19.195, 1702A-22-19.196, 1702A-22-19.197, 1702A-22-19.198, 1702A-22-19.199, 1702A-22-19.200, 1702A-22-19.201, 1702A-22-19.202, 1702A-22-19.203, 1702A-22-19.204, 1702A-22-19.205, 1702A-22-19.206, 1702A-22-19.207, 1702A-22-19.208, 1702A-22-19.209, 1702A-22-19.210, 1702A-22-19.211, 1702A-22-19.212, 1702A-22-19.213, 1702A-22-19.214, 1702A-22-19.215, 1702A-22-19.216, 1702A-22-19.217	

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DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (STEEL AND ALUMINUM), 2022 CBC					
PROJECT 1, Table T100A.2-1; ASCE 305.1-A; ASCE 341.1-A; ASCE 358.1-A; ASCE 360.18-A; AISI 150B.20 RESC. 2014; AWS D1.1-A; AWS D1.5-A; AWS D1.8-A					
Project Number	School Name	Nemo Scoreboard PC	School District	Date Completed	Inspector
DSA Project Number	Increment Number	Increment Number	Increment Number	Increment Number	Increment Number
Test or Special Inspection	Type	Performed By	Code Reference(s) and Notes		
<input type="checkbox"/> c. Storage rack and/or crane installation.	Periodic	SI	ANSI/MHI1.1 Section 7.2.2; Table 1705A.13.7		
<input type="checkbox"/> d. Completed storage rack system indicate compliance with the approved construction documents.	Periodic	SP*	Table 1705A.13.7 *May be performed by project inspector when specifically approved by DSA.		
SAT 11. Other Steel					
Test or Special Inspection	Type	Performed By	Code Reference(s) and Notes		
<input type="checkbox"/> a.					

DAS 103-22: LIST OF REQUIRED VERIFIED REPORTS, CBC 2022		
Verification Number: 067817	School Name: Newse-Southwells PC	School District: Newse-Southwells PC
DAS Form Number:	Increment Number:	Date Created: 2023-10-25 09:50:35
<p><input type="checkbox"/> 1. Structural Column and Inspection: Laboratory Verified Report Form DAS 291</p> <p><input type="checkbox"/> 2. Post-Installed Anchorage: Laboratory Verified Report Form DAS 291, or, for independently contracting SI, Special Inspection Verified Report Form DAS 292</p> <p><input type="checkbox"/> 3. Snow/Windling Inspection: Laboratory Verified Report Form DAS 291, or, for independently contracting SI, Special Inspection Verified Report Form DAS 292</p> <p><input type="checkbox"/> 4. Field Welding Inspection: Laboratory Verified Report Form DAS 291, or, for independently contracting SI, Special Inspection Verified Report Form DAS 292</p>		

NCS 103-22 LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOLIS). (2022) CBC					
NCS 103-22, Table 1705B.1 Table 1705B.1			School Owners: Newcomers Secondary PC District Contact: Newcomers PC Date Contacted: 2022-09-23 09:53:36		
NO	TEST	TEST FREQUENCY	TEST TYPE	PERFORMED BY	TEST COMMENTS
1	Test of Specimen	As required	Continuous	GI*	* By geotechnical engineer or he or she qualified representative.
2	Inspect drawing of pile and maintain complete and accurate records for project	Continuous	GI*		* By geotechnical engineer or he or she qualified representative.
3	Verify locations of piles and Test Results, confirm type and size of hammer, record blow counts per foot of penetration, determine penetration to achieve design capacity, record blow counts and penetration and record any pile damage.	Continuous	GI*		* By geotechnical engineer or he or she qualified representative.
4	Concrete piles	As required	Continuous	GI	Continuous tests and inspections per STEEL section below.
5	Steel piles and concrete filled piles	As required	Continuous	GI	Continuous tests and inspections per STEEL section below.
6	Concrete piles and concrete filled piles	As required	Continuous	GI	Continuous tests and inspections per CONCRETE section below.
7	For special cases, perform additional inspections as recommended by the registered design professional in accordance with	As required	Continuous	GI	As defined in drawings or specifications.
54 CAST-IN-PLACE DEEP FOUNDATIONS (PIERS)					
1	Test of Special Inspection	As required	Continuous	GI	Continuous tests and inspections per CONCRETE section below.
2	Inspect drawing of pier and maintain complete and accurate records for project	Continuous	GI		Continuous tests and inspections per CONCRETE section below.
3	Inspect drawing of pier and maintain complete and accurate records for project	Continuous	GI		Continuous tests and inspections per CONCRETE section below.
4	Verify pile locations, dimensions, penetration length/foot, determine penetration to achieve design capacity, record blow counts and penetration and record any pile damage.	Continuous	GI		Continuous tests and inspections per CONCRETE section below.
5	Concrete piers	As required	Continuous	GI	Continuous tests and inspections per CONCRETE section below.
6	Concrete piers	As required	Continuous	GI	Continuous tests and inspections per CONCRETE section below.

[illegible][illegible]

DAS 10A-22-2: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (OTHER), 2022 CBC			
Inspection Number:		School Name:	School District:
Inspector:		Name: Somerville PC	Nemo Somerville PC
Increment Number:		Date Created:	2022-09-21 10:55:15.6
XI. OTHER:			
<input type="checkbox"/>	Test or Special Inspection:	Type:	Code References and Notes
<input checked="" type="checkbox"/>	a. Load test for identified problem(s)	Test	LDR 1709A.2, 1709A.4. Testing is not required for a product with a valid evaluation service report per CSA approved as a product that can be certified by structural engineers.
<input checked="" type="checkbox"/>	b. Insulation/torque for non fire tests	Continuous	5"
<input checked="" type="checkbox"/>	c.		Applicable to commercial projects identified in Essential Service Facility Projects (ESFP) and/or other research was requested, verified by a listing inspection under Part 1801 Communication Tower, Post and Building project by State Agencies for Essential Services Communications (ESCS). ESCPs may use Fire-rated enclosures without need to OSA.

NCS 103-22 LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOLIS), 2022 CBC				
NCS 103-22, Table 1705B.1 Table 1705A.1		School District: Newcombs Public Date Created: 2023-05-29 09:38:36		
NO. 121717	ACTUAL NAME Newcombs Public Increment Number			
Test or Special Inspection	Type	Performed By	Code References and Notes	
55. RETAINING WALLS				
Test or Special Inspection	Type	Performed By	CODE 1-1 "By geotechnical engineer or his or her qualified representative. See section 3-2-1000.	
a. Placement, compaction and inspection of backfill	Continuous	GE*	"By geotechnical engineer or his or her qualified representative. See section 3-2-1000.	
b. Placement, compaction and inspection of drainage devices	Continuous	GE*	"By geotechnical engineer or his or her qualified representative.	
c. Segment retaining walls, inspect placement of utility, abutment, connectors, etc.	Continuous	GE*	"By geotechnical engineer or his or her qualified representative.	
d. Concrete retaining walls	Provide data on inspections per 3-2-1000 section below.			
e. Masonry retaining walls	Provide data on inspections per 3-2-1000 section below.			
56. OTHER SOLIS				
Test or Special Inspection	Type	Performed By	CODE 1-1 "By geotechnical engineer or his or her qualified representative. See section 3-2-1000.	
a. Soil Improvements	Continuous	GE*	"By geotechnical engineer or his or her qualified representative. See section 3-2-1000.	
b. Inspection of Soil Improvements	Continuous	GE*	"By geotechnical engineer or his or her qualified representative.	
c.				

ASA 103-22- LISTS OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC					
TMS 602-16 Table 4 Items 1 and 2					
Inspection Number		School District		School Districts	
Inspection Number		Newco Secondary PC		Newco Secondary PC	
Inspection Number		Inspection Number		Inspection Number	
Test or Test Description	Type	Performed By	Code Reference	Inspection Number	Inspection Number
1. Verify size, grade, placement of reinforcement, connections, and anchors bolts. Verify condition of structural members.	Periodic	SI	TMS 602-16 Table 4 Items 1 & 2c.		
2. Inspect placement of reinforcement for bar bolls, and connections.	Continuous	SI	TMS 602-16 Table 4 Item 2a.		
3. Placement, consolidation, and reinspection of concrete.	Continuous	SI	TMS 602-16 Table 4 Item 1 & 2b.		
4. Inspect placement of masonry units and construction of mortar joints.	Periodic	SI	TMS 602-16 Table 4 Item 1 & 2b.		
5. Verify placement, construction and protection of masonry and/or weathering elements below 40' (9' for hot weather temperature above 50°F).	Periodic	SI*	TMS 602-16 Table 4 Item 1 & 2b. May be performed by the project inspector when specifically approved by ISA.		
6. Inspect type, size, installation of anchors and all other items to be embedded in masonry including other details.	Continuous	SI	TMS 602-16 Table 4 Item 2a.		
7. Inspect type, size, installation of structural members, frames and other construction.	Continuous	SI	TMS 602-16 Table 4 Item 2a.		
8. Inspect ground cover, including mortar protrusions, joints and placement of ground.	Continuous	SI	TMS 602-16 Table 4 Item 2a.		
9. Wetting of reinforcing steel.			TMS 602-16 Table 4 Item 3a. Provide specific inspection for STEEL, Category S/AMG, 6 (at anchor) and S/AMG 10 (w/cover).		

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DSA 103-122-2: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC							
DSA 103-122-2, Tables 1 and 2		SCHOOL NAME: New Brunswick PC		SCHOOL DISTRICT: New Brunswick PC			
Job Number: 04-122-0000		Inspector Name: 04-122-0000		Inspector Number: 04-122-0000			
M1. MINERAL WOOL BLOSSOM PROTECTION							
Test or Special Inspection	Type	Performed By	Code References and Notes				
1. Verify projections of steel reinforcement bars and anchor bolts are embedded in masonry veneer including details of anchorage of masonry to concrete foundation and other connections.	Periodic	SI	TMS-602-16 Table 3 Item 5 and Table 3 Item 14, 16, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737				

[illegible][illegible]

NOTE: THE DSA 103 SHOWN INCLUDES MINIMUM SPECIAL INSPECTION REQUIREMENTS AND IS PROVIDED AS EXAMPLE. ADDITIONAL TESTING AND INSPECTIONS MAY BE REQUIRED BEYOND THE SCOPE OF THE SCOREBOARD. A FINAL DSA 103 FORM SHALL BE SUBMITTED BY THE DESIGN PROFESSIONAL OF RECORD AS PART OF THE SITE SPECIFIC SUBMITTAL REQUIREMENTS. THE DESIGN PROFESSIONAL OF RECORD IS RESPONSIBLE FOR REVIEWING PROJECT SPECIFIC SPECIAL INSPECTION REPORTS. SSG STRUCTURAL ENGINEERS OR NEVCO ARE NOT RESPONSIBLE FOR PROVIDING THE PROJECT DSA 103 OR REVIEWING SPECIAL INSPECTION REPORTS.

DSA 103 CAN BE FOUND AT: <https://forms.dgs.ca.gov/content/forms/at/dgs/dsa/form-103/public/dsa-form-103-22.html>

APPLICATION #
02-120886

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT

APP: 02-120886 INC:
REVIEWED FOR

DATE: ☒ FLS ☐ ACS ☐

DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
No. 5405
STATE OF CALIFORNIA

DATE ISSUED: 08.09.2023

PC SEQR SEAL

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THANK YOU FOR YOUR INTEREST IN SSG'S REGISTERED PROJECT.



NEVCO

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Phone: (618) 664-0360
www.nevco.com

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required

EXAMPLE DSA 103 - TESTING AND INSPECTIONS

SHEET INFORMATION	
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSQ JOB #	S23109
SHEET	3000

ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

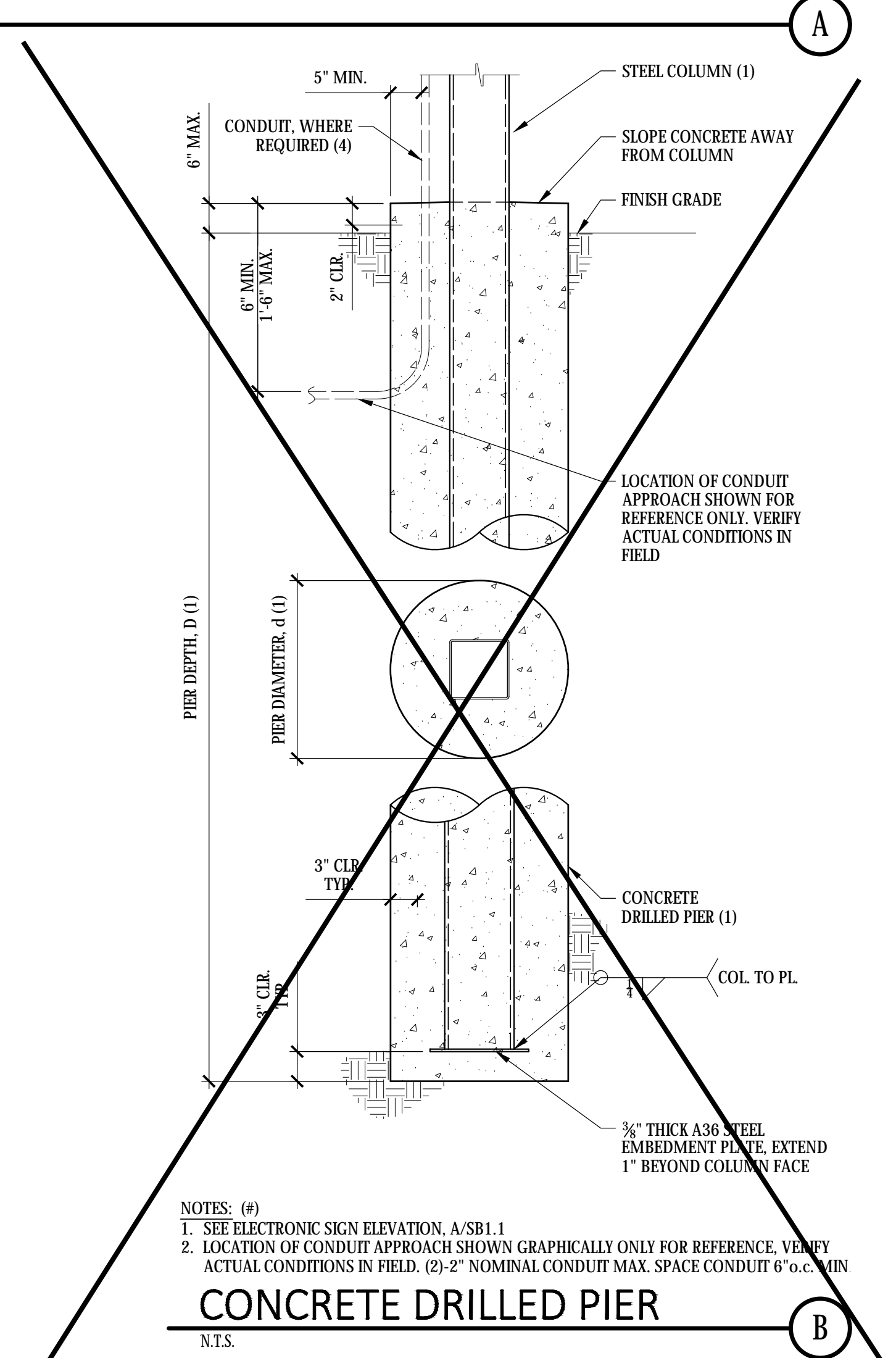
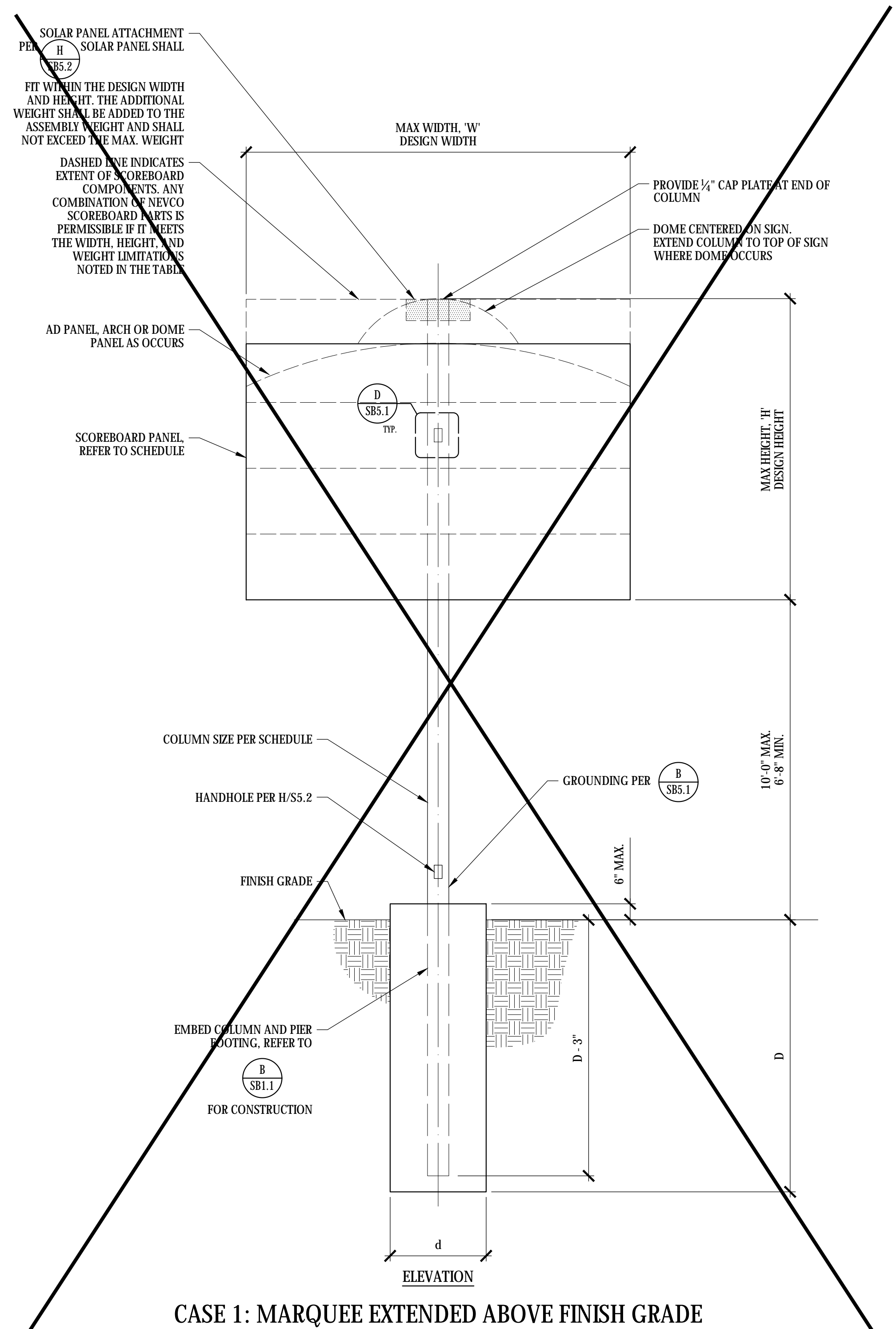
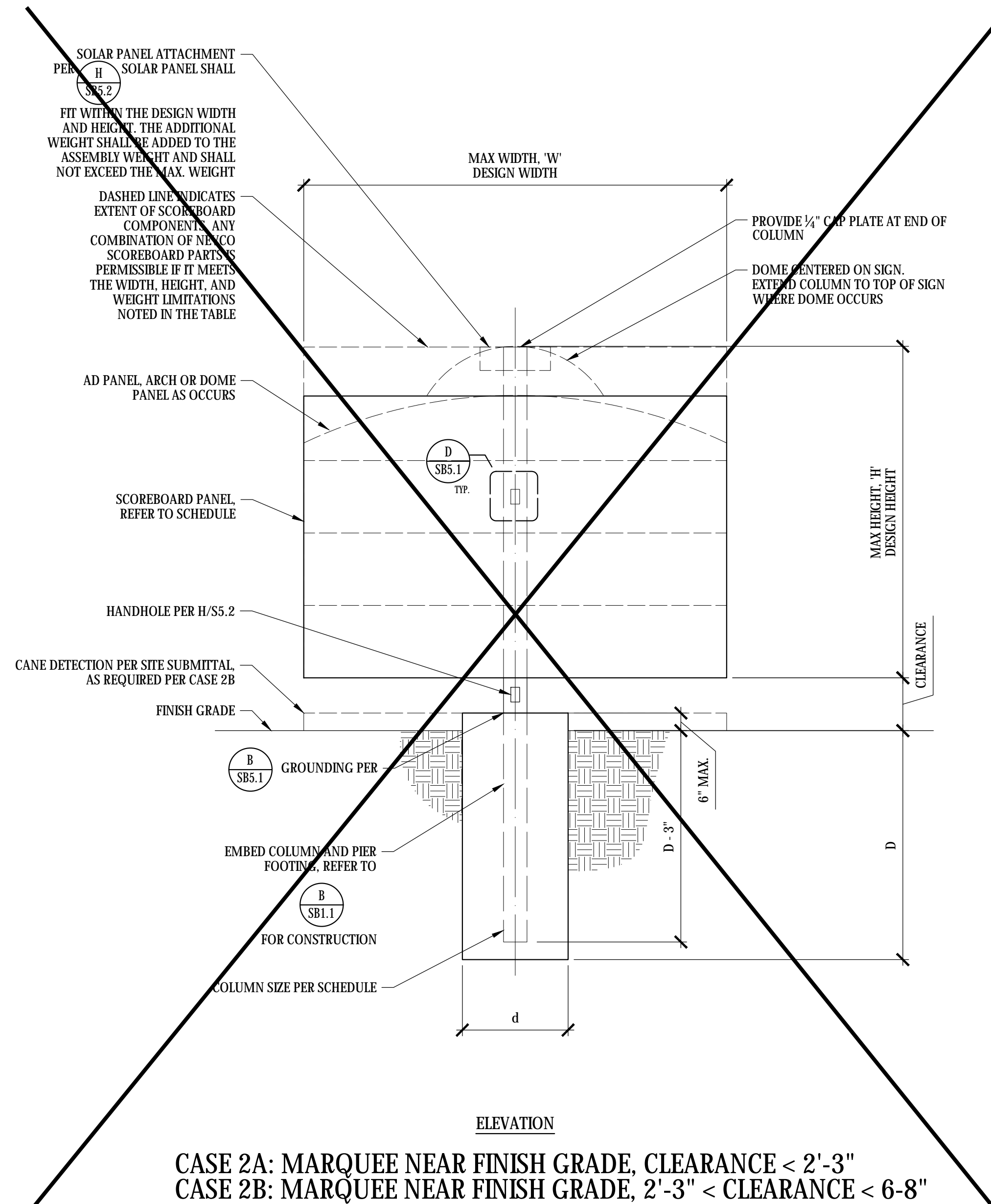
N.T.S.

ONE COLUMN ASSEMBLY - CASE 1								
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	6'-6"	N/R	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	9'-0"	N/R	N/R

NOTES:
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06

ONE COLUMN ASSEMBLY - CASE 2A ⁽²⁾ AND CASE 2B ⁽²⁾⁽³⁾								
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	5'-3"	N/R	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	6'-6"	N/R	N/R

NOTES:
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06
2. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.
3. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD



NOTES: (4)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB1.1
2. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2)-2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.

CONCRETE DRILLED PIER

N.T.S.

APPLICATION # 02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5485
STRUCTURAL
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC SEOR REAL

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nevco

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APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐ CG ☐
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CALIFORNIA R. RANNEY
No. 523366
EXPIRES 3-31-24
STRUCTURAL
DATE SIGNED: 08.09.2023

MARQUEE
CAISSON -
EMBEDDED

SHEET INFORMATION	
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	SB1.1

NOTES: (#)
1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

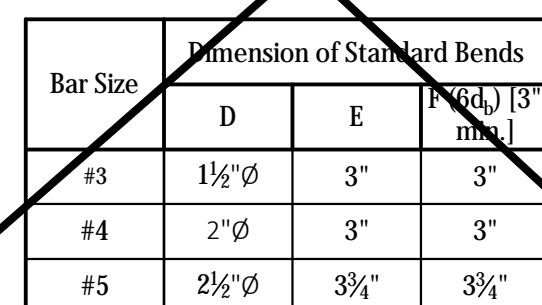
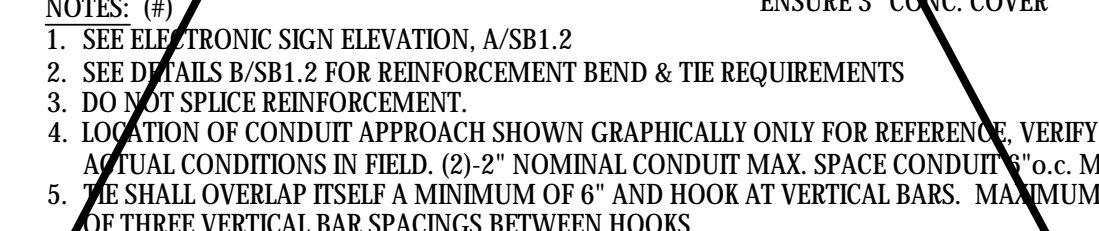
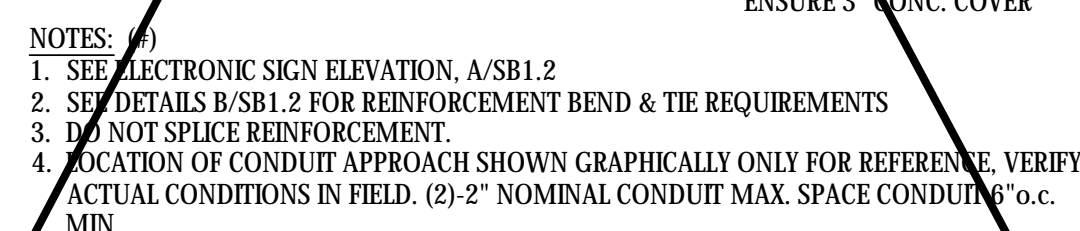
NOTES: (#)

1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB.1.2 FOR THE OPTION, SEE D/SB.1.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.
3. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.
4. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD



N.T.S.

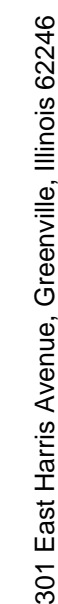
NOTES: (#)
1. SEE SCOREBOARD ELEVATION, A/SB1.2



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DSA STAMP

PRE-CHECK (PC) DOCUMENT

A separate project application for construction is required.



SHEET INFORMATION

DATE 08.09.2023

DRAWN
JMK

CHECKED MFP

SSG JOB # _____

SHEET **SB1.2**

ONE COLUMN ASSEMBLY - CASE 2A ⁽²⁾ AND CASE 2B ⁽²⁾⁽³⁾							
ASSEMBLY CRITERIA					MAT FOOTING CRITERIA		
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	WIDTH, A	DEPTH, B	LENGTH, L
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x8	4'-6"	2'-6"	4'-6"
10'-0"		1,355 lbs.	≤ 8'-0"	HSS8x8x8	6'-0"	2'-6"	6'-0"

ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

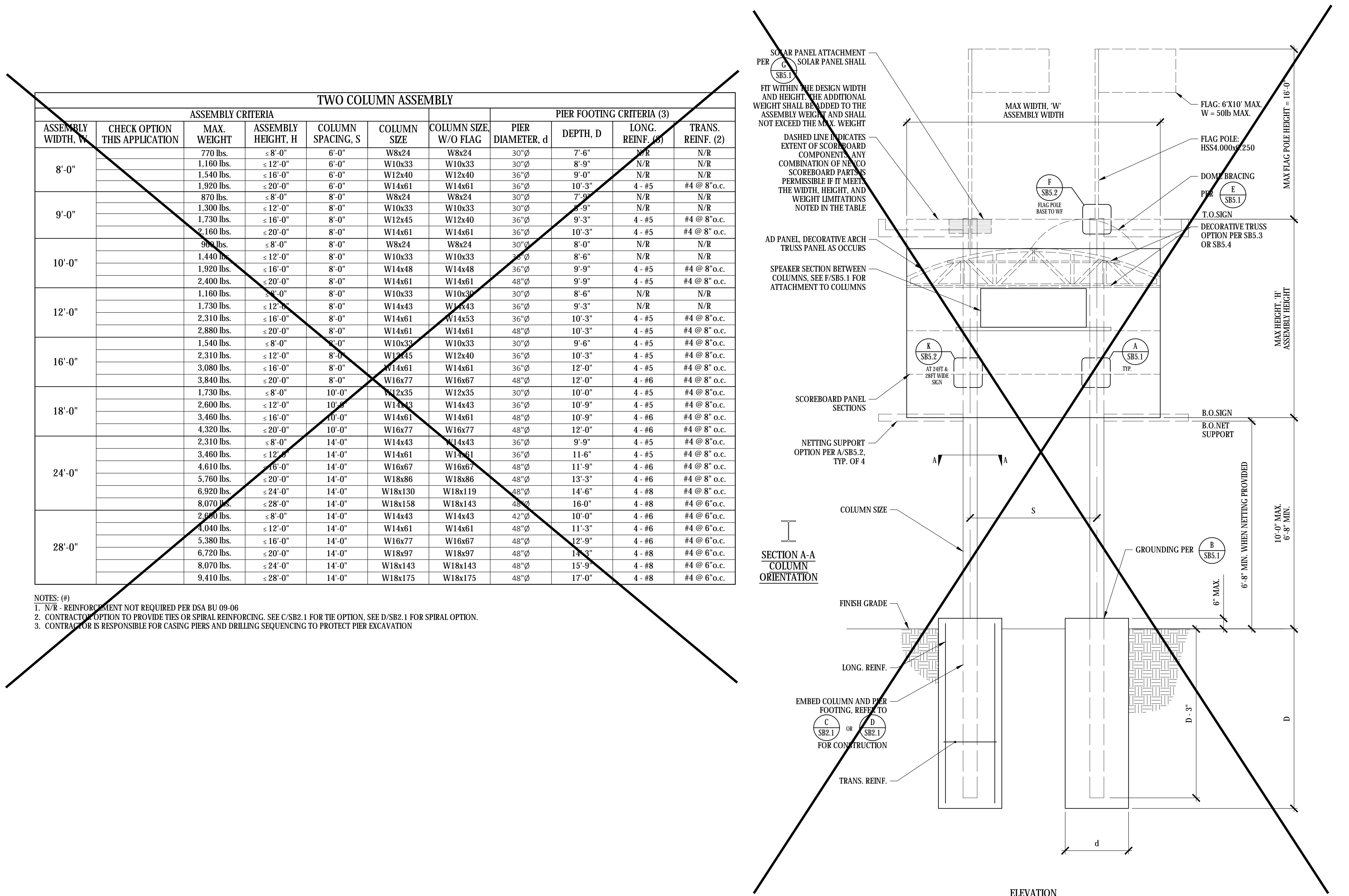


MAT FOOTING CONSTRUCTION AND ANCHORAGE

NOTES: (#)
1. SEE SCOREBOARD ELEVATION, A/SB1.3

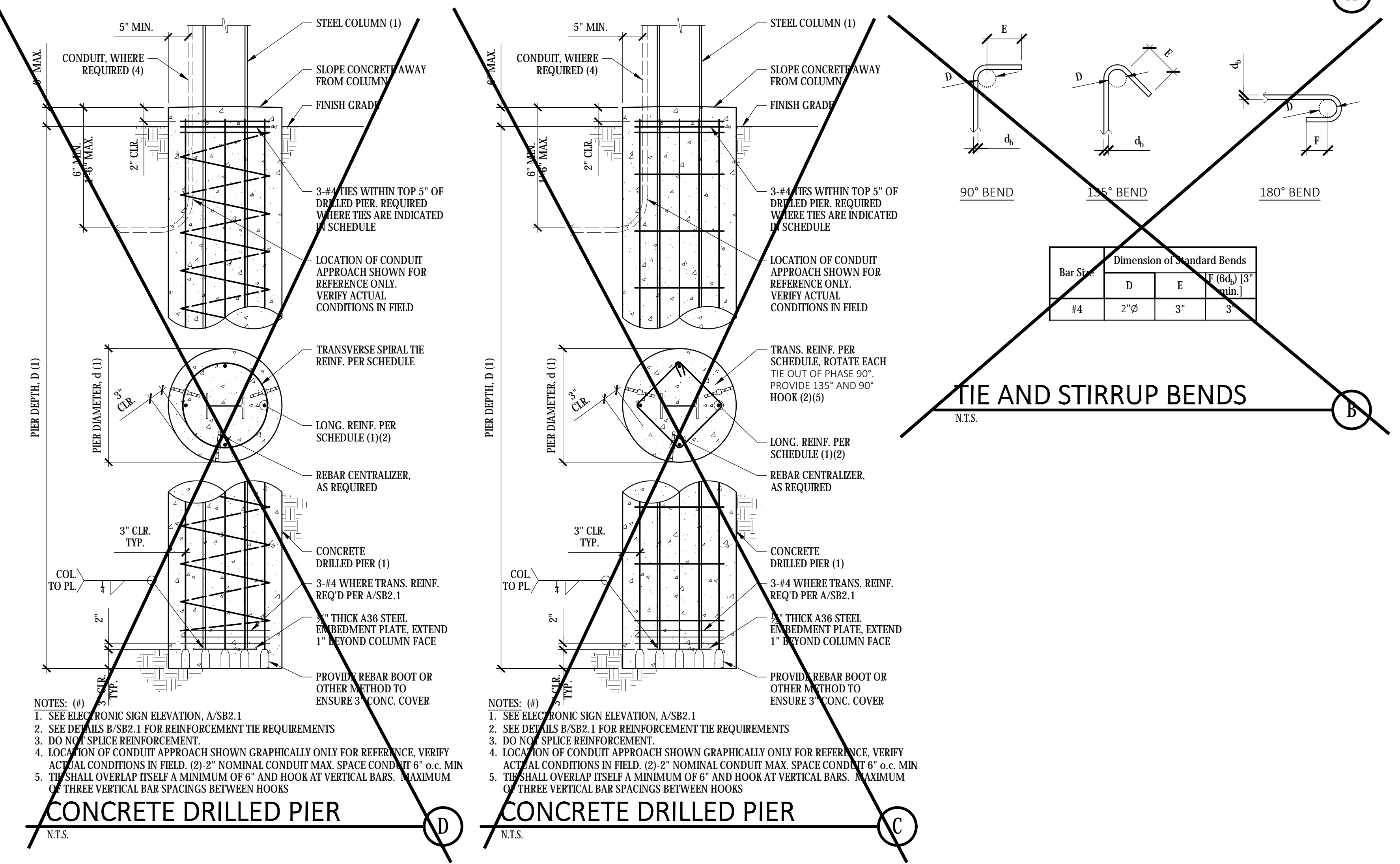


Bar Size	Dimension of Standard Bends		
	D	E	F (6d, 3 mm)
#3	1½"Ø	3"	3"
#4	2"Ø	3"	3"
#5	2½"Ø	3¾"	3¾"



TWO COLUMN SCOREBOARD INSTALLATION

N.T.S.



APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STATE OF CALIFORNIA
PC SEOR SEAL
08.09.2023

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APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐ CG ☐
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CALIFORNIA R. RANDOLPH
No. 52336
EXPIRES
3-31-25
STRUCTURAL

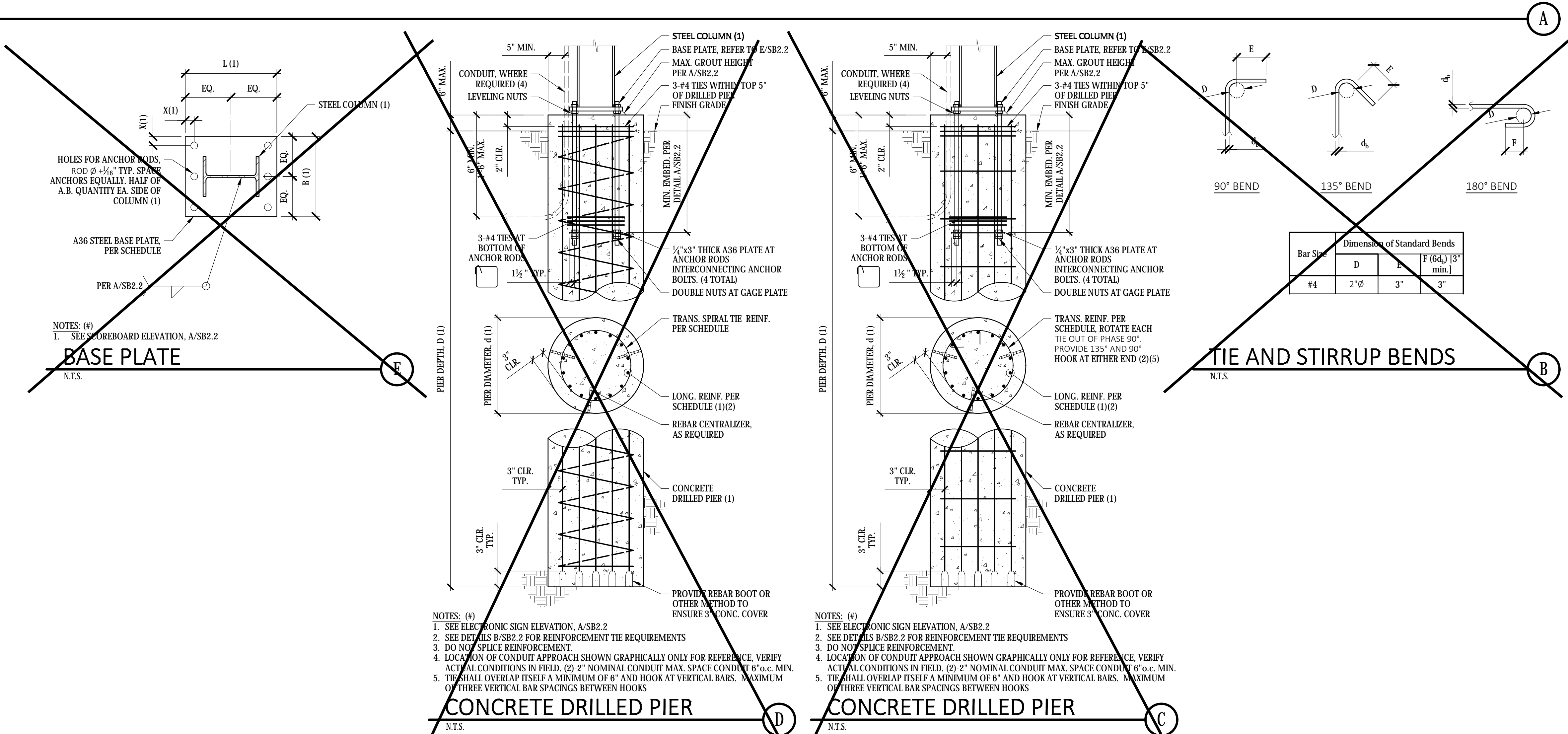
SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB2.1

TWO COLUMN ASSEMBLY																			
		ASSEMBLY CRITERIA			PIER FOOTING CRITERIA (2)				BASE PLATE				ANCHOR RODS						
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
8'-0"		770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	36"Ø	7'-0"	8-#6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	36"Ø	8'-0"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	36"Ø	9'-0"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		1,920 lbs.	≤ 20'-0"	6'-0"	W14x61	W14x61	42"Ø	9'-9"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
9'-0"		870 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	36"Ø	7'-3"	8-#6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-3"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x40	36"Ø	9'-3"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		2,160 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	42"Ø	10'-0"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
10'-0"		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	36"Ø	7'-6"	8-#6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-6"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,920 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	42"Ø	9'-9"	8-#6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.105	2½"	2"	48"
		2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	42"Ø	9'-9"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.105	2½"	2"	64"
12'-0"		1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-0"	8-#6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	W14x43	42"Ø	9'-3"	8-#6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	42"Ø	10'-3"	8-#8	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		2,880 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-3"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅞	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
16'-0"		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-9"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	36"Ø	10'-3"	8-#6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	48"
		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	12'-0"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	12-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.105	2½"	2"	64"
18'-0"		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	36"Ø	9'-0"	8-#6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	42"Ø	10'-0"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
		3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	48"Ø	10'-9"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		4,320 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x67	48"Ø	13'-0"	12-#8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"
24'-0"		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	36"Ø	9'-9"	8-#6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		3,460 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	36"Ø	11'-6"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		4,610 lbs.	≤ 16'-0"	14'-0"	W16x67	W16x67	48"Ø	11'-9"	12-#8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(4) - 1½"	F1554 - GR.55	3"	2"	64"
		5,760 lbs.	≤ 20'-0"	14'-0"	W18x86	W18x86	48"Ø	13'-3"	12-#8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"
28'-0"		6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	W18x119	48"Ø	14'-6"	12-#8	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"	F1554 - GR.105	3"	2"	64"
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	54"Ø	16'-0"	12-#8	#4 @ 6" o.c.	2½"	24"	36"	CIP	(6) - 2"	F1554 - GR.105	4"	2"	64"
		2,690 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	42"Ø	10'-0"	8-#7	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
		4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	48"Ø	11'-3"	8-#8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(4) - 1½"	F1554 - GR.55	3"	2"	64"
28'-0"		5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x67	48"Ø	12'-9"	12-#8	#4 @ 6" o.c.	2"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	48"Ø	14'-3"	12-#8	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"	F1554 - GR.105	3"	2"	64"
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	54"Ø	15'-9"	12-#8	#4 @ 6" o.c.	2½"	24"	36"	CIP	(6) - 2"	F1554 - GR.105	4"	2½"	64"
		9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	54"Ø	16'-6"	14-#8	#4 @ 6" o.c.	3"	24"	36"	CIP	(6) - 2"	F1554 - GR.105	4"	2½"	64"

NOTES: (4)
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB2.2 FOR TIE OPTION. SEE D/SB2.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.

TWO COLUMN SCOREBOARD INSTALLATION

N.T.S.



APPLICATION # 02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC.
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STATE OF CALIFORNIA
PC SEOR REAL 08.09.2023

THESE DRAWINGS, NOTES AND DETAILS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF SSG STRUCTURAL ENGINEERS, LLP. ALL DRAWINGS, INFORMATION, SPECIFICATIONS, NOTES, DETAILS AND INSTRUMENTS OF SERVICE, INCLUDING THIS DOCUMENT, SHALL REMAIN THE PROPERTY OF THE ENGINEER. NO PART THEREOF SHALL BE REPRODUCED, COPIED, OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED AND DEVELOPED WITHOUT THE EXPRESS WRITTEN CONSENT OF THE ENGINEER. COPYRIGHT 2024. THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS.

nevco

301 East Harris Avenue, Greenville, Illinois 62246
Phone: (618) 664-0960
www.nevco.com

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122088 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐ CG ☐
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022

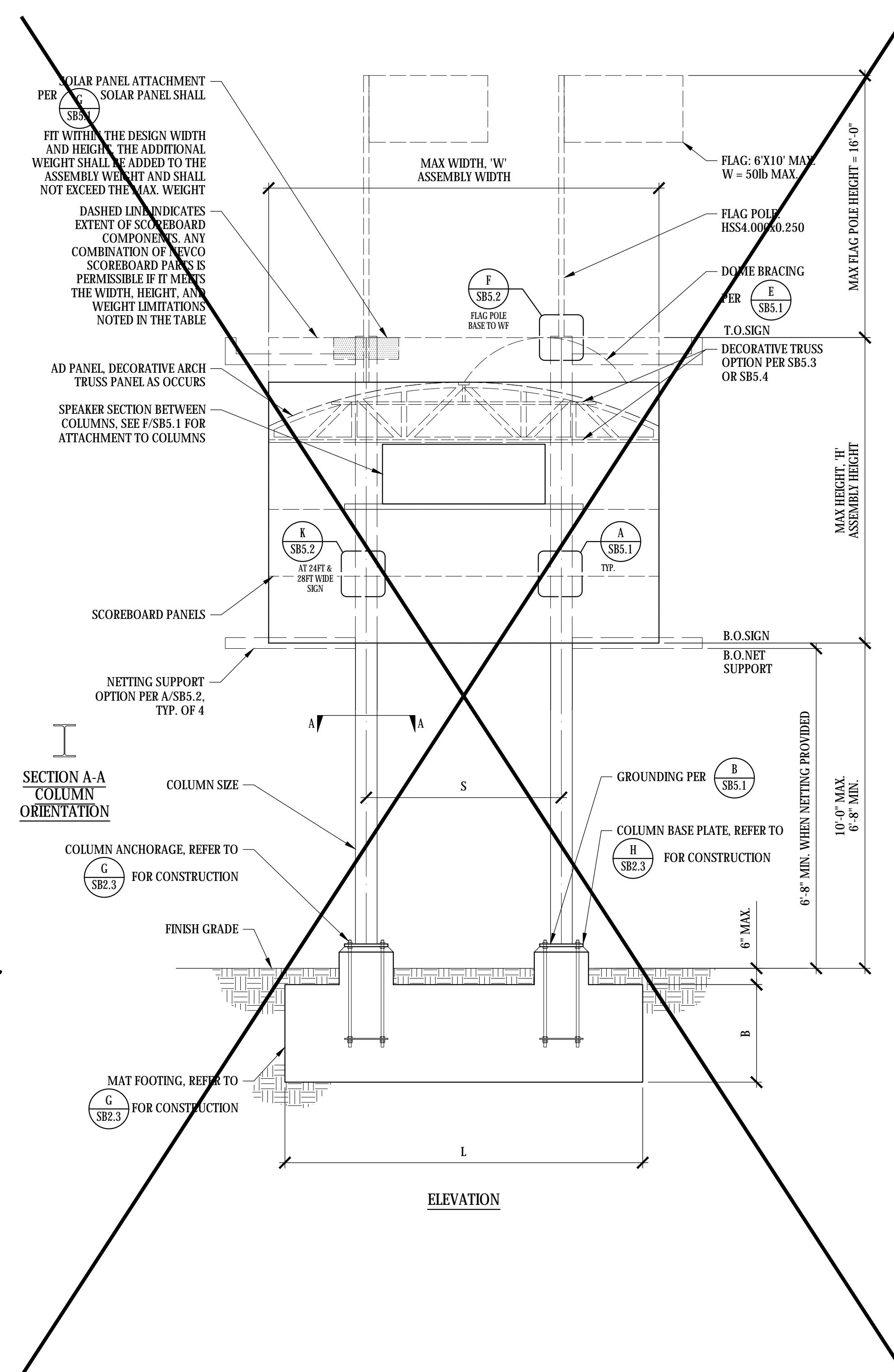
A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 523386
STATE OF CALIFORNIA
PC SEOR REAL 08.09.2023

TWO COLUMN
CAISSON -
BOLTED

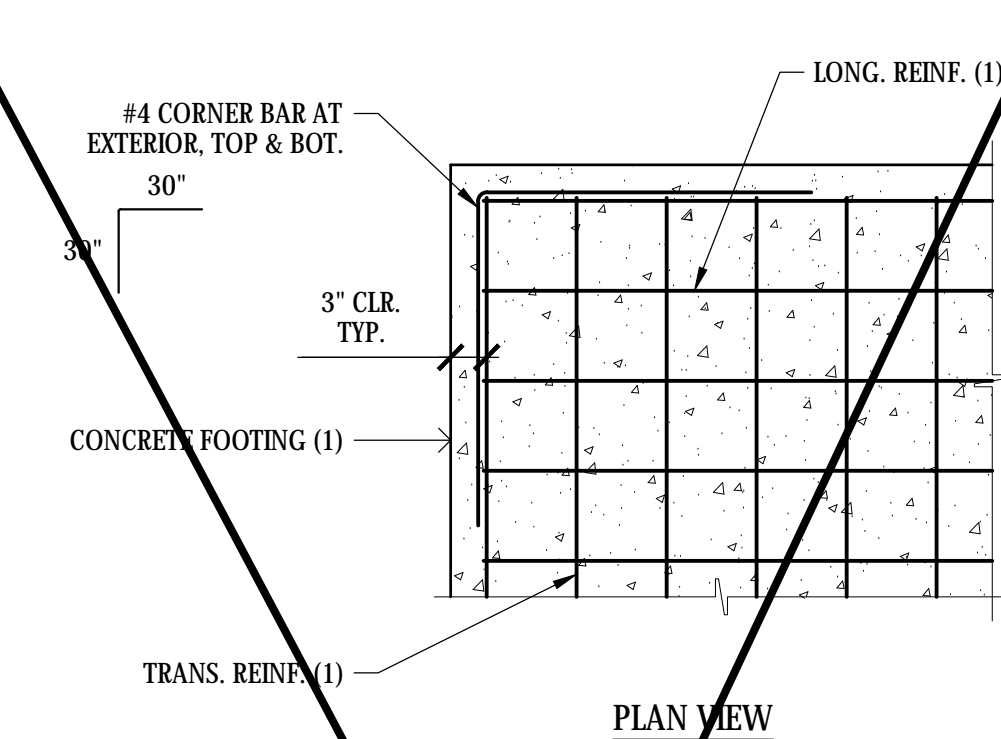
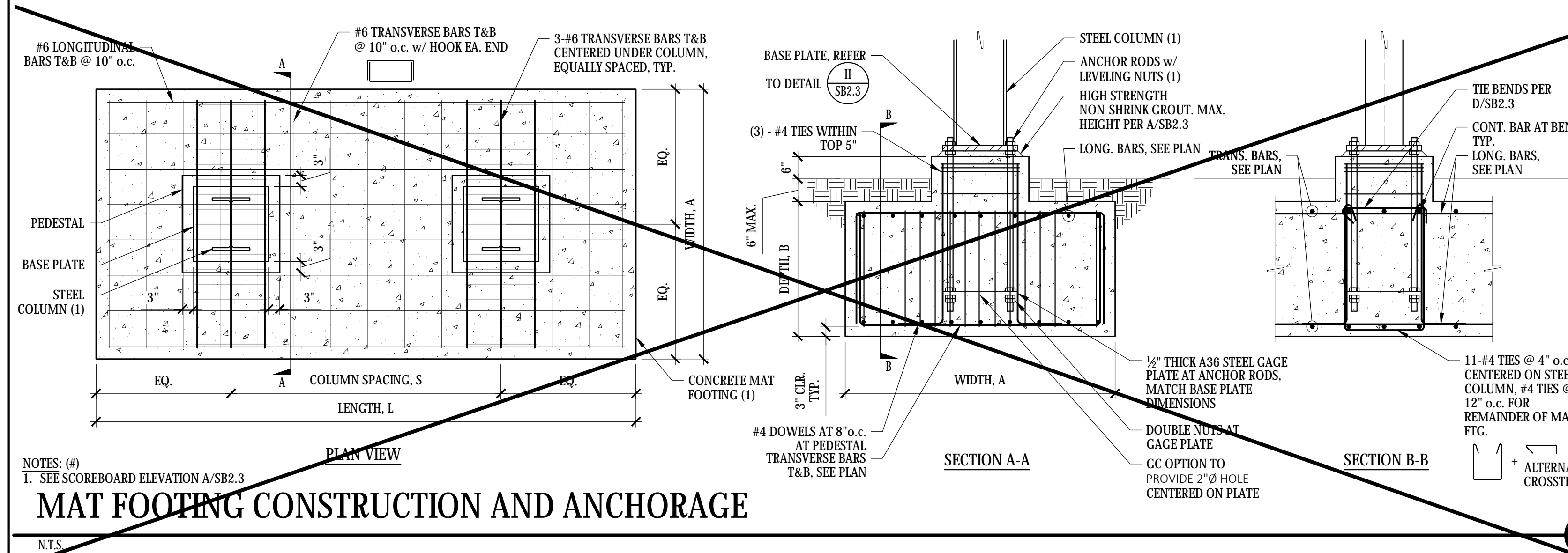
SHEET INFORMATION
DATE 08.09.2023
DRAWN JMK
CHECKED MEP
SSG JOB # S23109
SHEET SB2.2

TWO COLUMN ASSEMBLY																		
ASSEMBLY CRITERIA						MAT FOOTING CRITERIA				BASE PLATE				ANCHOR RODS				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
8'-0"		770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	7'-0"	3'-0"	10'-0"	1"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	8'-0"	3'-0"	12'-0"	1 1/4"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	9'-0"	3'-0"	13'-0"	1 1/2"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		1,920 lbs.	≤ 20'-0"	6'-0"	W14x61	W14x61	11'-0"	3'-0"	14'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
9'-0"		870 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	7'-0"	3'-0"	11'-0"	1"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	13'-0"	1 1/4"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x40	9'-0"	3'-0"	15'-0"	1 1/2"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		2,160 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	16'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
10'-0"		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	7'-0"	3'-0"	11'-0"	1"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	13'-0"	1 1/4"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,920 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	9'-6"	3'-0"	14'-0"	1 1/2"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.105	2 1/2"	2"	30"
		2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	11'-0"	3'-0"	16'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.105	2 1/2"	2"	30"
12'-0"		1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	7'-6"	3'-0"	12'-0"	1"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	W14x43	9'-0"	3'-0"	12'-0"	1 1/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	10'-0"	3'-0"	14'-0"	1 1/2"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		2,880 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	11'-0"	3'-0"	17'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
16'-0"		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	7'-6"	3'-0"	13'-0"	1"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	9'-0"	3'-0"	15'-0"	1 1/2"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	17'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	11'-6"	3'-0"	20'-0"	1 3/4"	24"	24"	1/2	(4) - 1/2"Ø	F1554-GR.105	2 1/2"	2"	30"
18'-0"		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	8'-0"	3'-0"	13'-0"	1"	20"	20"	3/8	(4) - 1/2"Ø	F1554-GR.36	2 1/2"	2"	30"
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	9'-6"	3'-0"	15'-0"	1 1/2"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	11'-0"	3'-0"	16'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		4,320 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x77	13'-0"	3'-0"	17'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	3"	2"	30"
24'-0"		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	8'-6"	3'-0"	17'-0"	1 1/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		3,460 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	9'-6"	3'-0"	19'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		4,610 lbs.	≤ 16'-0"	14'-0"	W16x67	W16x67	11'-0"	3'-0"	20'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	3"	2"	30"
		5,760 lbs.	≤ 20'-0"	14'-0"	W18x86	W18x86	13'-0"	3'-0"	21'-0"	1 3/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	3"	2"	30"
28'-0"		6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	W18x119	14'-0"	4'-0"	22'-0"	2"	24"	30"	CP	(6) - 1/2"Ø	F1554-GR.105	3"	2"	36"
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	15'-0"	4'-0"	23'-0"	2 1/2"	24"	36"	CP	(6) - 1/2"Ø	F1554-GR.105	4"	2 1/2"	36"
		2,600 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	8'-0"	3'-0"	18'-0"	1 1/4"	24"	24"	3/8	(4) - 1/2"Ø	F1554-GR.55	2 1/2"	2"	30"
		4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	10'-0"	3'-0"	19'-0"	1 3/4"	24"	30"	3/8	(4) - 1/2"Ø	F1554-GR.55	3"	2"	30"
28'-0"		5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x67	11'-0"	3'-0"	22'-0"	2"	24"	30"	1/2	(6) - 1/2"Ø	F1554-GR.55	3"	2"	30"
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	13'-0"	3'-0"	23'-0"	2"	24"	30"	CP	(6) - 1/2"Ø	F1554-GR.105	3"	2"	30"
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	14'-0"	4'-0"	24'-0"	2 1/2"	24"	36"	CP	(6) - 2"Ø	F1554-GR.105	4"	2 1/2"	36"
		9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	15'-0"	4'-0"	24'-0"	3"	24"	36"	CP	(6) - 2"Ø	F1554-GR.105	4"	2 1/2"	36"

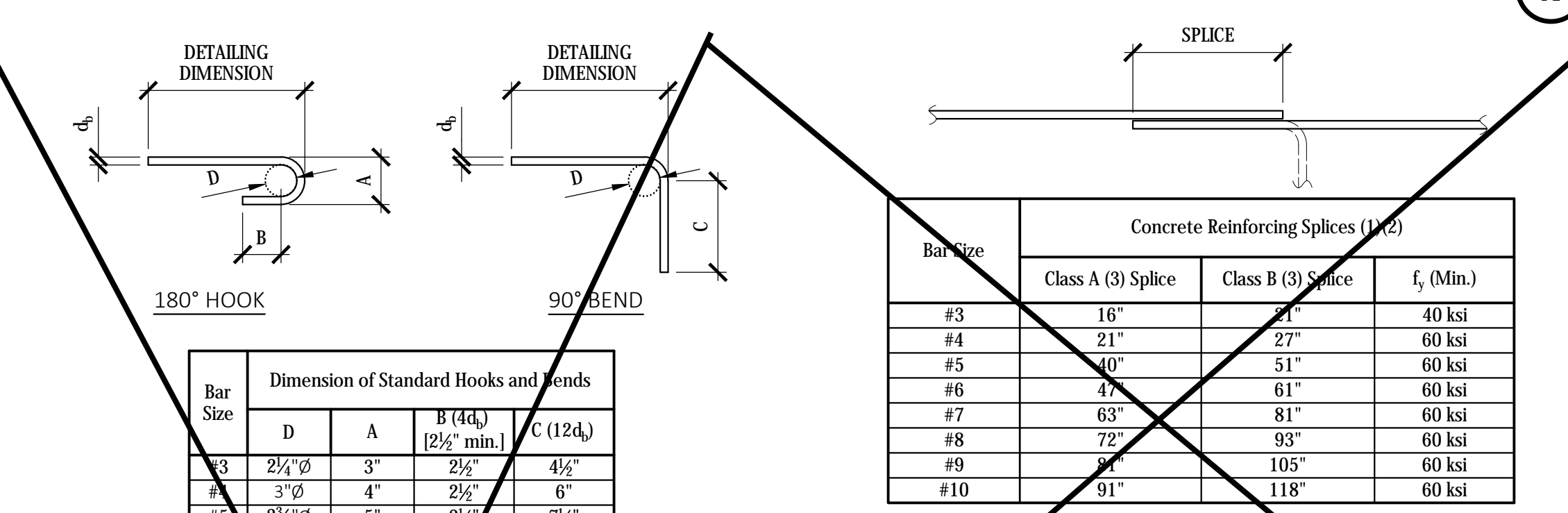


TWO COLUMN SCOREBOARD INSTALLATION

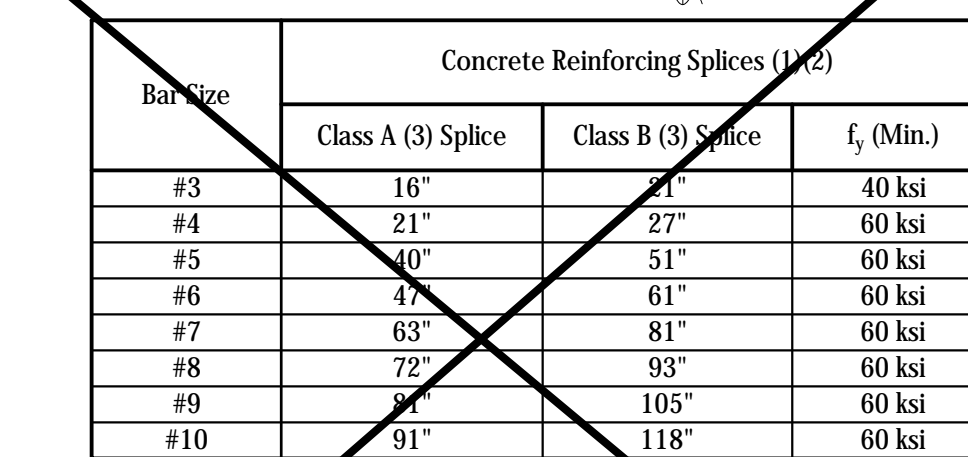
N.T.S.



NOTES: (#)
1. SEE G/SB2.3

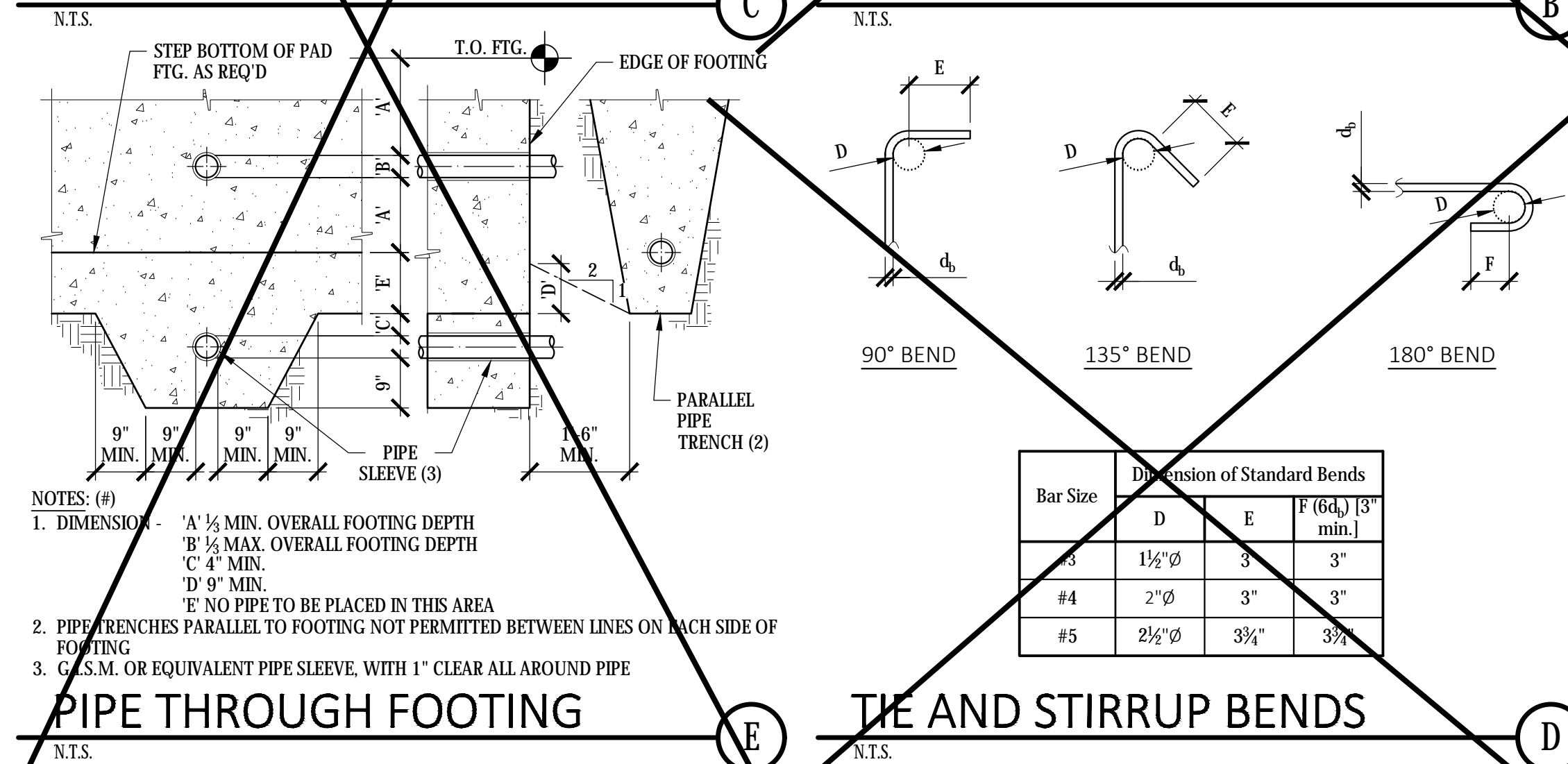


REBAR HOOKS & BENDS

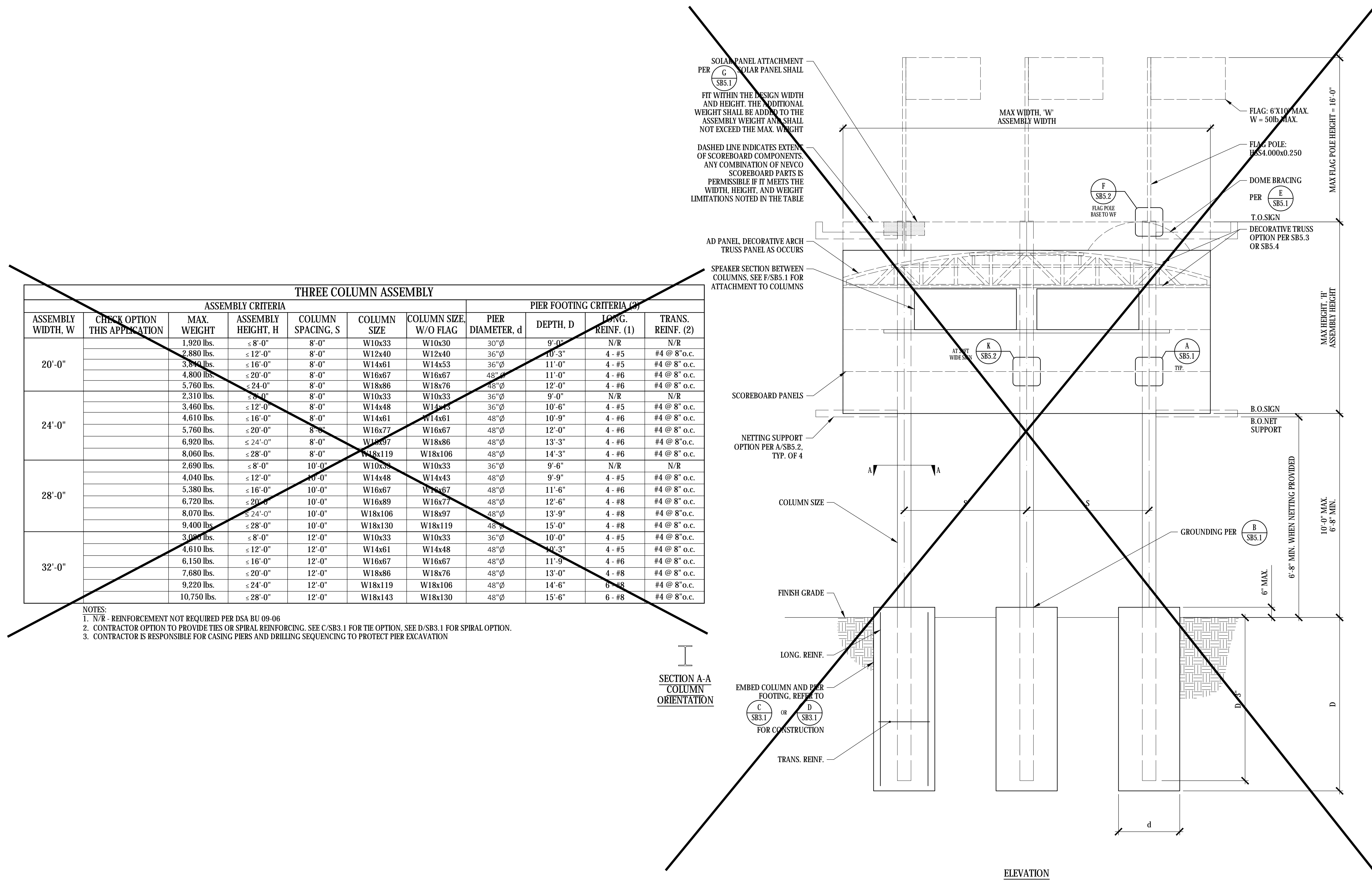


NOTES: (#)
1. LAP LENGTHS LISTED APPLY TO HORIZONTAL REINFORCEMENT
2. WHERE BARS OF A DIFFERENT SIZE ARE LAPPED, THE LAP LENGTH SHALL BE THE LENGTH REQUIRED BY THE LARGER BAR
3. ALL SPLICES SHALL BE CONSIDERED CLASS B UNLESS SPECIFICALLY NOTED OTHERWISE

~~TYPICAL LAP SPLICES~~



~~TIE AND STIRRUP BENDS~~

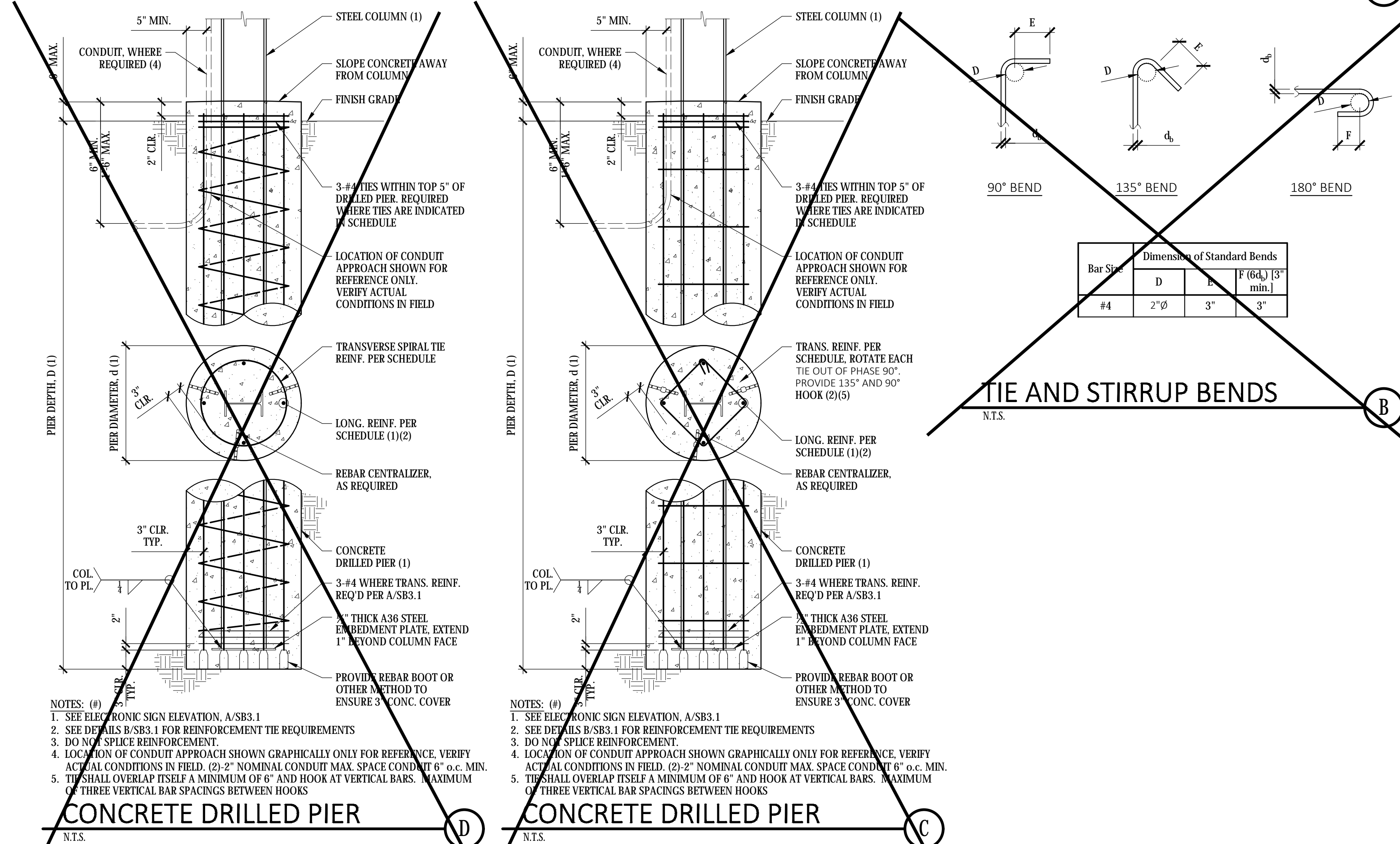


THREE COLUMN ASSEMBLY									
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA			PIER FOOTING CRITERIA (1)				
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	TRANS. REINF. (2)
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	30"Ø	9'-0"	N/R
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	36"Ø	10'-3"	4- #5
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x51	W14x53	36"Ø	11'-0"	4- #5
		4,800 lbs.	≤ 20'-0"	8'-0"	W16x67	W16x67	48"Ø	11'-0"	4- #6
24'-0"		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	48"Ø	12'-0"	4- #6
		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-0"	N/R
		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	36"Ø	10'-6"	4- #5
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-9"	4- #6
28'-0"		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	4- #6
		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	48"Ø	13'-3"	4- #6
		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	48"Ø	14'-3"	4- #6
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R
32'-0"		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	48"Ø	9'-9"	4- #5
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-6"	4- #6
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	4- #8
		8,070 lbs.	≤ 24'-0"	10'-0"	W18x106	W18x97	48"Ø	13'-9"	4- #8
		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	48"Ø	15'-0"	4- #8
		3,090 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-0"	4- #5
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	11'-3"	4- #5
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	11'-9"	4- #6
		7,680 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	48"Ø	13'-0"	4- #8
		9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	48"Ø	14'-6"	4- #8
		10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	48"Ø	15'-6"	4- #8

- NOTES:
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06
2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB3.1 FOR TIE OPTION, SEE D/SB3.1 FOR SPIRAL OPTION.
3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

THREE COLUMN SCOREBOARD INSTALLATION

N.T.S.



- NOTES: (H)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB3.1
2. SEE DETAILS B/SB3.1 FOR REINFORCEMENT TIE REQUIREMENTS
3. DO NOT SPICE REINFORCEMENT.
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

- NOTES: (H)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB3.1
2. SEE DETAILS B/SB3.1 FOR REINFORCEMENT TIE REQUIREMENTS
3. DO NOT SPICE REINFORCEMENT.
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

APPLICATION #
02-122088

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APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STATE OF CALIFORNIA
PC SEOR REAL
08.09.2023

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

nevco
301 East Harris Avenue, Greenville, Illinois 62246
Phone: (618) 664-0960
www.nevco.com

APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

DSA STAMP

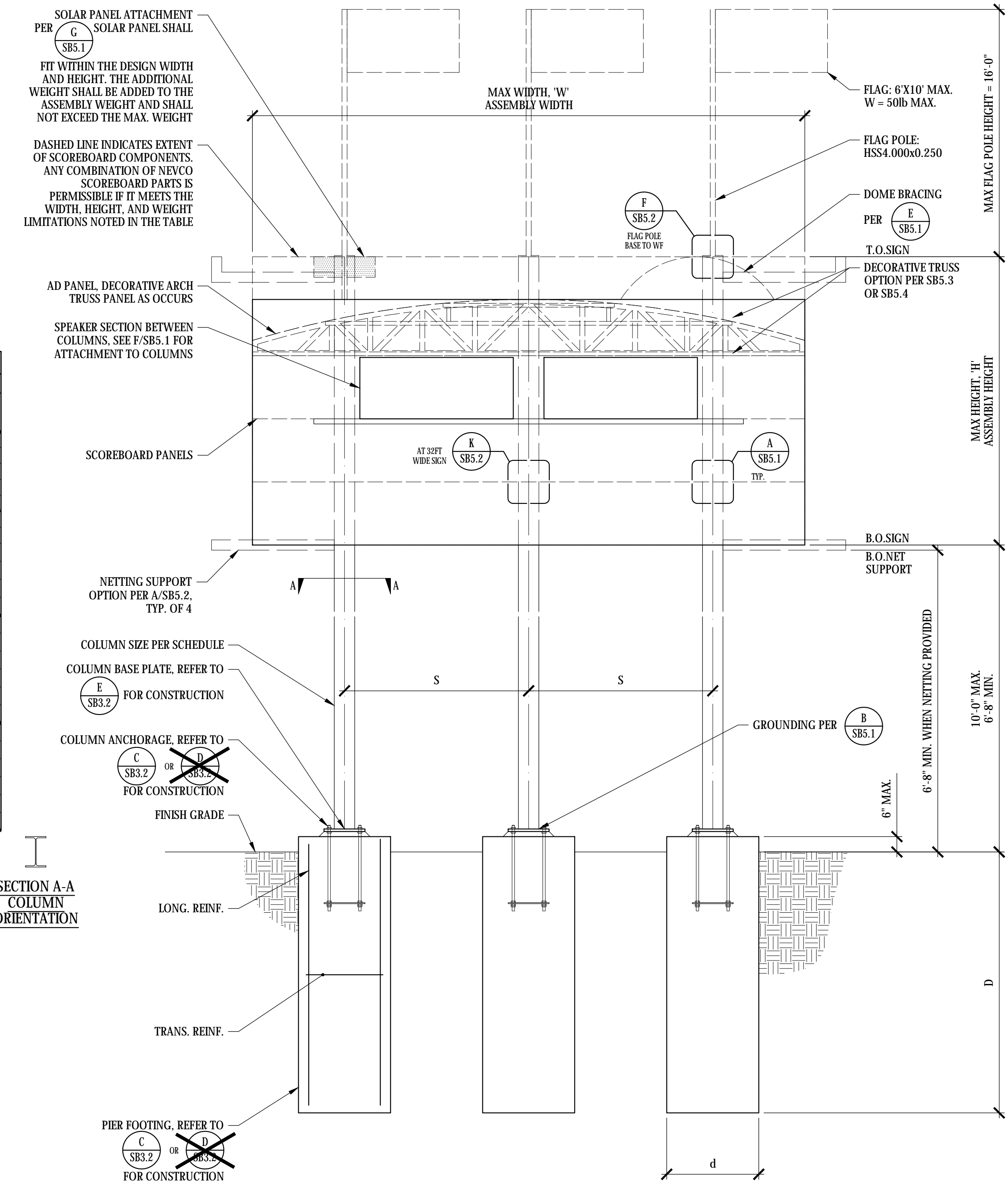
PRE-CHECK (PC) DOCUMENT
CODE: 2022
A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CALIFORNIA R. RANDOLPH
No. 52386
EXPIRES
3-31-25
STRUCTURAL

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB3.1

THREE COLUMN ASSEMBLY																			
ASSEMBLY CRITERIA						PIER FOOTING CRITERIA (2)					BASE PLATE				ANCHOR RODS				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅝	(4) - 1½"Ø	F1554 - GR.36	2½"	2"	48"
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x46	W12x40	36"Ø	10'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅝	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	42"Ø	10'-6"	14 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅝	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		4,800 lbs.	≤ 20'-0"	8'-0"	W16x67	W16x60	48"Ø	11'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅝	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	48"Ø	12'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
24'-0"	X	2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅝	(4) - 1½"Ø	F1554 - GR.36	2½"	2"	48"
		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	36"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	⅝	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		4,610 lbs.	≤ 16'-0"	8'-0"	W16x61	W16x54	42"Ø	10'-6"	14 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅝	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	10'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅝	(6) - 1½"Ø	F1554 - GR.105	2½"	2"	64"
		6,880 lbs.	≤ 24'-0"	8'-0"	W18x87	W18x80	48"Ø	10'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
28'-0"		8,060 lbs.	≤ 28'-0"	8'-0"	W18x110	W18x106	48"Ø	14'-0"	14 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅝	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	48"Ø	9'-9"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅝	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	14'-6"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅝	(4) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	14 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
32'-0"		8,070 lbs.	≤ 24'-0"	10'-0"	W18x106	W18x97	48"Ø	13'-9"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	48"Ø	15'-0"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		3,080 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅝	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-3"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅝	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	14'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅝	(4) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		7,680 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	48"Ø	13'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		10,730 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	54"Ø	15'-6"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.55	4"	2"	64"

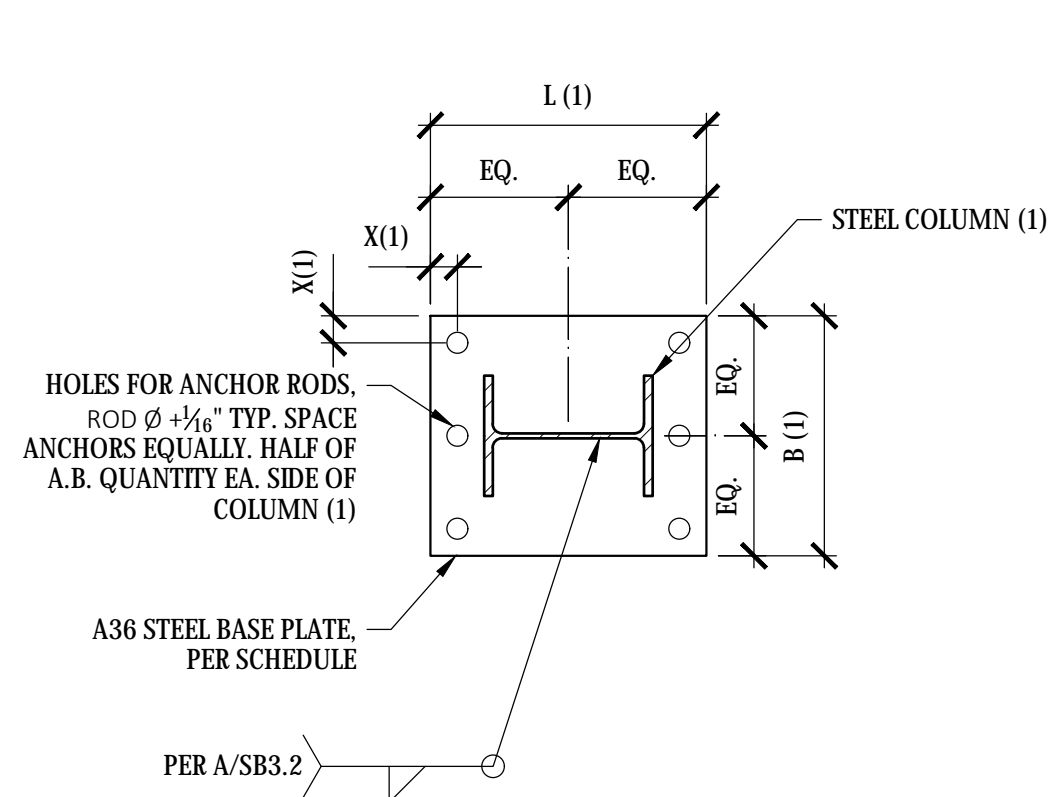
NOTES:
1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB3.2 FOR THE OPTION, SEE D/SB3.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



ELEVATION

THREE COLUMN SCOREBOARD INSTALLATION

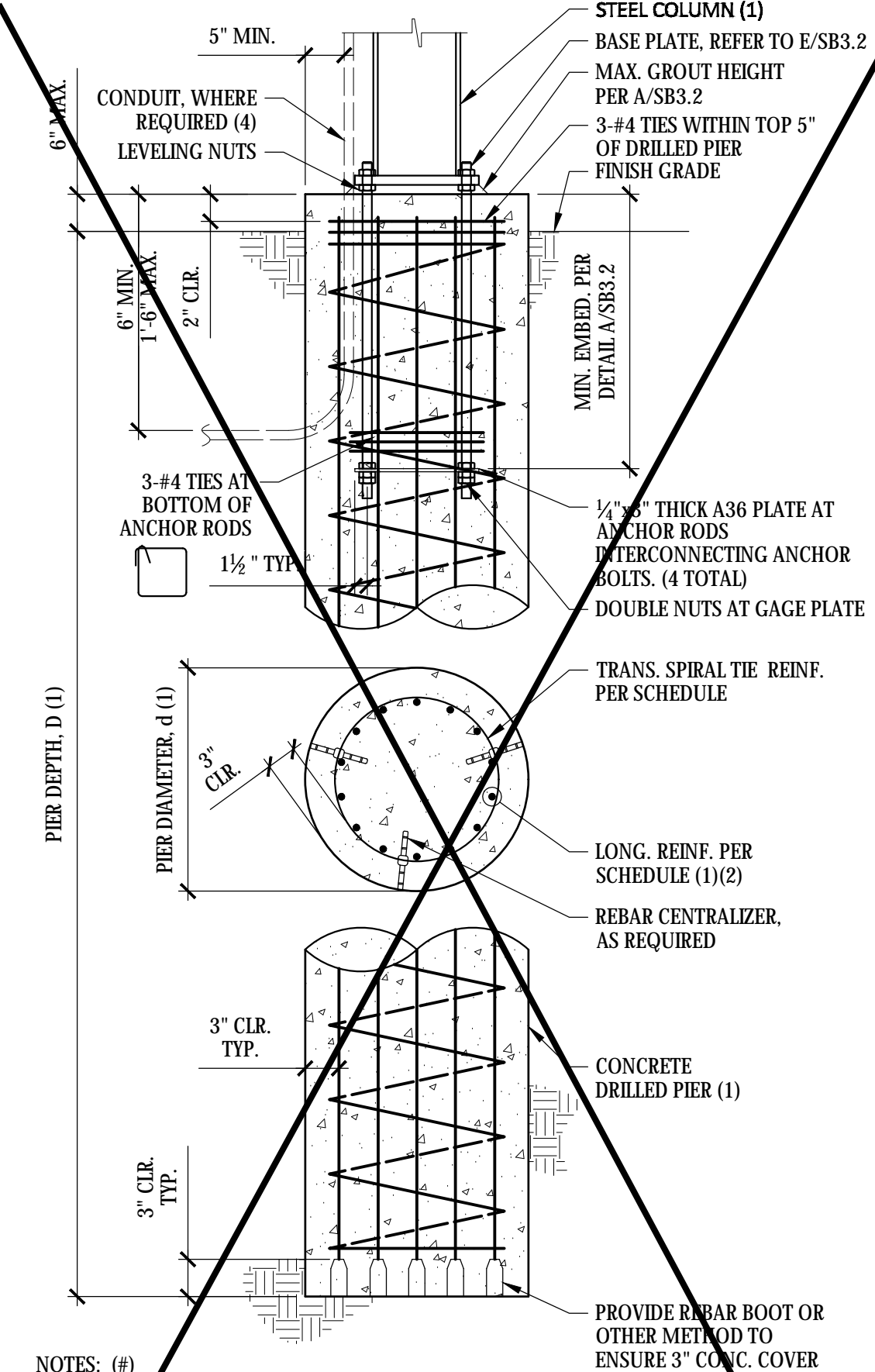
N.T.S.



NOTES: (#)
1. SEE SCOREBOARD ELEVATION, A/SB3.2

BASE PLATE

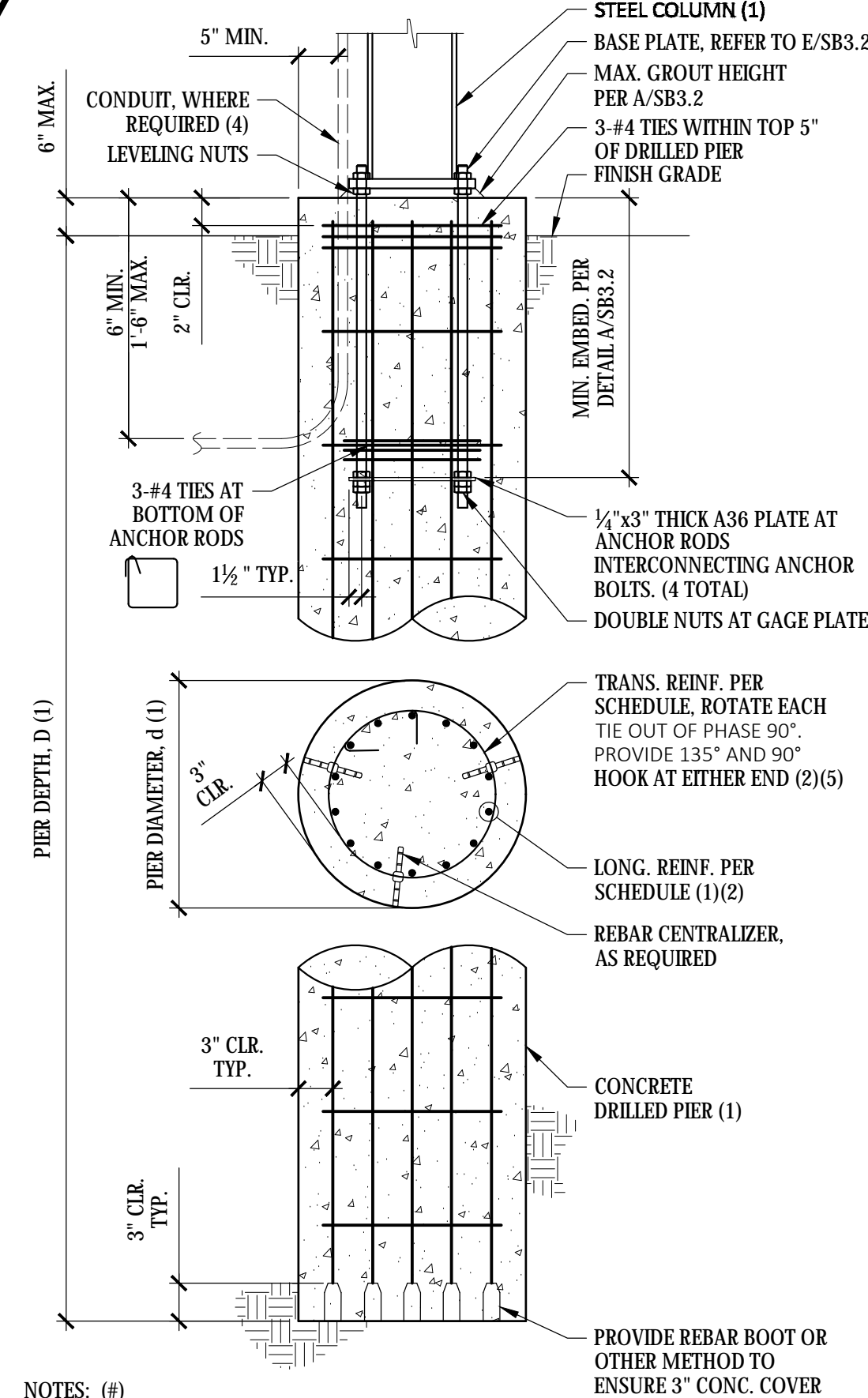
N.T.S.



NOTES: (#)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB3.2
2. SEE DETAILS B/SB3.2 FOR REINFORCEMENT TIE REQUIREMENTS
3. DO NOT SPICE REINFORCEMENT.
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

CONCRETE DRILLED PIER

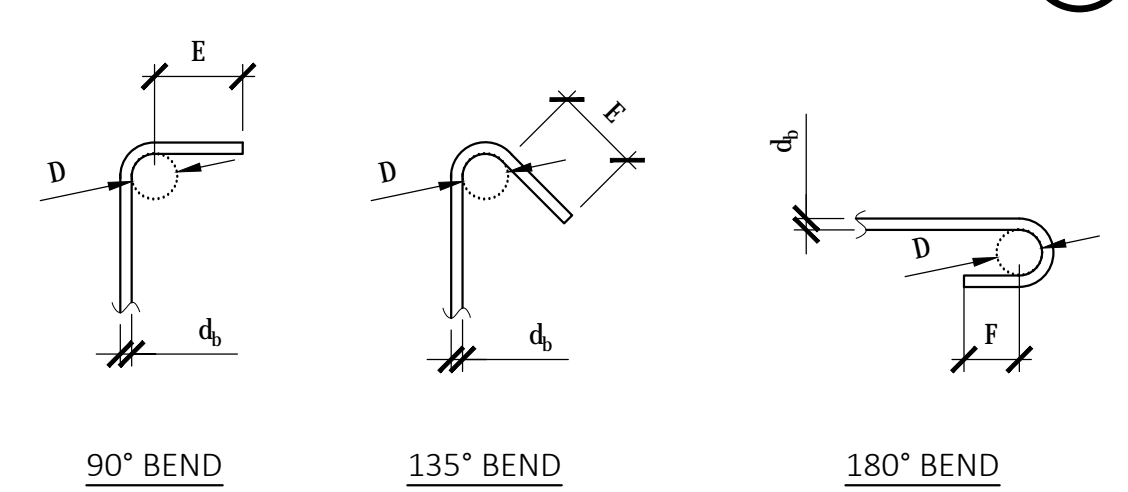
N.T.S.



NOTES: (#)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB3.2
2. SEE DETAILS B/SB3.2 FOR REINFORCEMENT TIE REQUIREMENTS
3. DO NOT SPICE REINFORCEMENT.
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

CONCRETE DRILLED PIER

N.T.S.



Bar Size	Dimension of Standard Bends		
	D	E	F (6d) [3" min.]
#4	2"Ø	3"	3"

TIE AND STIRRUP BENDS

N.T.S.

APPLICATION # 02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC.
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. RANOLD
No. 5405
STATE OF CALIFORNIA
PC SEOR REAL 08.09.2023

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DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application for construction is required.

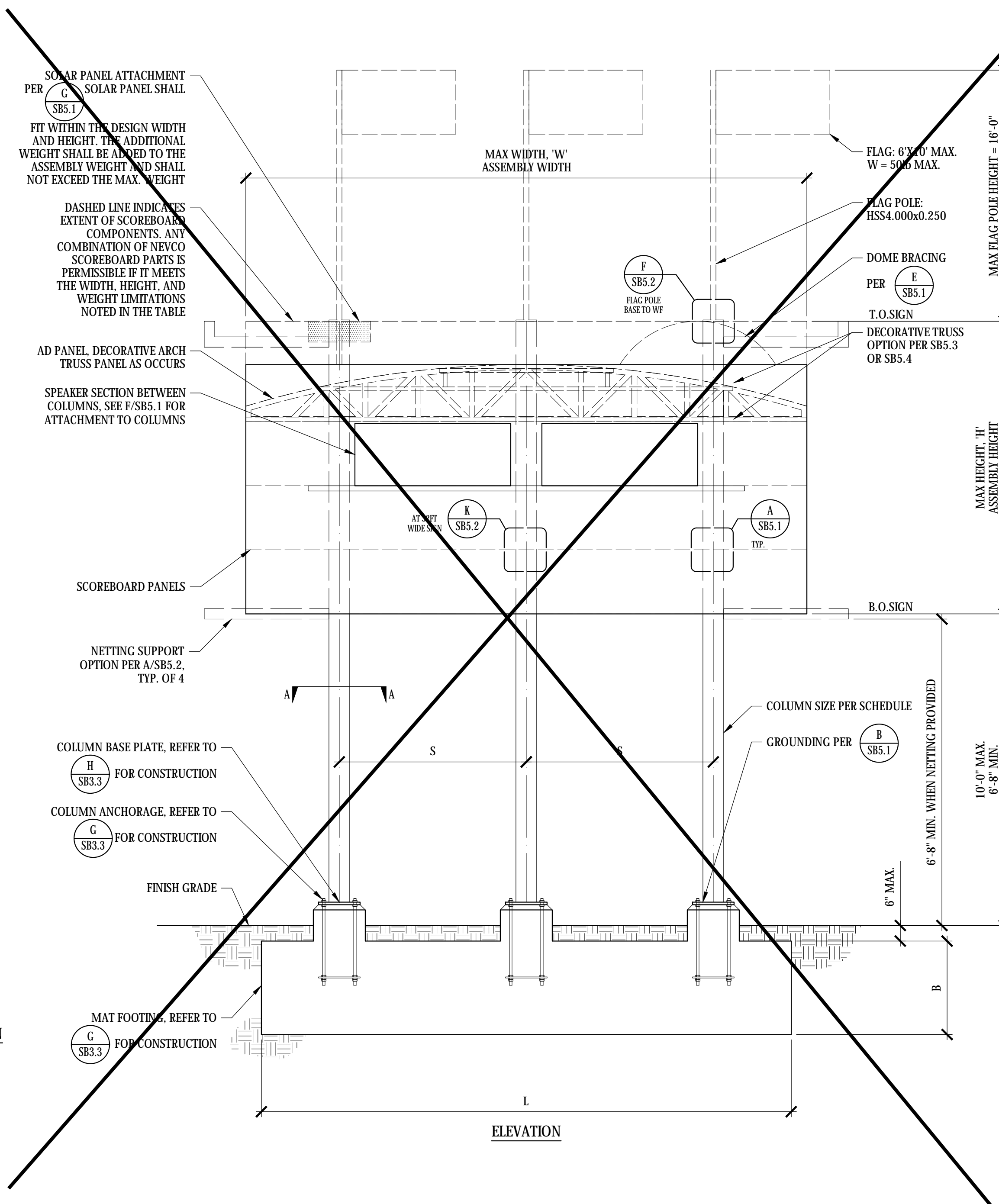
REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. RANOLD
No. 52386
STATE OF CALIFORNIA
PC SEOR REAL 08.09.2023

THREE COLUMN
CAISSON -
BOLTED

SHEET INFORMATION

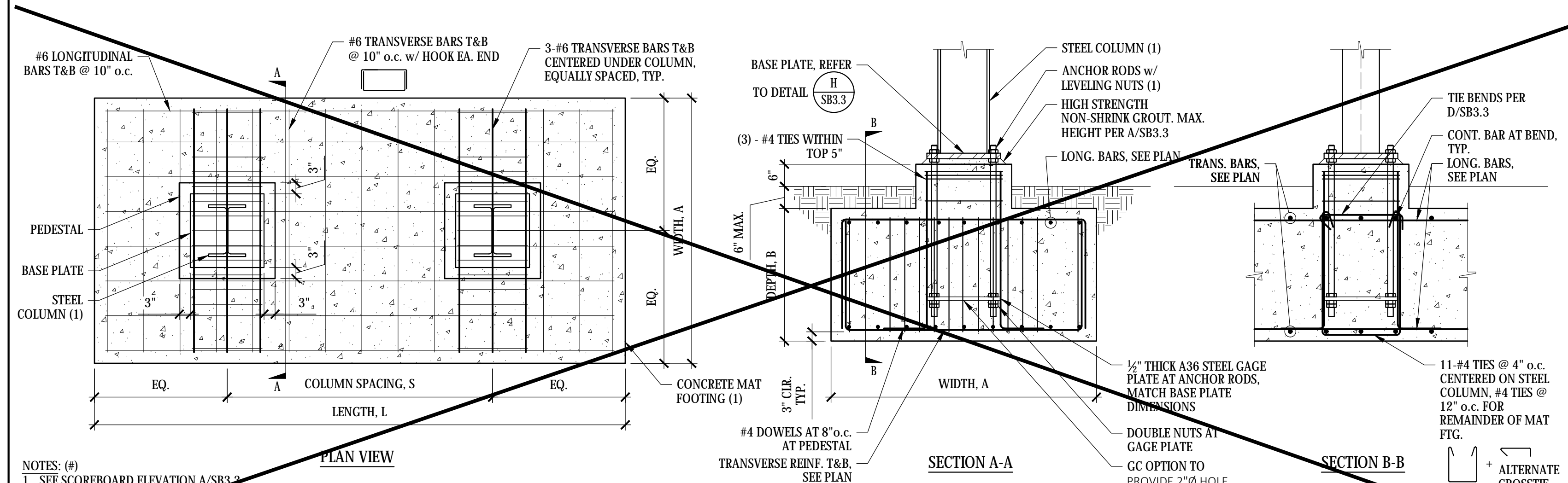
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	SB3.2

THREE COLUMN ASSEMBLY																		
ASSEMBLY CRITERIA						MAT FOOTING CRITERIA				BASE PLATE				ANCHOR RODS				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	8'-0"	3'-0"	20'-0"	1½"	20"	20"	¾"	(4) - 1¼"	F1554 - GR.36	2½"	2"	30"
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	9'-0"	3'-0"	23'-0"	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	11'-0"	3'-0"	24'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		4,800 lbs.	≤ 20'-0"	8'-0"	W16x67	W16x67	12'-0"	3'-0"	27'-0"	1½"	30"	30"	1"	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	14'-0"	3'-0"	27'-0"	1½"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
24'-0"		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	23'-0"	1½"	20"	20"	¾"	(4) - 1¼"	F1554 - GR.55	2½"	2"	30"
		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	9'-0"	3'-0"	26'-0"	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	28'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	12'-0"	3'-0"	30'-0"	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.105	2½"	2"	30"
		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	14'-0"	3'-0"	30'-0"	1½"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
28'-0"		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	15'-0"	3'-0"	30'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1½"	20"	20"	¾"	(4) - 1¼"	F1554 - GR.55	2½"	2"	30"
		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	9'-0"	3'-0"	29'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	30'-0"	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-6"	3'-0"	32'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
32'-0"		8,070 lbs.	≤ 24'-0"	10'-0"	W18x97	W18x97	14'-0"	3'-0"	33'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	15'-0"	3'-0"	33'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.105	3"	2"	30"
		3,080 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-6"	3'-0"	30'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	11'-0"	3'-0"	32'-0"	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.55	3"	2"	30"
	7,080 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	13'-0"	3'-6"	32'-0"	1½"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"	
	9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	14'-0"	4'-0"	33'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	36"	
	10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	15'-0"	4'-0"	33'-0"	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	36"	



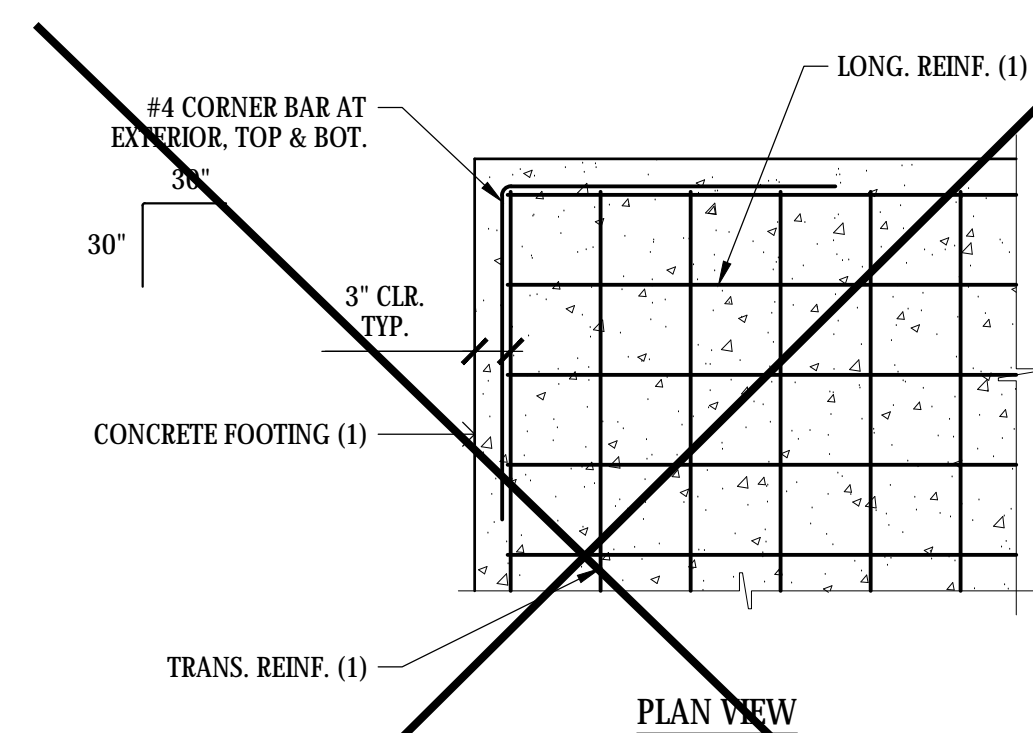
THREE COLUMN SCOREBOARD INSTALLATION

N.T.9



~~1. SUBSCOREBOARD ELEVATION FORM~~

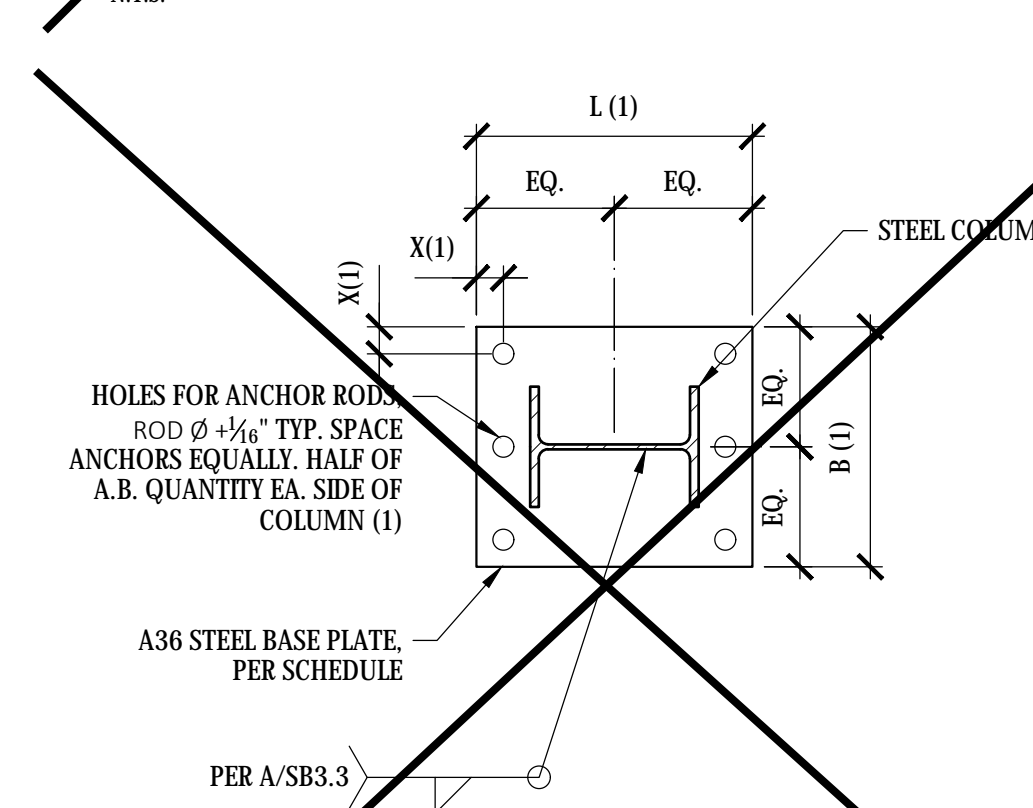
NTS



NOTES: (#)
1 SEE G/SB3.2

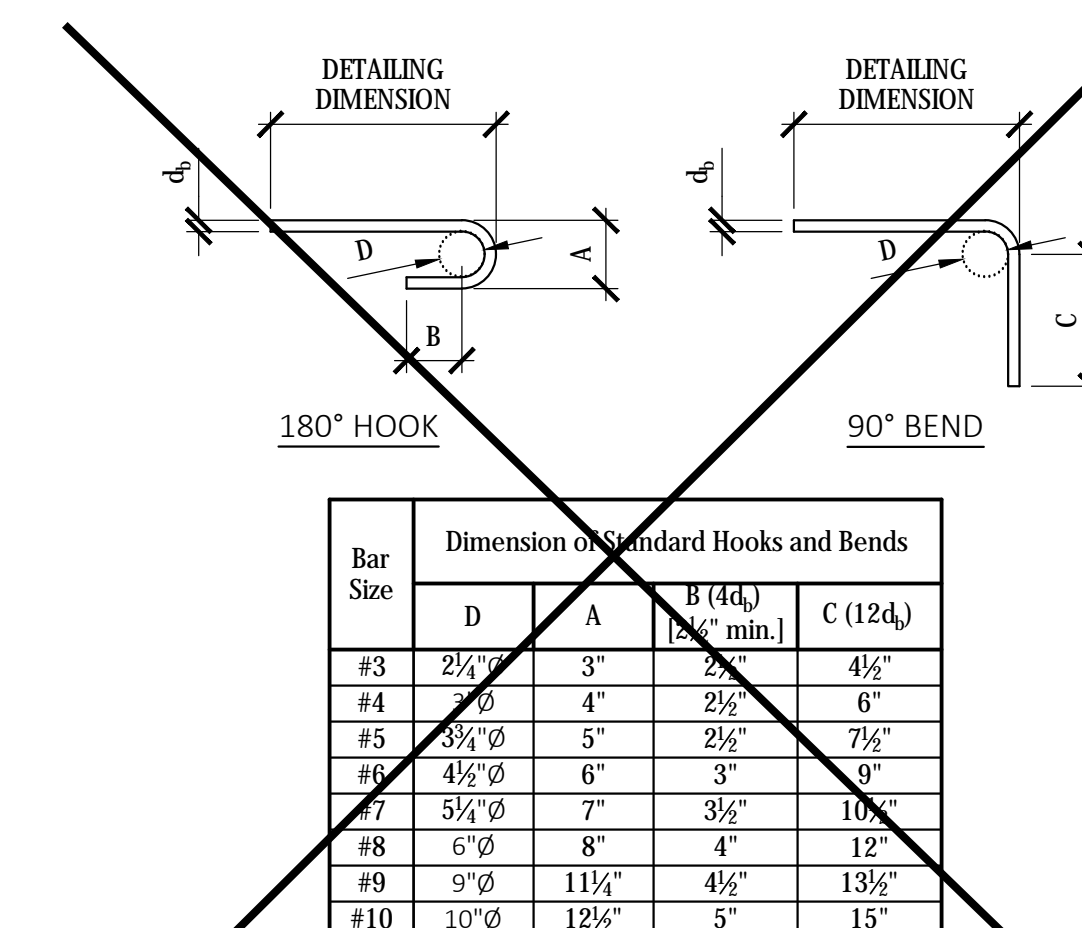
~~TYP. FOOTING CORNER~~

N.T.S.



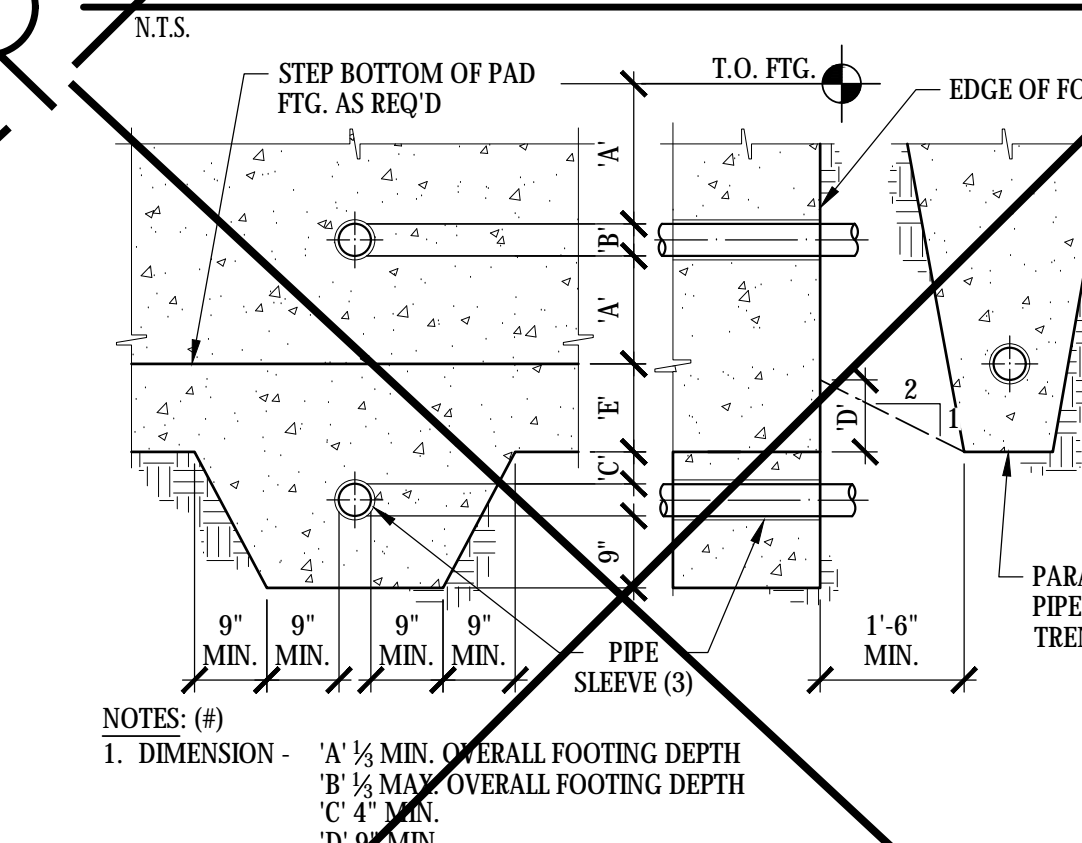
NOTES: (#)
1 SEE SCORING

~~BASE PLATE~~

~~NTS~~

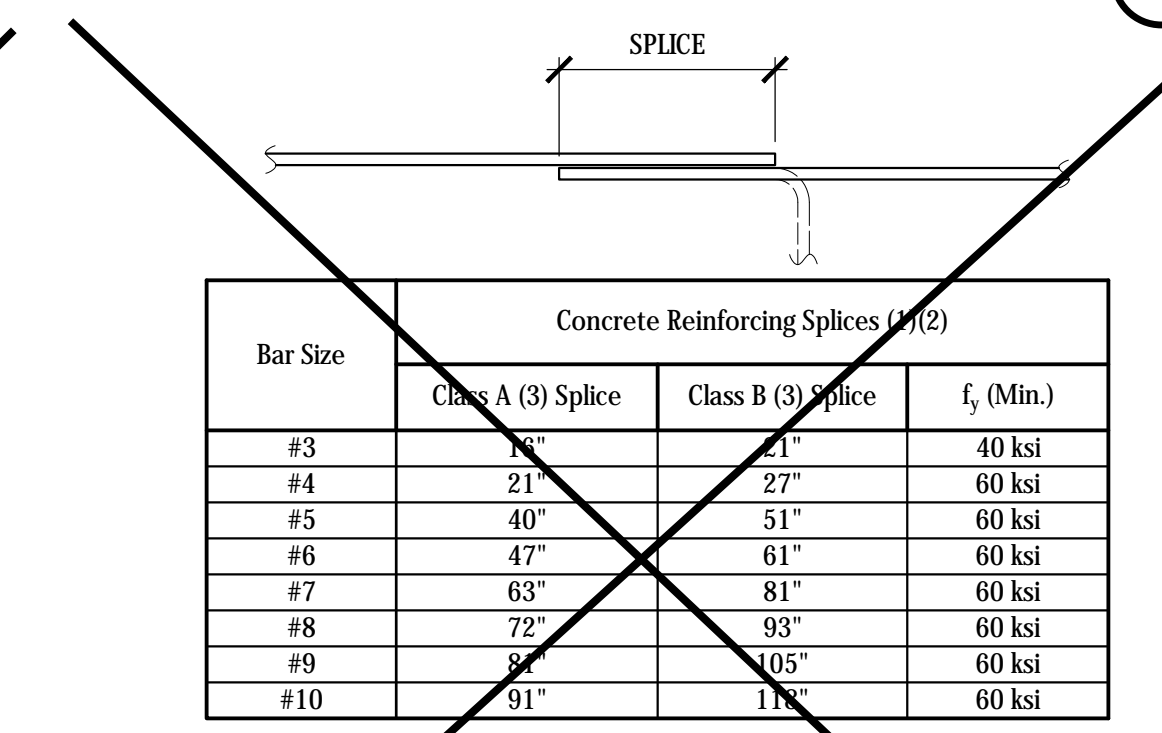
~~REB.~~

~~REBAR HOOKS & BENDS~~



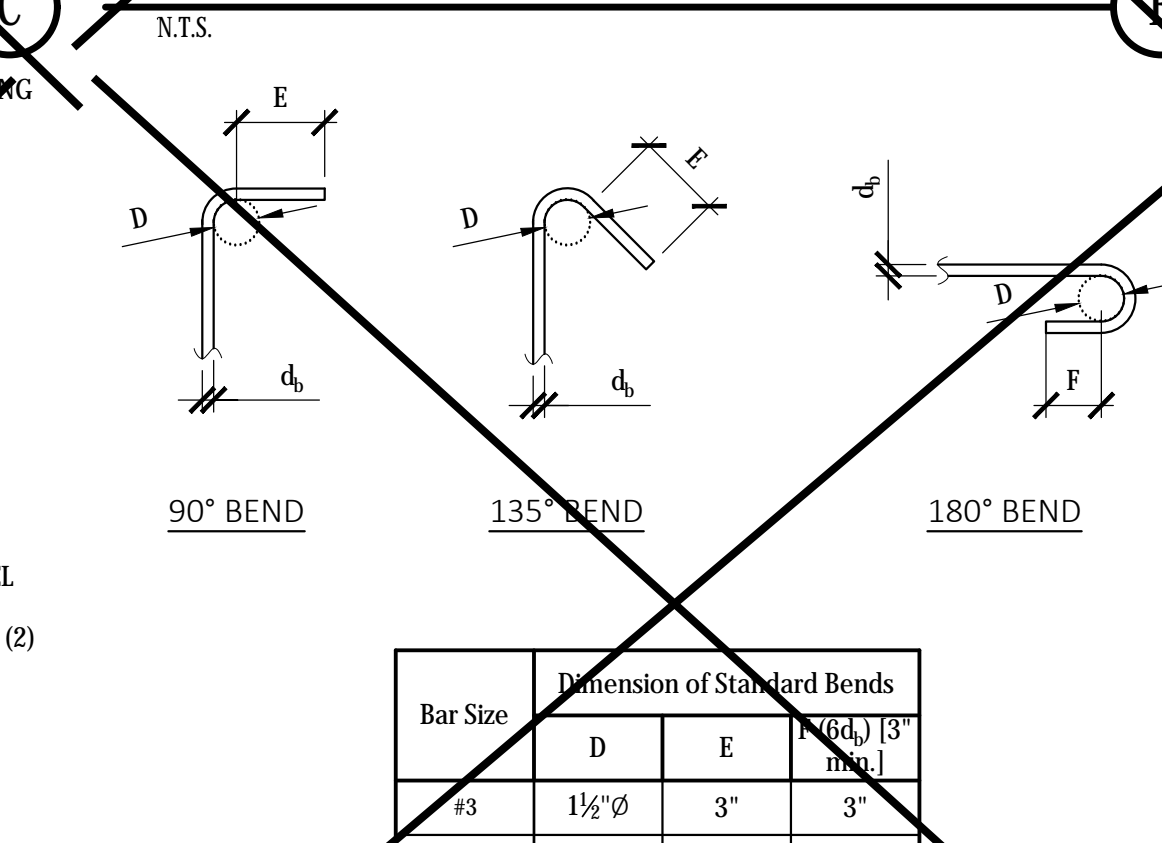
2. PIPE TRENCH FOOTING
3. G.I.S.M.P.

PIPE THROUGH FOOTING

~~CONFIDENTIAL~~

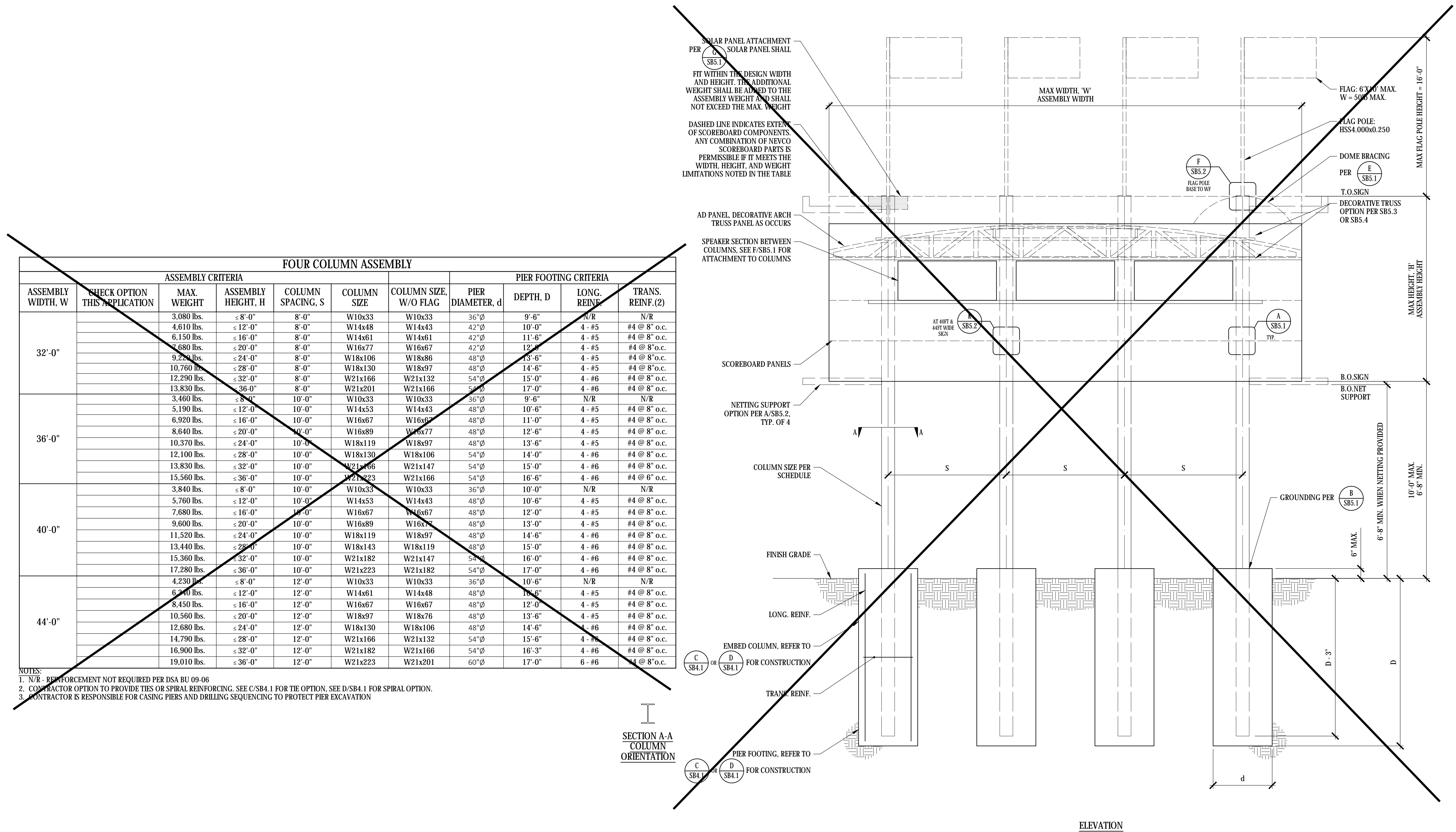
NOTES: (#)
1. LAP LENGTHS LISTED APPLY TO HORIZONTAL REINFORCEMENT
2. WHERE BARS OF A DIFFERENT SIZE ARE LAPPED, THE LAP LENGTH SHALL BE THE LENGTH REQUIRED BY THE LARGER BAR
3. ALL SPLICES SHALL BE CONSIDERED CLASS B UNLESS SPECIFICALLY NOTED OTHERWISE

~~TYPICAL LAP SPLICES~~



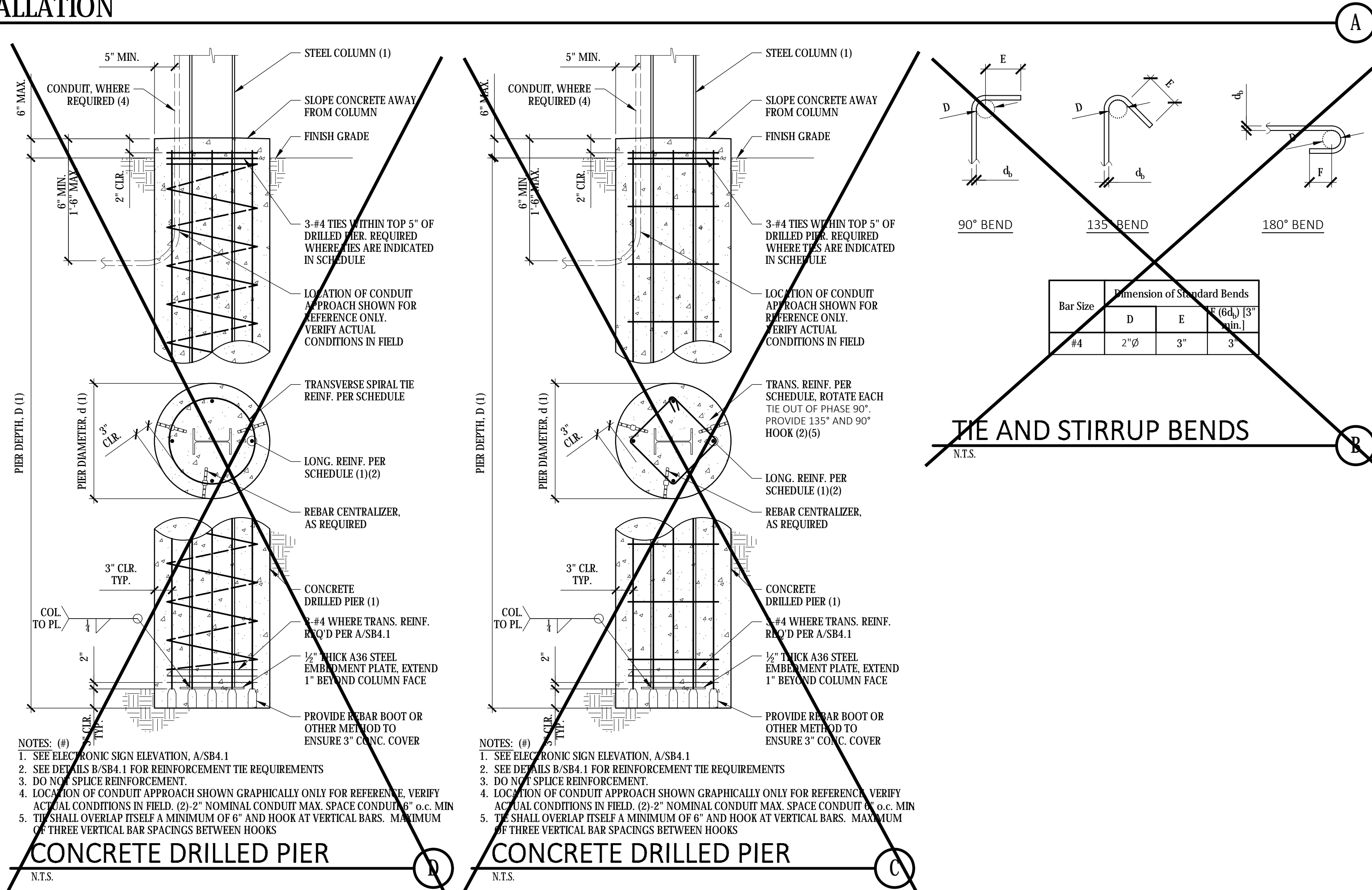
~~TIE AND STIRRUP BENDS~~

7



FOUR COLUMN SCOREBOARD INSTALLATION

N.T.S.



APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC.
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STATE OF CALIFORNIA
PC SEOR SEAL
08.09.2023

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

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SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CALIFORNIA R. RANDOLPH
No. 52366
EXPIRES
3-31-24
STRUCTURAL

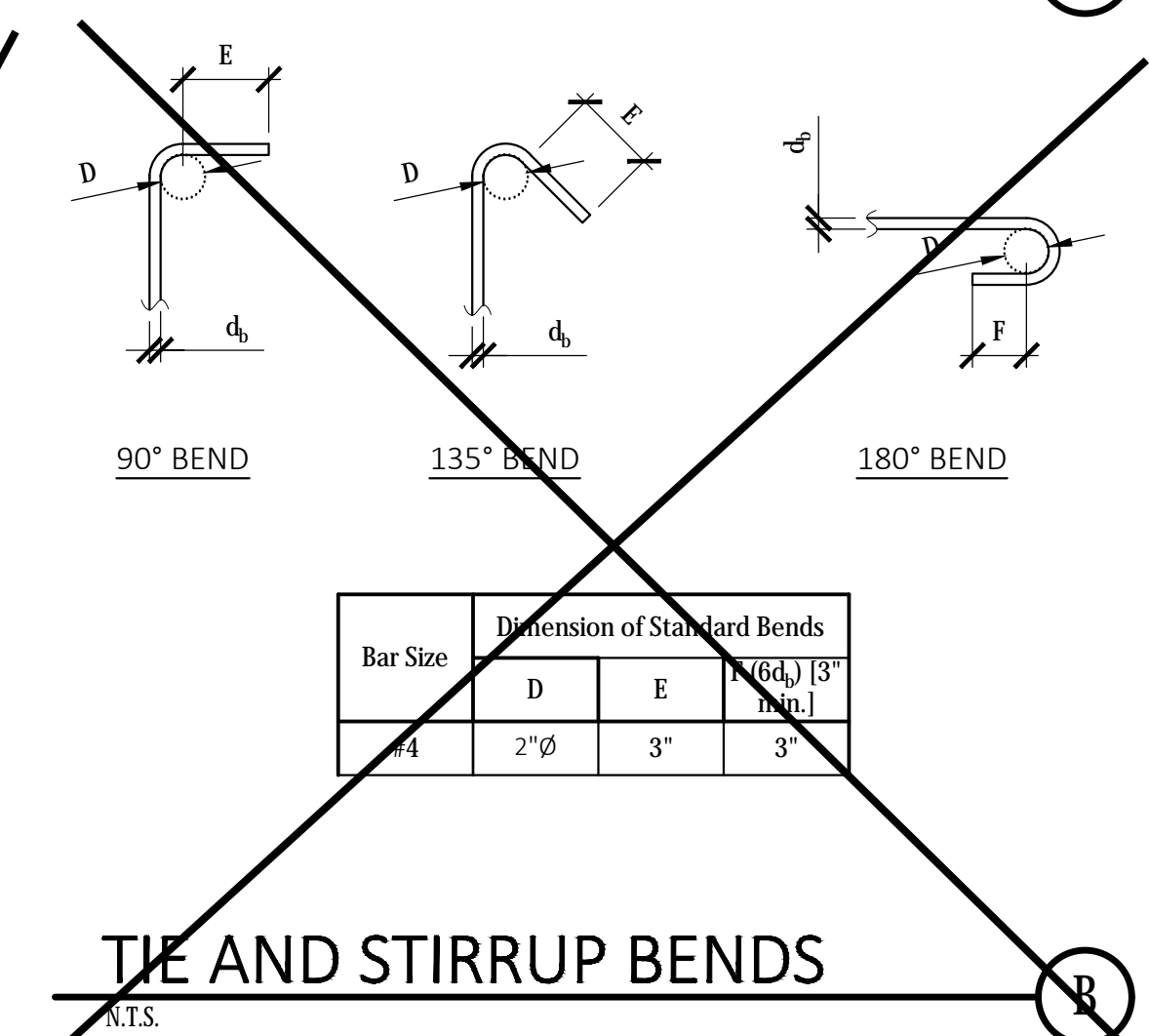
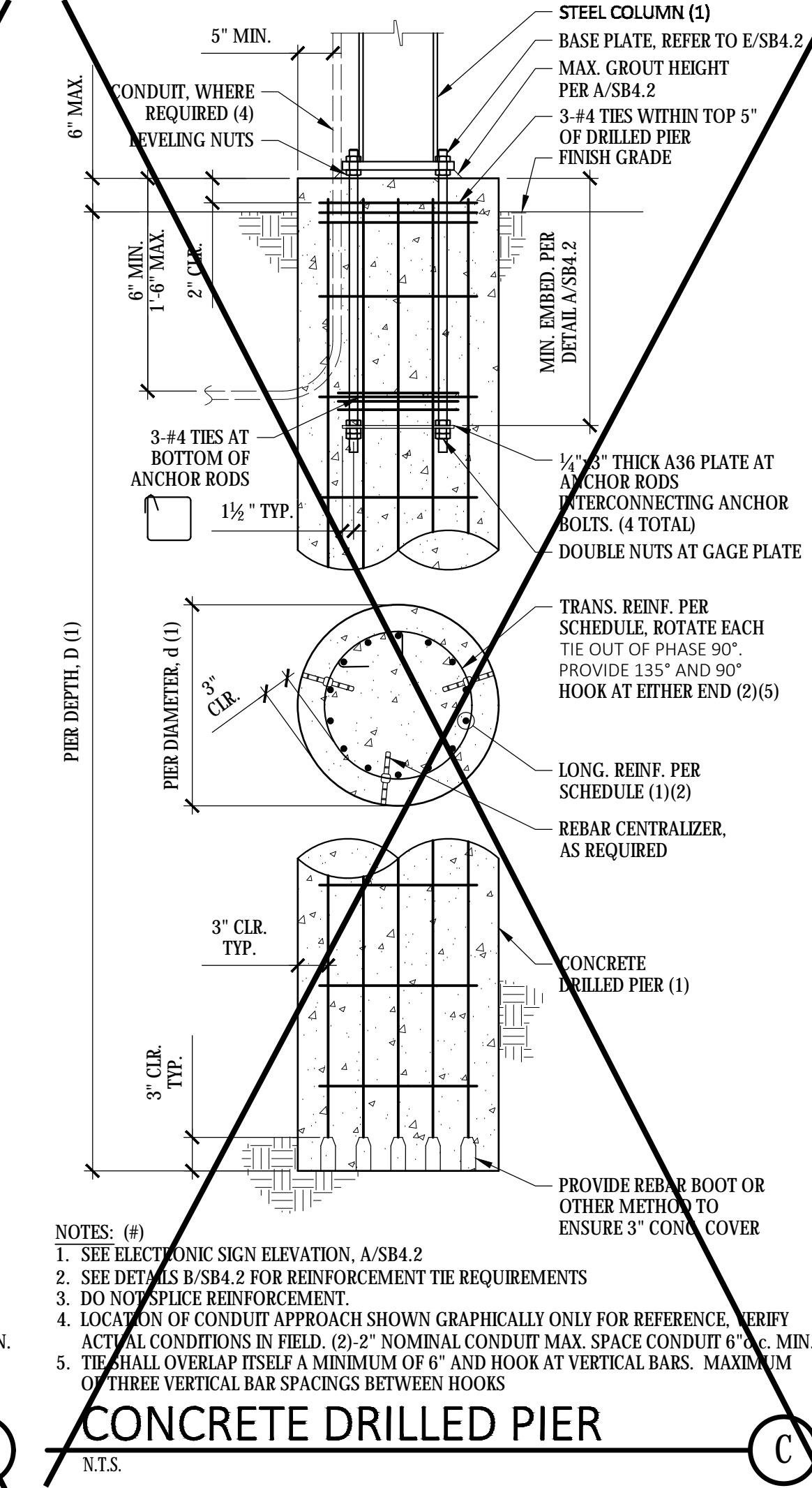
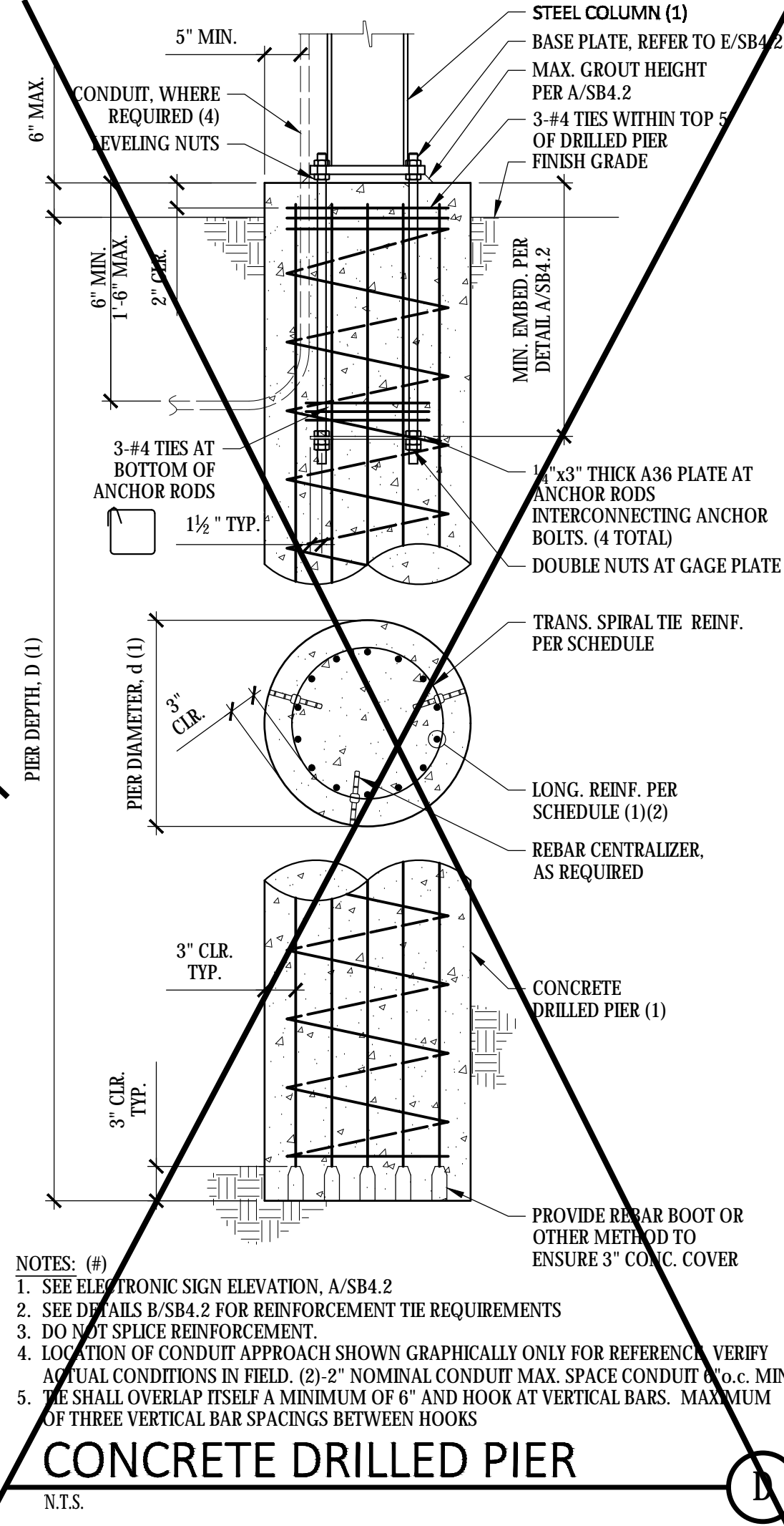
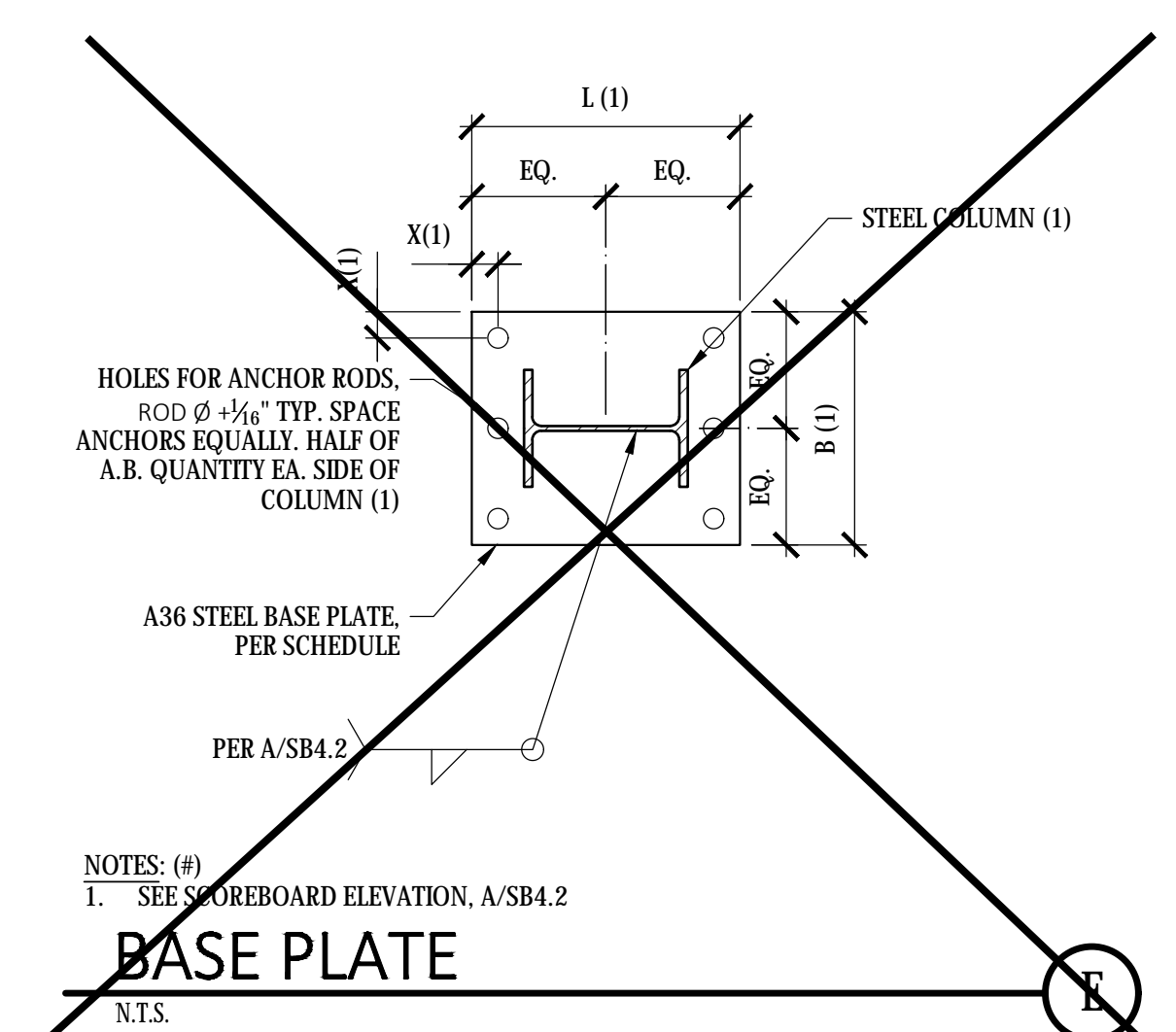
SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB4.1

FOUR COLUMN ASSEMBLY																			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA				PIER FOOTING CRITERIA				BASE PLATE				ANCHOR RODS					
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	CROUT HEIGHT	EMBED
32'-0"		3,080 lbs.	± 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		4,610 lbs.	± 16'-0"	8'-0"	W14x48	W14x43	42"Ø	10'-0"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,150 lbs.	± 16'-0"	8'-0"	W14x61	W14x61	42"Ø	11'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		7,680 lbs.	± 20'-0"	8'-0"	W16x77	W16x67	42"Ø	12'-9"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		9,220 lbs.	± 24'-0"	8'-0"	W18x106	W18x86	48"Ø	13'-6"	14 - #9	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	70"
		10,760 lbs.	± 28'-0"	8'-0"	W18x130	W18x97	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	70"
36'-0"		12,290 lbs.	± 32'-0"	8'-0"	W21x166	W21x132	54"Ø	15'-0"	18 - #9	#4 @ 6" o.c.	2"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		13,830 lbs.	± 36'-0"	8'-0"	W21x201	W21x166	54"Ø	17'-0"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
		3,460 lbs.	± 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,190 lbs.	± 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,920 lbs.	± 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-0"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(4) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		8,640 lbs.	± 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
40'-0"		10,370 lbs.	± 24'-0"	10'-0"	W18x119	W18x97	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		12,100 lbs.	± 28'-0"	10'-0"	W18x130	W18x106	54"Ø	14'-0"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		13,830 lbs.	± 32'-0"	10'-0"	W21x166	W21x147	54"Ø	15'-0"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		15,560 lbs.	± 36'-0"	10'-0"	W21x223	W21x166	54"Ø	16'-6"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		3,840 lbs.	± 8'-0"	10'-0"	W10x33	W10x33	36"Ø	10'-0"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,760 lbs.	± 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
44'-0"		7,680 lbs.	± 16'-0"	10'-0"	W16x67	W16x67	48"Ø	12'-0"	10 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		9,600 lbs.	± 20'-0"	10'-0"	W16x89	W16x77	48"Ø	13'-0"	12 - #8	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		11,520 lbs.	± 24'-0"	10'-0"	W18x119	W18x97	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		13,440 lbs.	± 28'-0"	10'-0"	W18x143	W18x119	48"Ø	15'-0"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	64"
		15,360 lbs.	± 32'-0"	10'-0"	W21x182	W21x147	54"Ø	16'-0"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
		17,280 lbs.	± 36'-0"	10'-0"	W21x223	W21x182	54"Ø	17'-0"	27 - #9	#4 @ 6" o.c.	3"	30"	42"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
44'-0"		4,230 lbs.	± 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,340 lbs.	± 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		8,450 lbs.	± 16'-0"	12'-0"	W16x67	W16x67	48"Ø	12'-0"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		10,560 lbs.	± 20'-0"	12'-0"	W18x97	W18x76	48"Ø	13'-6"	12 - #8	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		12,680 lbs.	± 24'-0"	12'-0"	W18x130	W18x106	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		14,790 lbs.	± 28'-0"	12'-0"	W21x166	W21x132	54"Ø	15'-6"	18 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
44'-0"		16,900 lbs.	± 32'-0"	12'-0"	W21x182	W21x166	54"Ø	16'-3"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
		19,040 lbs.	± 36'-0"	12'-0"	W21x223	W21x201	60"Ø	17'-0"	27 - #9	#4 @ 6" o.c.	3"	30"	42"	CIP	(10) - 2"Ø	F1554 - GR.105	4"	2½"	70"

- NOTES: (1) CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB4.2 FOR TIE OPTION. SEE D/SB4.2 FOR SPIRAL OPTION.
(2) CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

FOUR COLUMN SCOREBOARD INSTALLATION

NTS



APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC.
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STRUCTURAL
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC SEOR REAL

THESE DRAWINGS, NOTES AND DETAILS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF SSG STRUCTURAL ENGINEERS, LLP. ALL DRAWINGS, INFORMATION, SPECIFICATIONS, REVISIONS, AND AMENDMENTS REPRESENTED HEREIN, INCLUDING ANY CORRECTIONS, SHALL REMAIN THE PROPERTY OF THE ENGINEER. NO PART THEREOF SHALL BE REPRODUCED, COPIED, OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED AND DEVELOPED WITHOUT THE EXPRESS WRITTEN CONSENT OF THE ENGINEER. CONTACT: 301 East Harris Avenue, Greenville, Illinois 62246
Phone: (618) 664-0960
www.newco.com

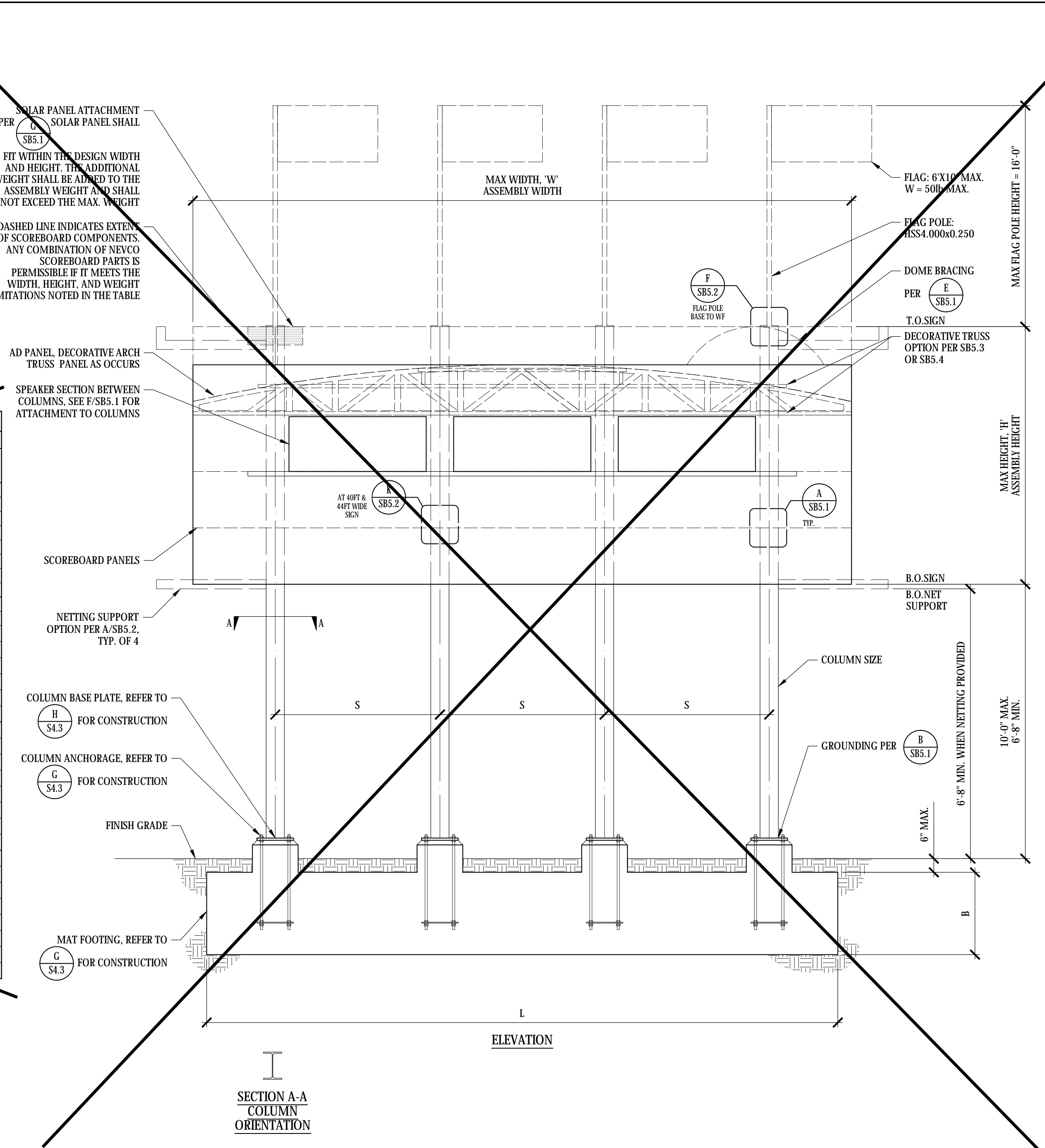
APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐ CG ☐
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022
A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CALIFORNIA R. RANDOLPH
No. 52366
STRUCTURAL
DATE SIGNED: 03-31-24

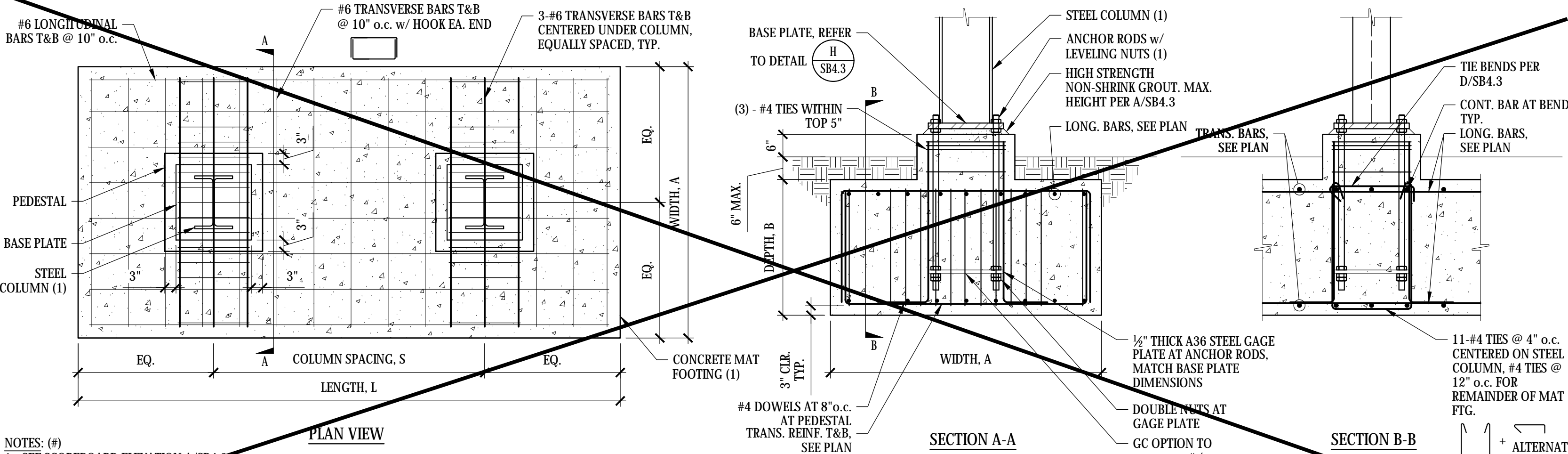
SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB4.2

FOUR COLUMN ASSEMBLY																		
ASSEMBLY CRITERIA						MAT FOOTING CRITERIA				BASE PLATE			ANCHOR RODS					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
32'-0"		3,080 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1 1/4"	20"	20"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		4,610 lbs.	≤ 8'-0"	8'-0"	W14x48	W14x43	9'-0"	3'-0"	30'-0"	1 1/4"	24"	24"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		6,150 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	35'-0"	1 1/2"	24"	24"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		7,680 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	12'-6"	3'-0"	36'-0"	1 1/4"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		9,220 lbs.	≤ 24'-0"	8'-0"	W18x106	W18x86	14'-0"	3'-0"	39'-0"	1 1/4"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x97	15'-0"	3'-6"	39'-0"	2"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.105	3"	2"	30"
36'-0"		12,290 lbs.	≤ 32'-0"	8'-0"	W21x196	W21x132	17'-0"	4'-0"	42'-0"	2"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	18'-0"	4'-0"	46'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	7'-6"	3'-0"	36'-0"	1 1/4"	20"	20"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	36'-0"	1 1/4"	24"	24"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	36'-0"	1 1/4"	24"	30"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		8,640 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-0"	3'-0"	40'-0"	1 1/4"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
40'-0"		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	1 1/4"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	14'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	16'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	18'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CJP	(8) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	36'-0"	1 1/2"	20"	20"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
44'-0"		7,680 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		9,600 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	13'-0"	3'-0"	40'-0"	2"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	2"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.105	3"	2"	30"
		13,440 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	14'-0"	4'-0"	42'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	16'-0"	4'-0"	44'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	18'-0"	4'-0"	46'-0"	3"	30"	42"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	7'-0"	3'-0"	42'-0"	1 1/2"	20"	20"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		6,340 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-0"	3'-0"	43'-0"	1 1/2"	24"	24"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/4"	2"	30"
		8,450 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	10'-0"	3'-0"	45'-0"	1 1/4"	24"	30"	3/16	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	12'-0"	3'-6"	46'-0"	1 1/4"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	14'-0"	3'-6"	47'-0"	2"	24"	30"	CJP	(6) - 1 1/2"Ø	F1554 - GR.105	3"	2"	30"
		14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	15'-0"	4'-0"	48'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	30"
		16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	16'-0"	4'-0"	49'-0"	2 1/4"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	18'-0"	4'-0"	52'-0"	3"	30"	42"	CJP	(10) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"



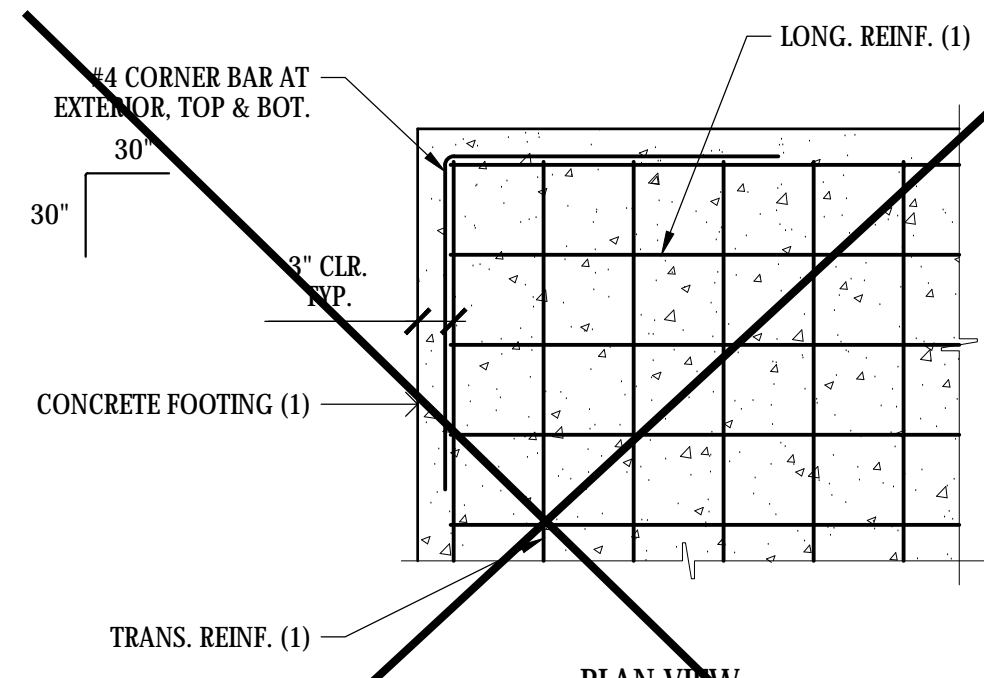
FOUR COLUMN SCOREBOARD INSTALLATION

NTS.



MAT FOOTING CONSTRUCTION AND ANCHORAGE

NTS.

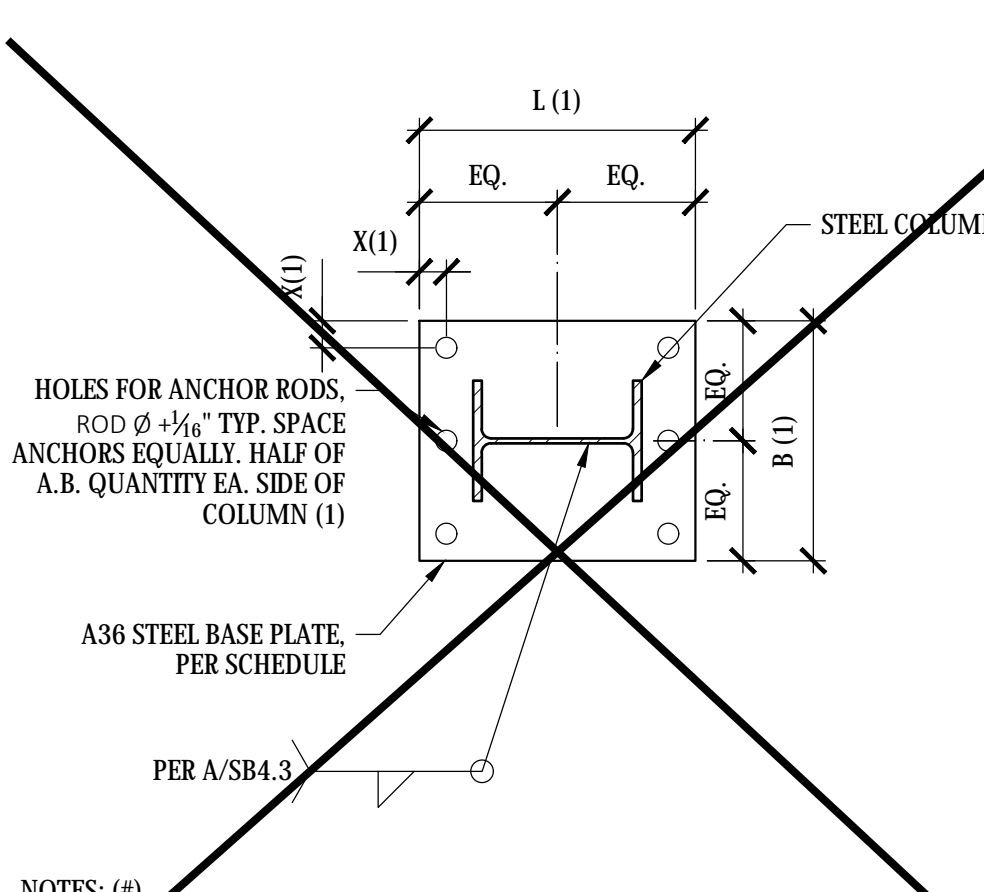


TYP. FOOTING CORNER

NTS.

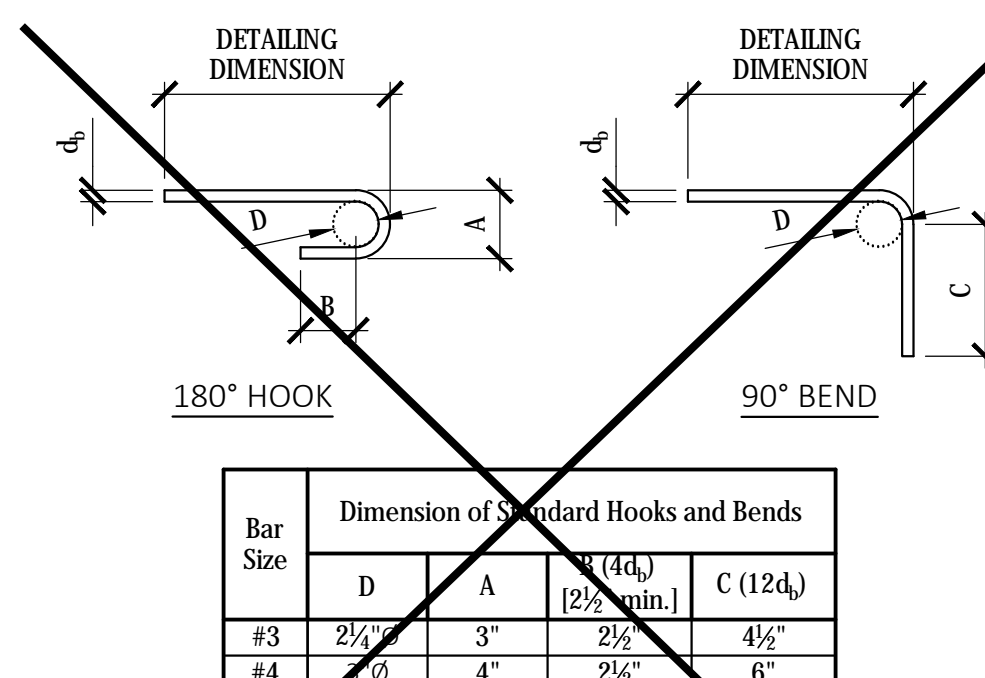
BASE PLATE

NTS.



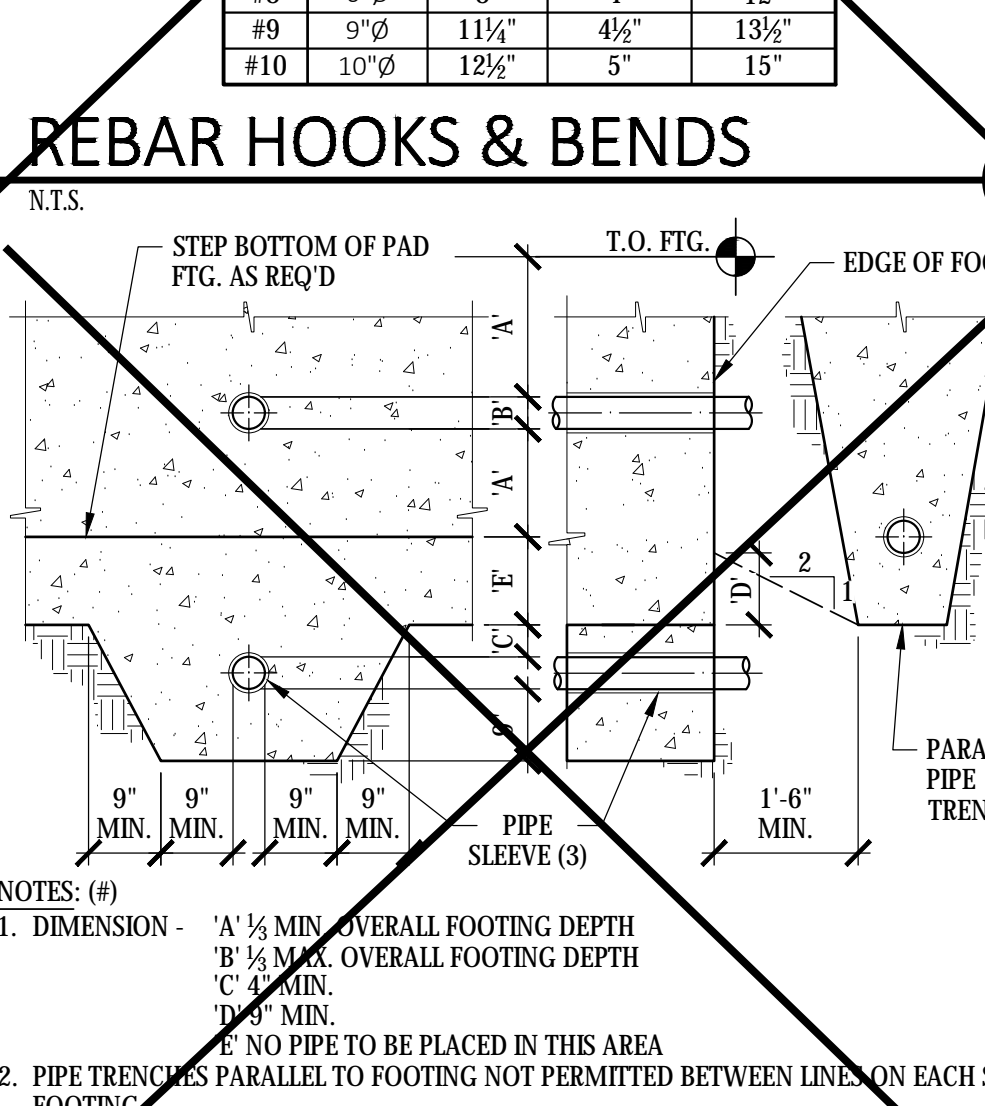
PIPE THROUGH FOOTING

NTS.



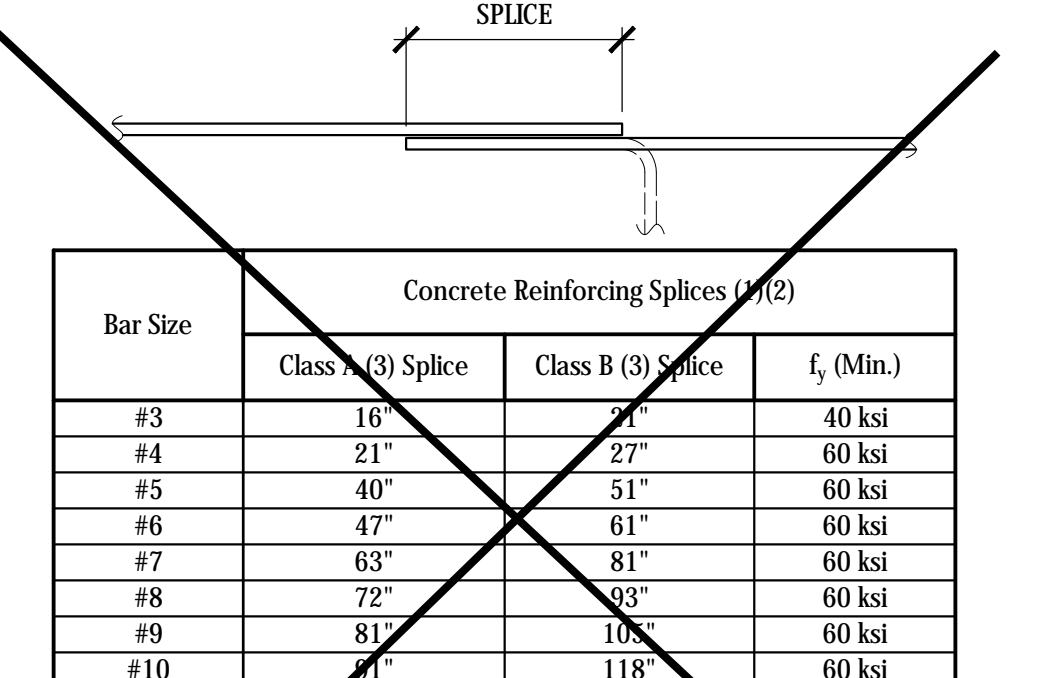
REBAR HOOKS & BENDS

NTS.



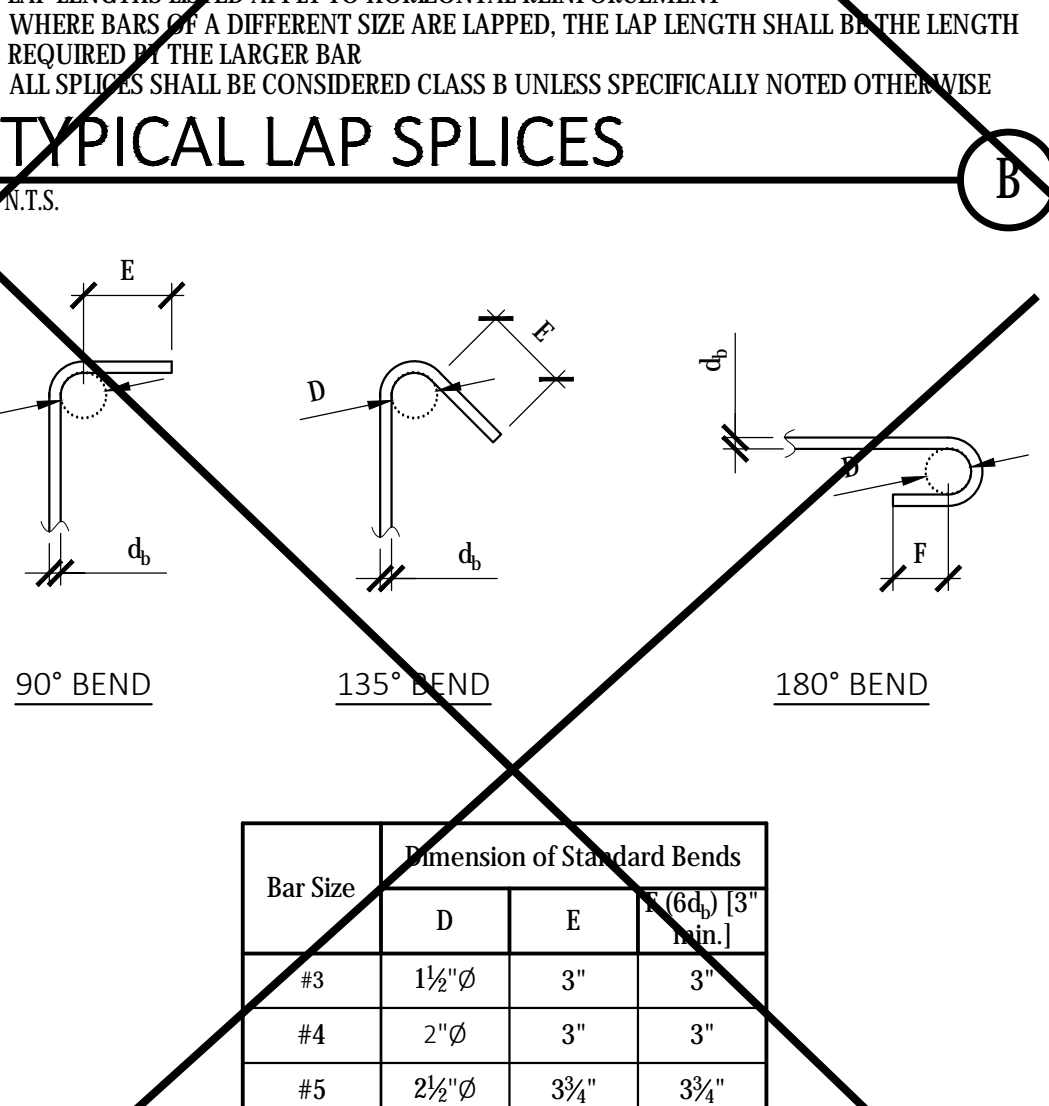
PIPE THROUGH FOOTING

NTS.



TYPICAL LAP SPLICES

NTS.



TIE AND STIRRUP BENDS

NTS.

APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC.
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STATE OF CALIFORNIA
DATE REISSUED: 08.09.2023

PC SEOR SEAL

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

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DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐ CG ☐
DATE: 09/20/2023

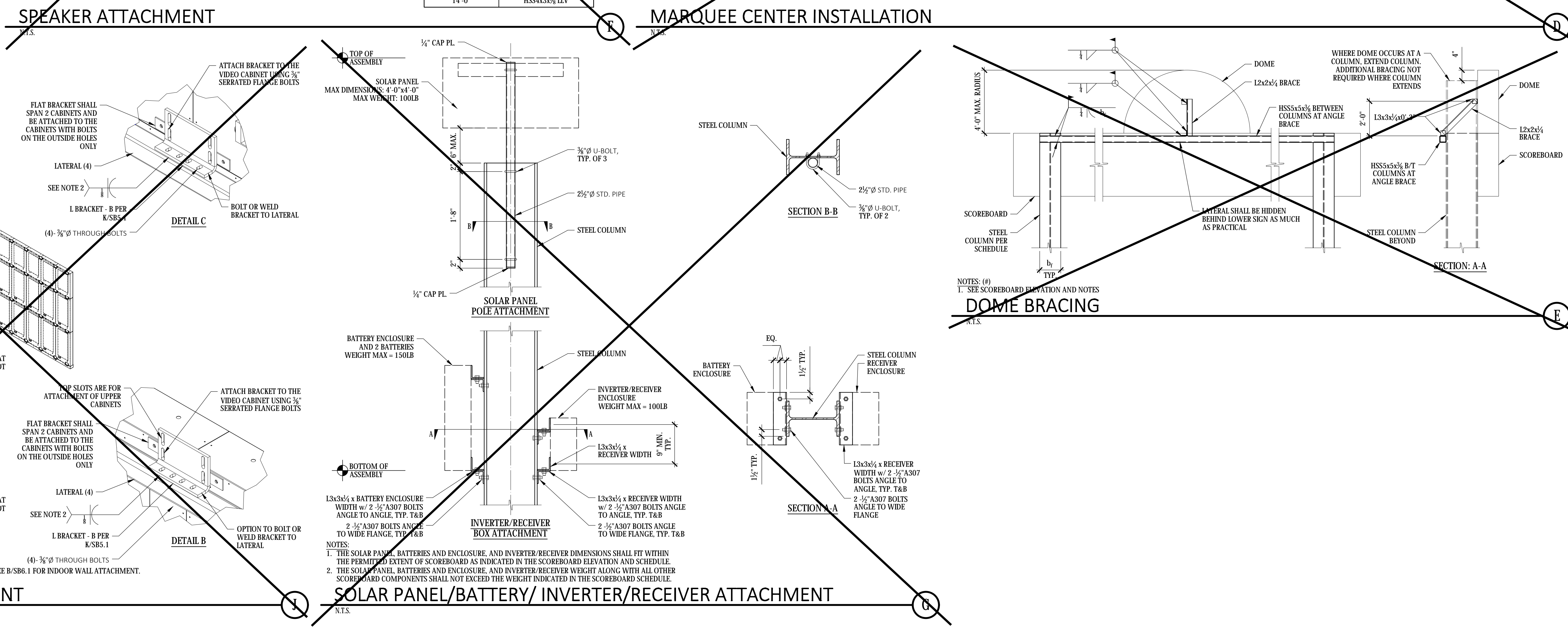
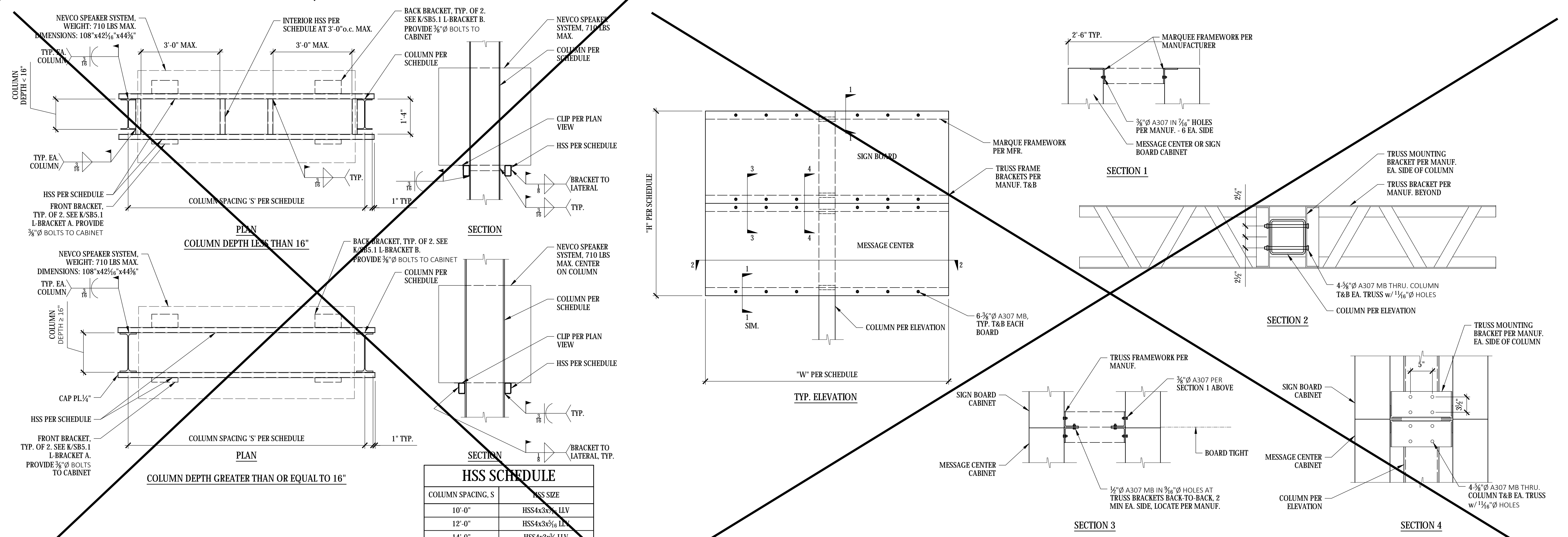
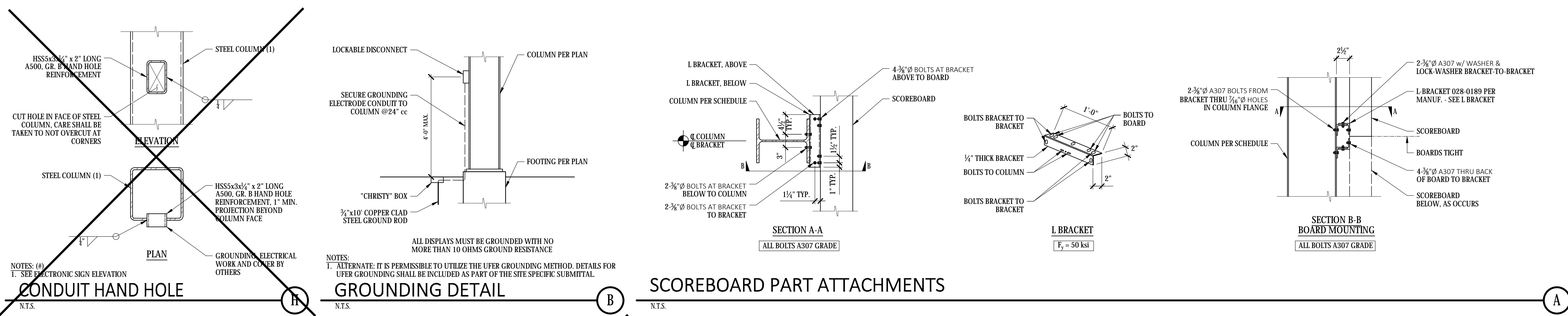
DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 523386
STATE OF CALIFORNIA
DATE REISSUED: 3-31-24

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB4.3



APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

structural engineers

MICHAEL E. PARRY
No. 5405
STRUCTURAL
STATE OF CALIFORNIA
08.09.2023

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SS ☒ FLS ☒ ACS ☐ CG ☐
DATE: 09/20/2023

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PRE-CHECK (PC) DOCUMENT
CODE: 2022
A separate project application
for construction is required.

R. RANSOM
No. 52366
STRUCTURAL
STATE OF CALIFORNIA
3-31-24

ATTACHMENT
DETAILS

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB5.1

INSTALLATION OF ALUMINUM TRUSS ON FOUR COLUMNS

BRACKET ORIENTATION

* DIMENSION SHOWN IS TYPICAL. SLOTS IN BRACKETS ALLOW FOR APPROXIMATELY 1/2" VARIATION

CL. SPLICE

TYP. SPLICE: 1 CHANNEL EA. SIDE OF SPLICE

RIVETS PER MFR.

SCREWS PER MFR. AT SPLICE

ALUMINUM SHEET EA. SIDE T = 0.05"

TOP CHORD: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

BOTTOM CHORD: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

ALUMINUM SHEET EA. SIDE T = 0.05"

TYP. WEB: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

Variation Chart

Truss Width (a)	Truss Height (b)	Bracket Height (c)	Bracket Height (d)	Column Spacing (e)	Est Weight
32'	4'	32 1/4"	42 7/8"	8'	294
36'	4'	15 1/2"	39 9/16"	10'	331

CL. SPLICE

NEVCO ALUMINUM TRUSS

CL. SPLICE

CLIP CONNECTION T&B AT EA. COLUMN

COLUMN PER ELEVATION TYP. OF 4

FRONT VIEW

NOTES:

1. DESIGN WIND SPEED FOR TRUSS AND CONNECTIONS = 100 MPH
2. DECORATIVE TRUSS AND COMPONENTS HAVE BEEN DESIGNED PER THE 2022 CBC.
3. DECORATIVE TRUSS AND COMPONENTS ARE NOT PART OF DSA TESTING AND INSPECTION REVIEW
4. T = MINIMUM ALUMINUM SHEET THICKNESS, $F_{TY,MIN} = 19$ ksi

INSTALLATION OF ALUMINUM TRUSS ON FOUR COLUMNS

INSTALLATION OF ALUMINUM TRUSS ON FOUR COL. w/ LATERAL

BRACKET ORIENTATION

CLIP CONNECTION PER A/SS.1

CL. SPLICE

NEVCO ALUMINUM TRUSS

CL. SPLICE

CLIP CONNECTION T&B AT EA. COLUMN

COLUMN PER ELEVATION TYP. OF 4

FRONT VIEW

NOTES:

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INSTALLATION OF ALUMINUM TRUSS ON FOUR COL. w/ LATERAL

INSTALLATION OF ALUMINUM TRUSS ON THREE COLUMNS

BRACKET ORIENTATION

* DIMENSION SHOWN IS TYPICAL. SLOTS IN BRACKETS ALLOW FOR APPROXIMATELY 1/2" VARIATION

CL. SPLICE

TYP. SPLICE: 1 CHANNEL EA. SIDE OF SPLICE

RIVETS PER MFR.

SCREWS PER MFR. AT SPLICE

ALUMINUM SHEET EA. SIDE T = 0.05"

TOP CHORD: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

BOTTOM CHORD: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

ALUMINUM SHEET EA. SIDE T = 0.05"

TYP. WEB: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

Variation Chart

Truss Width (a)	Truss Height (b)	Bracket Height (c)	Bracket Height (d)	Column Spacing (e)	Est Weight
28'	4'	38"	43 1/2"	10'	257
24'	4'	32 1/4"	42 7/8"	8'	220
24'	3'	19 15/16"	30 15/32"	8'	185
20'	4'	25 5/8"	44 3/4"	8'	110

CL. SPLICE

NEVCO ALUMINUM TRUSS

CL. SPLICE

CLIP CONNECTION T&B AT EA. COLUMN

COLUMN PER ELEVATION TYP. OF 3

FRONT VIEW

NOTES:

1. DESIGN WIND SPEED FOR TRUSS AND CONNECTIONS = 100 MPH
2. DECORATIVE TRUSS AND COMPONENTS HAVE BEEN DESIGNED PER THE 2022 CBC.
3. DECORATIVE TRUSS AND COMPONENTS ARE NOT PART OF DSA TESTING AND INSPECTION REVIEW
4. T = MINIMUM ALUMINUM SHEET THICKNESS, $F_{TY,MIN} = 19$ ksi

INSTALLATION OF ALUMINUM TRUSS ON THREE COLUMNS

INSTALLATION OF ALUMINUM TRUSS ON THREE COL. w/ LATERAL

BRACKET ORIENTATION

CLIP CONNECTION PER A/SS.1

CL. SPLICE

NEVCO ALUMINUM TRUSS

CL. SPLICE

CLIP CONNECTION T&B AT EA. COLUMN

COLUMN PER ELEVATION TYP. OF 3

FRONT VIEW

NOTES:

1. DESIGN WIND SPEED FOR TRUSS AND CONNECTIONS = 100 MPH
2. DECORATIVE TRUSS AND COMPONENTS HAVE BEEN DESIGNED PER THE 2022 CBC.
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INSTALLATION OF ALUMINUM TRUSS ON THREE COL. w/ LATERAL

INSTALLATION OF ALUMINUM TRUSS ON TWO COLUMNS

BRACKET ORIENTATION

* DIMENSION SHOWN IS TYPICAL. SLOTS IN BRACKETS ALLOW FOR APPROXIMATELY 1/2" VARIATION

CL. SPLICE

TYP. SPLICE: 1 CHANNEL EA. SIDE OF SPLICE

RIVETS PER MFR.

SCREWS PER MFR. AT SPLICE

ALUMINUM SHEET EA. SIDE T = 0.05"

TOP CHORD: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

BOTTOM CHORD: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

ALUMINUM SHEET EA. SIDE T = 0.05"

TYP. WEB: 2 CHANNELS - TOE TO TOE WITH LAP T = 0.05"

Variation Chart

Truss Width (a)	Truss Height (b)	Bracket Height (c)	Bracket Height (d)	Column Width (e)	Est Weight
18'	3'	28 13/32"	3 7/32"	10"	145
16'	4'	34 1/2"	2"	8"	136
18'	4'	37 1/8"	2"	10"	165
12'	2'	17 3/4"	2"	8"	90
10'	2'	18 7/8"	2"	8"	64
8'	2'	18 7/8"	2"	6"	58
14'	2'	18 1/8"	2"	8"	78
11'	2'	20 3/4"	2"	8"	77
16'	4'	40"	2"	8"	136
18'	2'	17 1/4"	2"	10"	111

CL. SPLICE

NEVCO ALUMINUM TRUSS

CL. SPLICE

CLIP CONNECTION T&B AT EA. COLUMN

COLUMN PER ELEVATION TYP. OF 2

FRONT VIEW

NOTES:

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INSTALLATION OF ALUMINUM TRUSS ON TWO COLUMNS

INSTALLATION OF ALUMINUM TRUSS ON TWO COL. w/ LATERAL

BRACKET ORIENTATION

CLIP CONNECTION PER A/SS.1

CL. SPLICE

NEVCO ALUMINUM TRUSS

CL. SPLICE

CLIP CONNECTION T&B AT EA. COLUMN

COLUMN PER ELEVATION TYP. OF 2

FRONT VIEW

NOTES:

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INSTALLATION OF ALUMINUM TRUSS ON TWO COL. w/ LATERAL

DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS

DETAIL INFORMATION

DATE: 08.09.2023

DRAWN: JMK

CHECKED: MEP

SSG JOB #: S23109

SHEET: SB5.3

APPLICATION # 02-122088

IDENTIFICATION STAMP

DIV. OF THE STATE ARCHITECT

APP: 02-122088 INC.

REVIEWED FOR

SS ☒ FLS ☒ ACS ☐

DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER

NO. 5405

STATE OF CALIFORNIA

PC SEOR SEAL

08.09.2023

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DATE: 09/20/2023

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PRE-CHECK (PC) DOCUMENT

CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER

CALIFORNIA

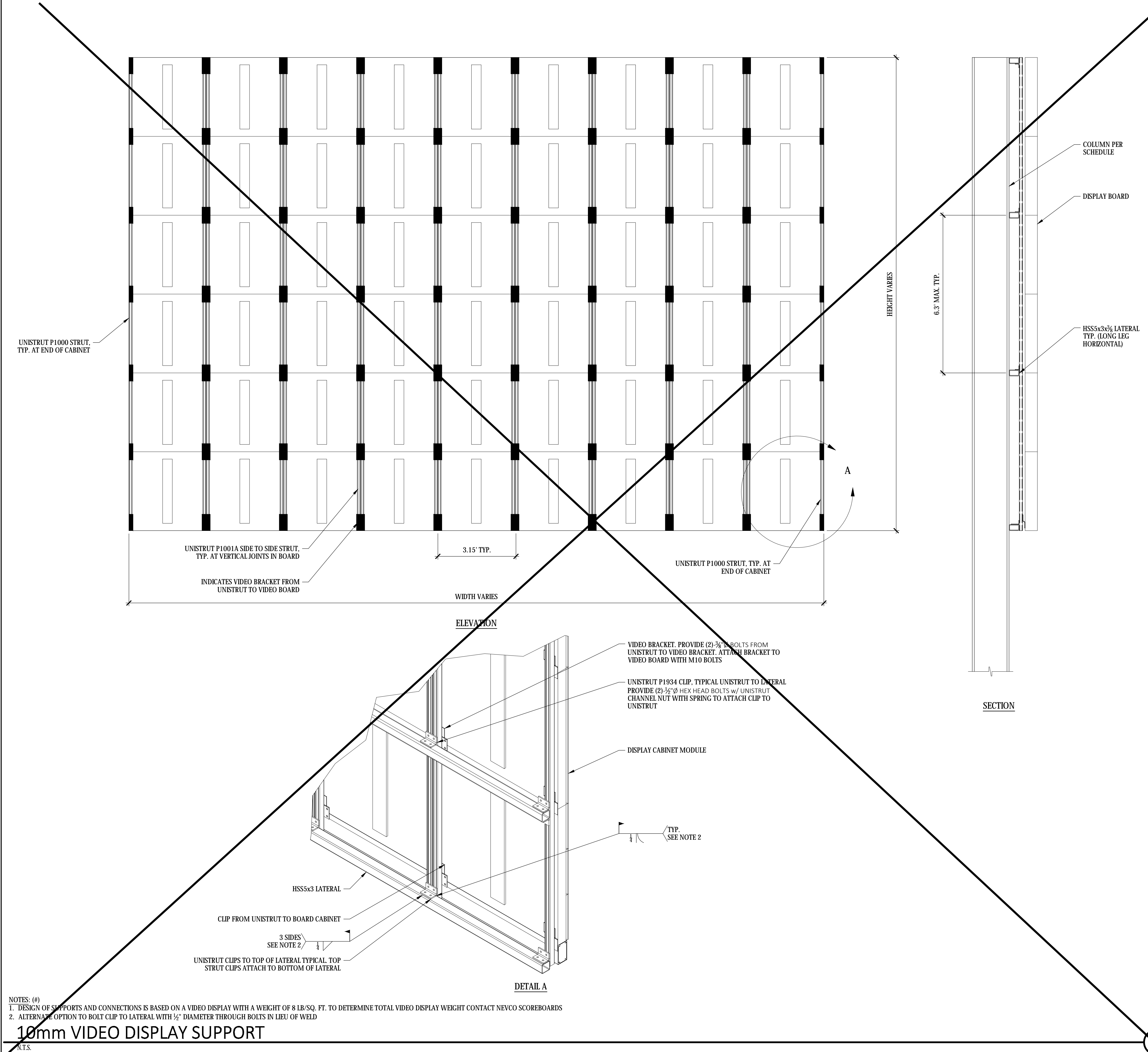
NO. 52336

EXPIRES 3-31-24

STATE OF CALIFORNIA

PC SEOR SEAL

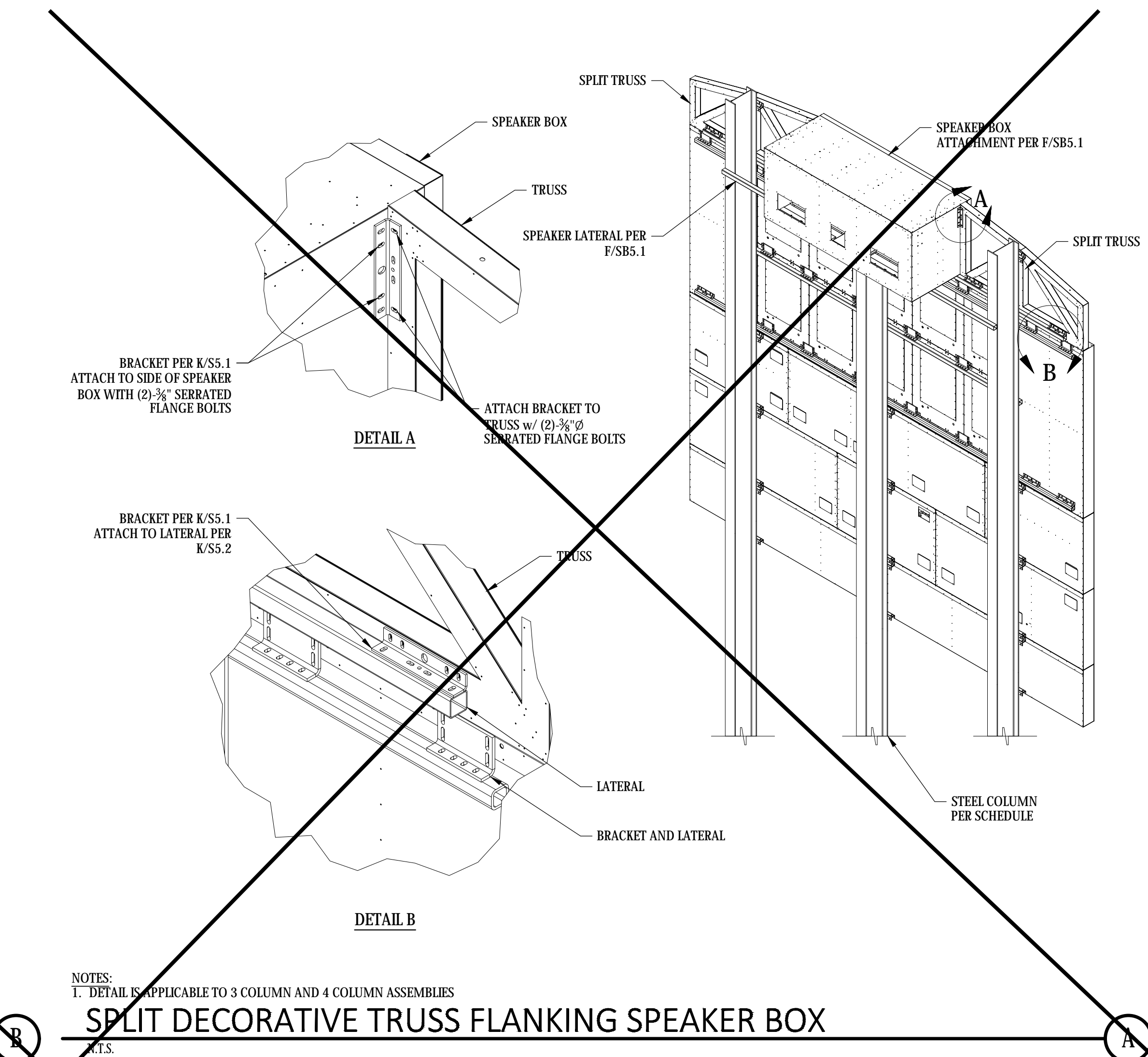
08.09.2023



NOTES: (#)
1. DESIGN OF SUPPORTS AND CONNECTIONS IS BASED ON A VIDEO DISPLAY WITH A WEIGHT OF 8 LB/SQ. FT. TO DETERMINE TOTAL VIDEO DISPLAY WEIGHT CONTACT NEVCO SCOREBOARDS
2. ALTERNATE OPTION TO BOLT CLIP TO LATERAL WITH 1/2\"/>

10mm VIDEO DISPLAY SUPPORT

N.T.S.



SPLIT DECORATIVE TRUSS FLANKING SPEAKER BOX

NOTES:
1. DETAIL IS APPLICABLE TO 3 COLUMN AND 4 COLUMN ASSEMBLIES

N.T.S.

APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. RANNEY
No. 5485
STRUCTURAL
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC SEOR SEAL

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DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022
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for construction is required.

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. RANNEY
No. 52366
STRUCTURAL
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023

ALUMINUM TRUSS
ATTACHMENT DETAILS &
10mm VIDEO DISPLAY
SUPPORT

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB5.4

18 March 2024 9:32 AM Z:\Clients\BrooksRansom Associates\23134 - Modera USD Scoreboards (MHS, MSHS)\CAD Files\23134 - Modera Baseball - Electrical - v3.dwg patrick may

ELECTRICAL EQUIPMENT ANCHORAGE NOTES

ALL ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. WHERE NO DETAIL IS INDICATED, THE FOLLOWING COMPONENTS SHALL BE ANCHORED OR BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2022 CBC, SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16, CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING ELECTRICAL UTILITY SERVICE. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 120/220 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.

THE FOLLOWING ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:

- COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVE A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT.
- COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

THE ANCHORAGE OF ALL ELECTRICAL COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

ELECTRICAL DISTRIBUTION BRACING NOTES

THE ELECTRICAL DISTRIBUTION SYSTEM SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16, SECTION 13.3 AS DEFINED IN ASCE 7-16, SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8, AND 2022 CBC, SECTIONS 1617A.1.24, 1617A.1.25, AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

THE ELECTRICAL DISTRIBUTION SYSTEM IS DETAILED ON THE APPROVED DRAWINGS WITH SPECIFIC NOTES AND DETAILS.

ELECTRICAL GENERAL NOTES

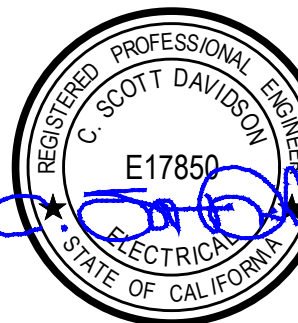
- ALL WORK SHALL MEET THE LATEST ADOPTED ADDITIONS OF THE CALIFORNIA CODE OF REGULATIONS, TITLE 24 AND ALL OTHER APPLICABLE REGULATIONS, WHICH INCLUDE:

CALIFORNIA BUILDING CODE 2022
CALIFORNIA ELECTRICAL CODE 2022
NON RESIDENTIAL CEC ENERGY STANDARDS 2022

- NOTHING IN THE PLANS OR SPECIFICATIONS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES.
- IT IS THE INTENTION OF THESE PLANS AND SPECIFICATIONS TO COVER EVERYTHING REQUIRED TO PROVIDE FOR COMPLETE AND OPERATIVE SYSTEMS. THE CONTRACTOR IS TO FURNISH LABOR, MATERIAL, TRANSPORTATION, EQUIPMENT, MISCELLANEOUS SERVICES, ETC. REQUIRED TO ACCOMPLISH THIS RESULT. ANYTHING WHICH MAY BE REASONABLY CONSTRUED AS A NECESSARY PART OF THE INSTALLATION IS TO BE INCLUDED, WHETHER OR NOT SPECIFICALLY SHOWN OR MENTIONED.
- THE CONTRACTOR SHALL EXAMINE THE SITE AND EXISTING CONDITIONS AND MAKE ALLOWANCES IN THE BID FOR ANY CONDITIONS NOT SHOWN ON THE ELECTRICAL DOCUMENTS.
- THE PLANS AND SPECIFICATIONS ARE INTENDED TO BE USED AS CONSTRUCTION GUIDELINES AND ARE NOT THE TOTAL INSTRUMENT OF CONTRACT DOCUMENTS. IT IS NOT THE INTENTION OF ANY CONSTRUCTION PLANS TO DIVIDE WORK AMONG DIFFERENT TRADES. VERIFY THE SCOPE OF WORK WITH THE ARCHITECT AND THE GENERAL CONTRACTOR.
- ELECTRICAL ROUTING IS DIAGRAMMATIC ONLY. ACTUAL ROUTING & PHYSICAL CONDITIONS MAY VARY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE ACTUAL ROUTING, CONNECTIONS, & PROVISION OF ALL APPURTENANCES NECESSARY FOR A COMPLETE & OPERATING SYSTEM.
- ELECTRICAL EQUIPMENT SHALL HAVE AN APPROVED TESTING LABORATORY LABEL ATTACHED (UL, CSA ETC.) PER CEC 110.2.
- PROVIDE LABELING AND DIRECTORIES FOR ALL SWITCHBOARDS AND PANELBOARDS PER CEC 408.4.
- ELECTRICAL EQUIPMENT SHALL HAVE A SHORT CIRCUIT CURRENT RATING CAPABLE OF WITHSTANDING THE AVAILABLE SHORT CIRCUIT CURRENT PER CEC 110.9.
- PROVIDE MINIMUM 30" WIDE x 78" HIGH x 36" DEEP [42" DEEP] WORK CLEARANCES IN FRONT OF PANELS, SERVICE OR EQUIPMENT RATED AT 120/208V 30 4W [277/480V 30 4W] PER CEC 110.26.
- ELECTRICAL RECEPTACLE OUTLETS ON BRANCH CIRCUIT OF 30 AMPERES OR LESS AND COMMUNICATION SYSTEM RECEPTACLES SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), RECEPTACLES SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.
- CONTROLS OR SWITCHES INTENDED TO BE USED BY THE OCCUPANT OF A ROOM OR AREA TO CONTROL LIGHTING AND RECEPTACLE OUTLETS, APPLIANCES, OR COOLING, HEATING AND VENTILATING EQUIPMENT SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), SWITCHES AND CONTROLS SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.
- ALL WALL AND SURFACE MOUNTED FIXTURES PROTRUDING IN THE PATH OF TRAVEL (POT) OR COMMON PEDESTRIAN WAYS SHALL COMPLY WITH CBC 11B-307.2. OR SHALL BE MOUNTED LESS THAN 27" AFF OR GREATER THAN 80" AFF, OR SHALL BE PROVIDED WITH A BARRIER CONFORMING TO CBC 11B-307.4.
- WIRING FOR 120/208V AND 277/480V SYSTEMS SHALL BE MIN. #12 AWG THHN/THWN-2 COPPER.
- FEEDERS SIZE #4 AND LARGER SHALL BE MEGGER TESTED. TEST RESULTS SHALL BE SUBMITTED TO THE ENGINEER.
- COLORS/FINISHES/MATERIALS FOR ALL ELECTRICAL DEVICES, PLATES, LIGHT FIXTURES, ETC. SHALL BE CHOSEN BY THE ARCHITECT.
- EXISTING EQUIPMENT TO BE REMOVED AND/OR REPLACED SHALL BE DELIVERED TO THE DISTRICT MAINTENANCE DEPARTMENT OR DISPOSED OF, AT THE DISCRETION OF THE DISTRICT.
- CALL USA UNDERGROUND ALERT AND VERIFY WITH DISTRICT THE DESIRED ROUTING AND LOCATIONS OF UNDERGROUND CONDUITS AND STRUCTURES PRIOR TO TRENCHING.
- ALL UNDERGROUND CONDUITS SHALL HAVE MINIMUM 2" COVER. TRENCH PER DETAIL 5/E-1. INSTALL GALVANIZED RIGID STEEL RISERS & ELBOWS WHERE RISERS OCCUR. WRAP GRS BELOW GRADE OR PROVIDE PVC COATED GRS. EXPOSED CONDUIT SHALL BE GRS TO 8'-0", THEN ENT ABOVE AS APPROPRIATE. UNDER NO CIRCUMSTANCE SHALL PVC CONDUIT BE INSTALLED ABOVE GRADE.
- CONDUIT INSTALLED ABOVE GRADE SHALL BE MIN. 3/4" TRADE SIZE. CONDUIT BELOW GRADE SHALL BE MIN. 1" TRADE SIZE.
- INCLUDE ALL WORK REQUIRED TO INVESTIGATE, DEMOLISH, & RECONNECT EXISTING ITEMS.
- ALL LOW VOLTAGE EQUIPMENT SHALL BE DEENERGIZED PRIOR TO DEMO WORK. CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO LIVE EQUIPMENT.

ELECTRICAL SYMBOLS

SYMBOL	DESCRIPTION	NOTES
	POLE WITH POST TOP AREA LUMINAIRE	
	POLE WITH AREA LUMINAIRE	
	RECESSED TROFFER LIGHT FIXTURE	
	SURFACE CEILING LIGHT FIXTURE	
	RECESSED DOWN LIGHT	
	WALL LIGHT	
	EXIT SIGN, CEILING	
	EXIT SIGN, WALL	AT +80" AFF
	EMERGENCY LIGHT FIXTURE	PROVIDE UNSWITCHED HOT CONDUCTOR TO BATTERIES
	SWITCHBOARD	REFER TO POWER SINGLE LINE DIAG.
	POWER PANEL	REFER TO PANEL SCHEDULE
	TERMINAL CABINET	
	DISCONNECT SWITCH, FUSIBLE, WP	REFER TO MECH. PLANS & SPECS.
	COMBO STARTER/DISCONNECT SWITCH, WP	REFER TO MECH. PLANS & SPECS.
	JUNCTION BOX	4-11/16" SQUARE BOX & COVER PLATE MIN.
	MOTOR	REFER TO MECH. PLANS AND SPECS.
	DUPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	QUADPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	WP, GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	DATA OUTLET (RJ-45 CAT6) WITH 2 JACKS	HOMERUN CABLES TO IDF.
	(2) WAP DATA JACKS (RJ-45 CAT6A) MOUNTED IN ATTIC SPACE	HOMERUN CABLES TO IDF.
	WALL MOUNT VoIP OUTLET (RJ-45 CAT6)	HOMERUN CABLES TO IDF.
	WALL MOUNT DATA/COMM OUTLET	HOMERUN CABLES TO IDF.
	WIREMOLD 5400 SURFACE WIREWAY	
	CEILING MOUNT PA SPEAKER	MATCH EXISTING SYSTEM COMPONENTS
	WALL MOUNT PA SPEAKER IN SURFACE ENCLOSURE	MATCH EXISTING SYSTEM COMPONENTS
	AUDIO/VISUAL INPUT WITH HDMI/VGA/ 3.5MM AUDIO/USB JACKS AND WALL PLATE	INSTALL CABLES BETWEEN TEACHER STATION AND PROJECTOR. SEE DETAIL 6/E-2.
	ANALOG CLOCK, BATTERY POWERED	VERIFY COMPATIBILITY WITH EXISTING SYSTEM
	MAIN DISTRIBUTION FRAME (MDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	INTERMEDIATE DISTRIBUTION FRAME (IDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM TERMINAL BLOCK	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM HEAD END	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	TEL. SYSTEM TERMINAL BLOCK	WHERE EXISTING
	TEL. SYSTEM HEAD END	WHERE EXISTING
	FIBER OPTIC SPLICE LOCATION	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	CAT6 PATCH PANEL	WHERE EXISTING
	FIRE ALARM SLC & NAC TERMINAL BLOCKS	LOCATION FOR REFERENCE. SEE F.A. PLANS.
	FIRE ALARM EXPANDER PANEL	SEE F.A. PLANS
	FIRE ALARM CONTROL PANEL	SEE F.A. PLANS
	EMERGENCY VOICE/ALARM COMMUNICATION PANEL	SEE F.A. PLANS
	EXISTING WIRING TO REMAIN	
	WIRING BELOW GRADE	3/4" CONDUIT MIN.
	WIRING IN WALL OR CEILING	3/4" CONDUIT MIN.
	LOW VOLTAGE WIRING	
	CONDUIT RISER	3/4" CONDUIT MIN.
	FLEXIBLE CONDUIT	3/4" CONDUIT MIN.
	CONDUIT STUB AND CAP	3/4" CONDUIT MIN.
	HASH MARKS DENOTES QTY. OF CONDUCTORS	
	WIRE SIZE INDICATED, IF OTHER THAN #12 AWG	3/4" CONDUIT MIN.
	HOME RUN (TO PANEL 'A', CIRCUIT '15')	3/4" CONDUIT MIN.
	"EXISTING"	
	"UNLESS OTHERWISE NOTED"	
	"WEATHERPROOF" / NEMA 3R	
	"GROUND FAULT INTERRUPTER"	



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APPROVALS
APPLICATION #
02-122088

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122088 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☐
DATE: 04/02/2024

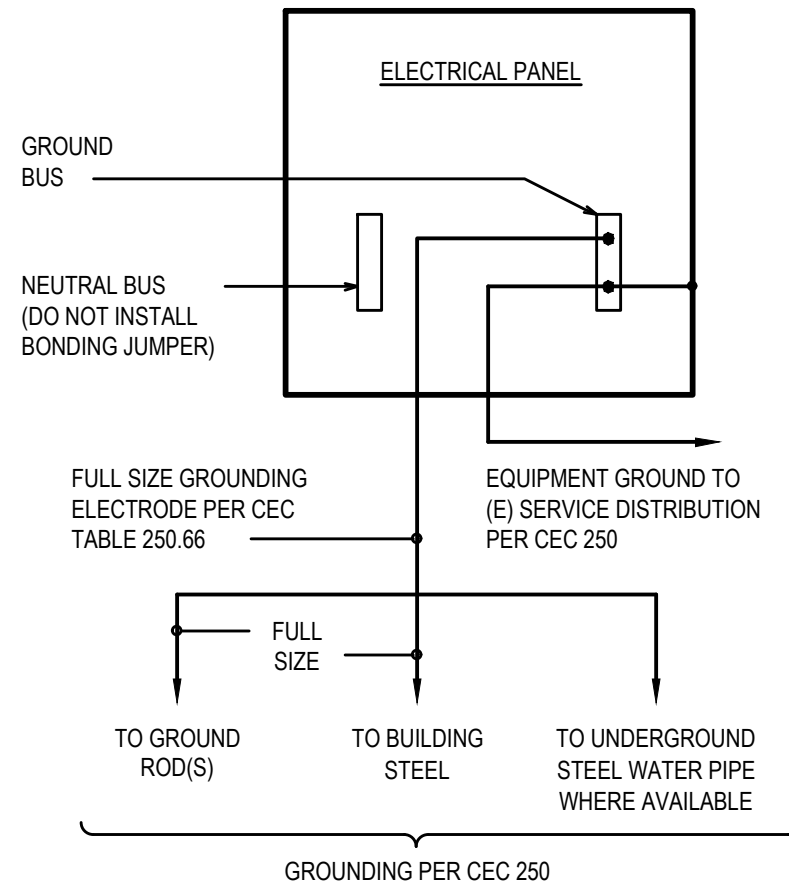
DATE: 12/05/2023

PARTIAL SITE PLAN
MADERA HIGH SCHOOL
BASEBALL SCOREBOARD
MADERA, CA 93637

REVISIONS
1
2
3
4
5

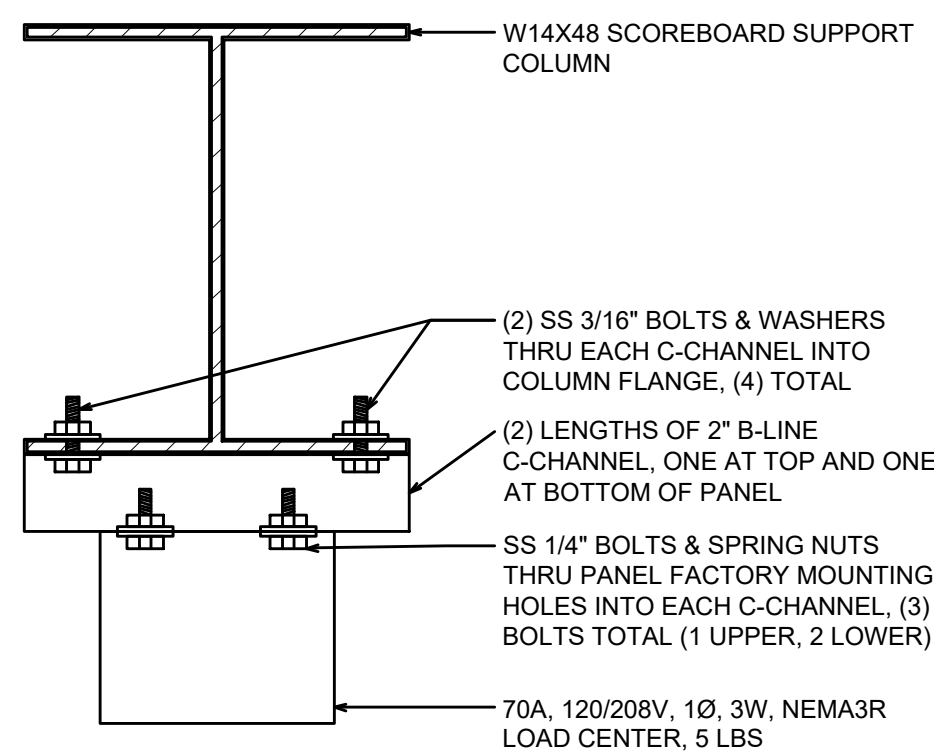
BrooksRansom
ASSOCIATES
7415 N. PALM AVE. STE 100 | FRESNO, CA 93711
(559) 449-8444 OFFICE | (559) 449-8404 FAX

SHEET:
E-1
PROJECT: 23934



3 Panel Grounding Detail

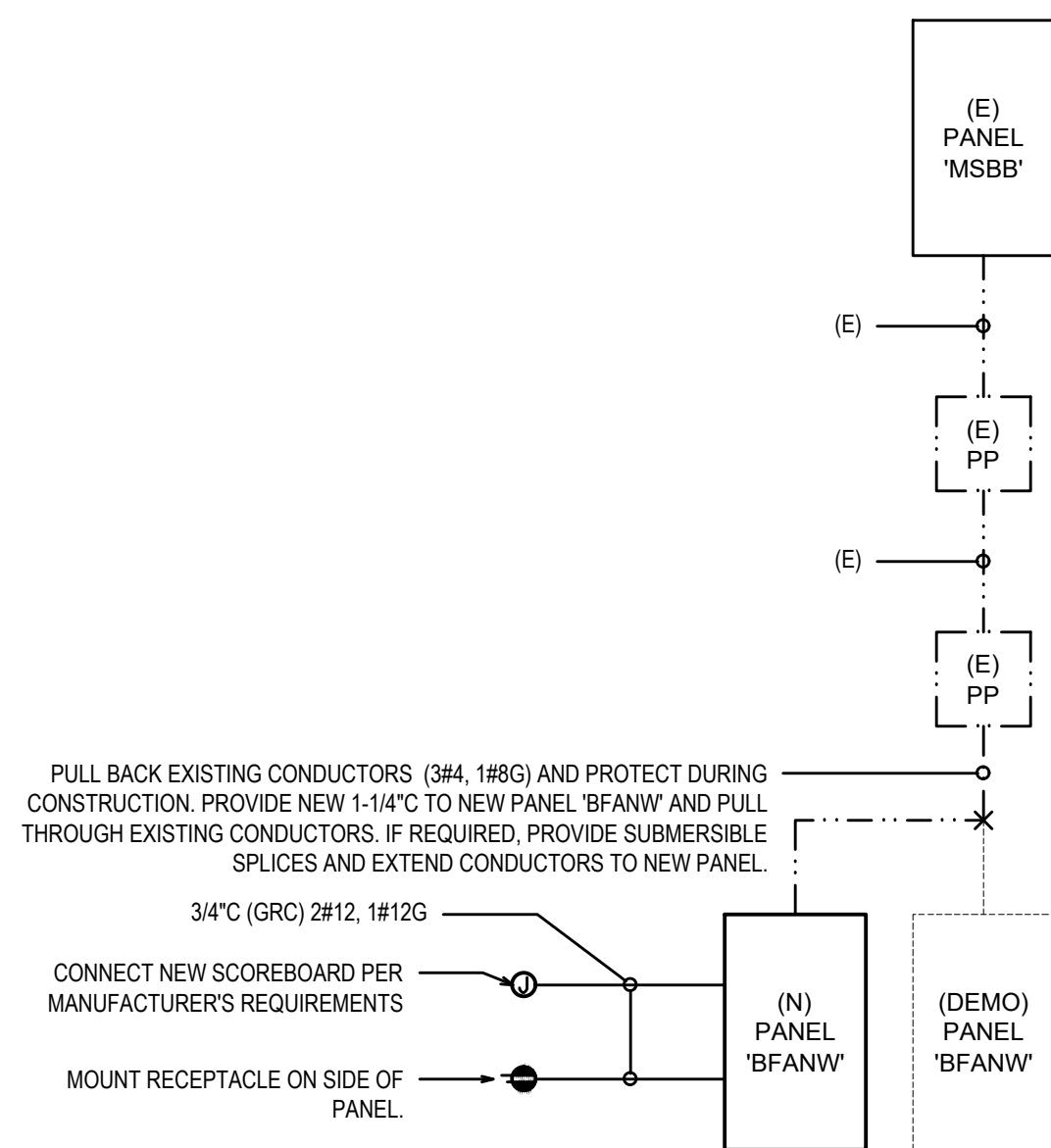
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SECTION THROUGH SCOREBOARD SUPPORT COLUMN

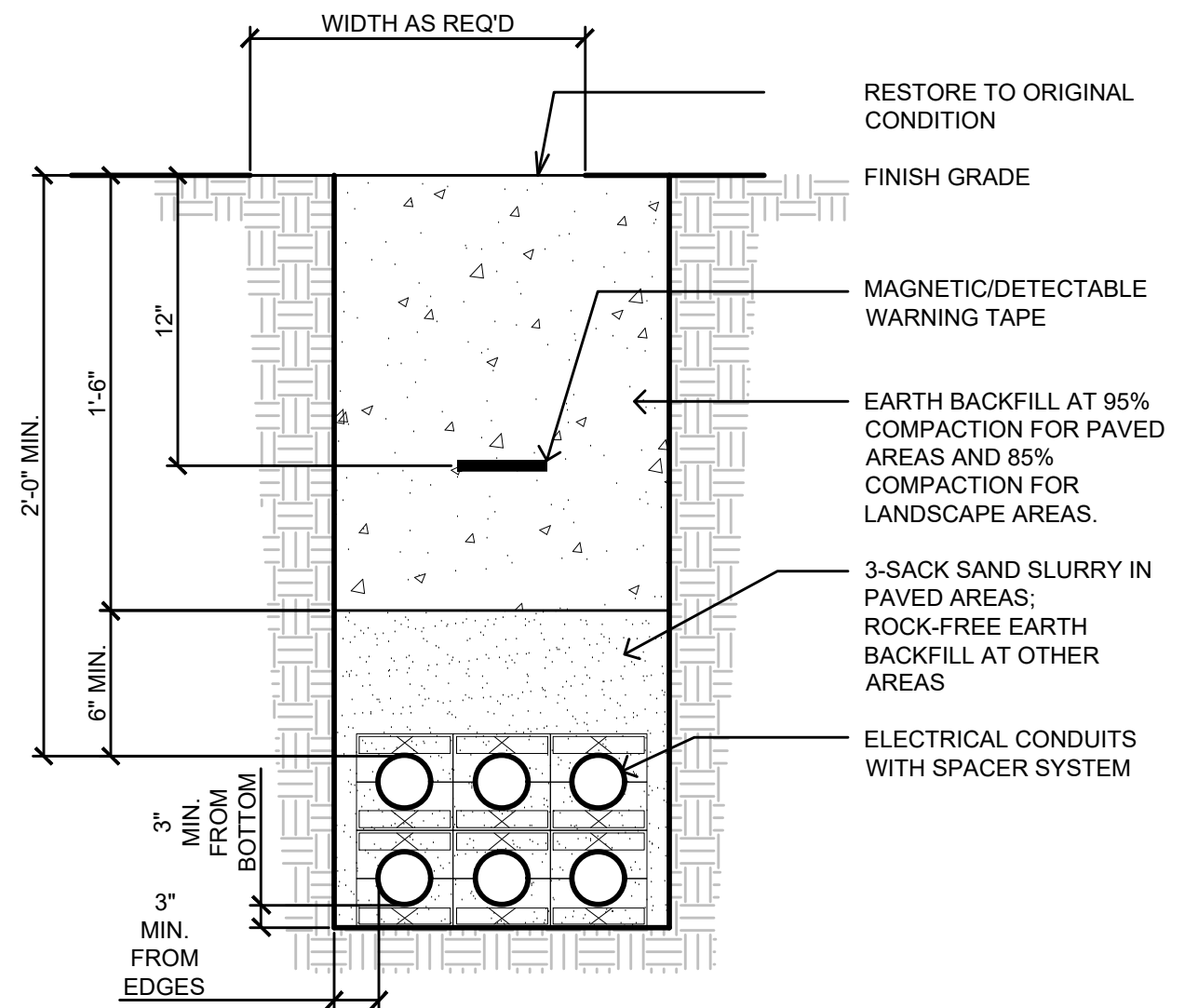
2 Panel Mounting Detail

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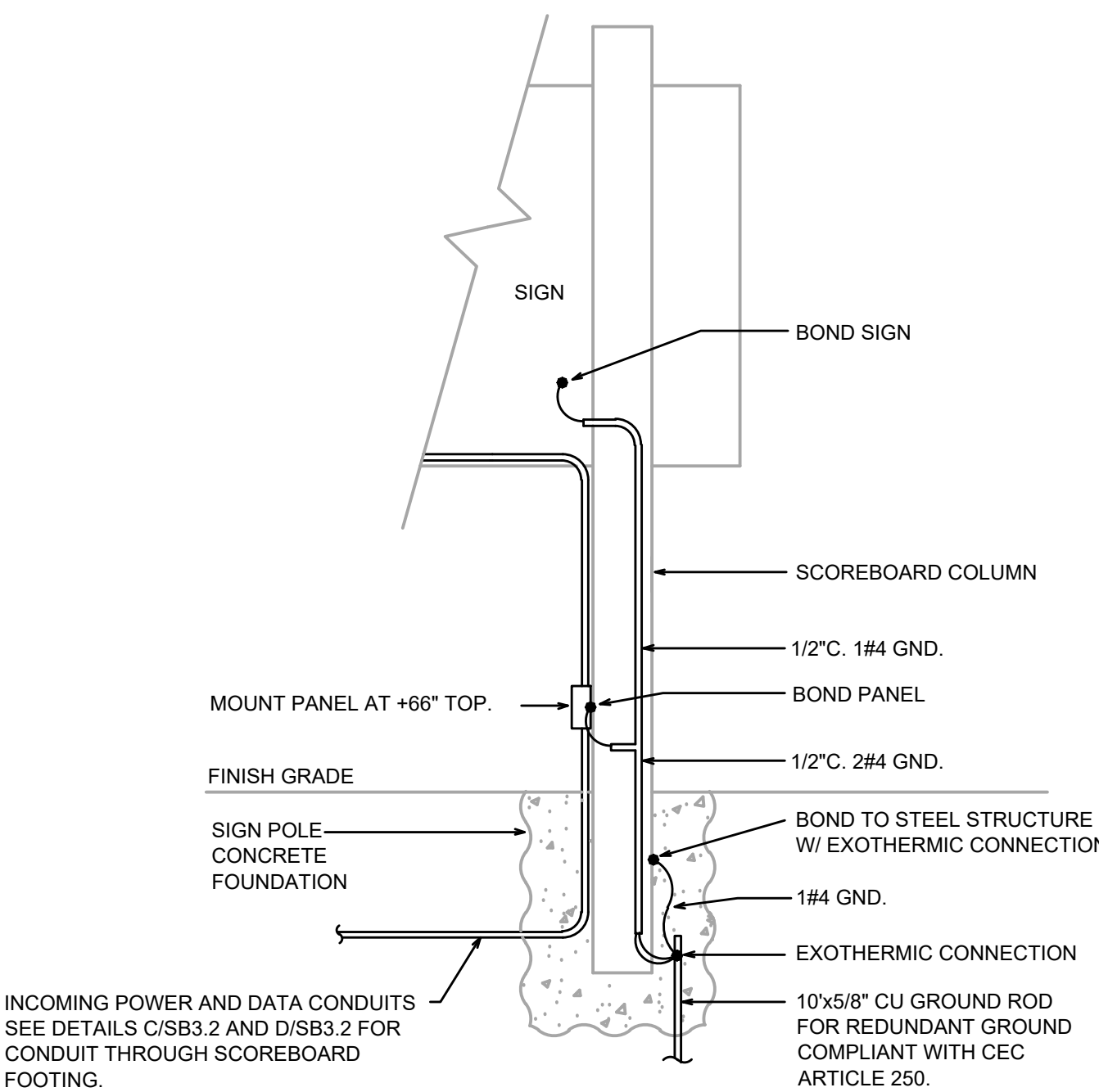
1 Single Line Diagram

No Scale



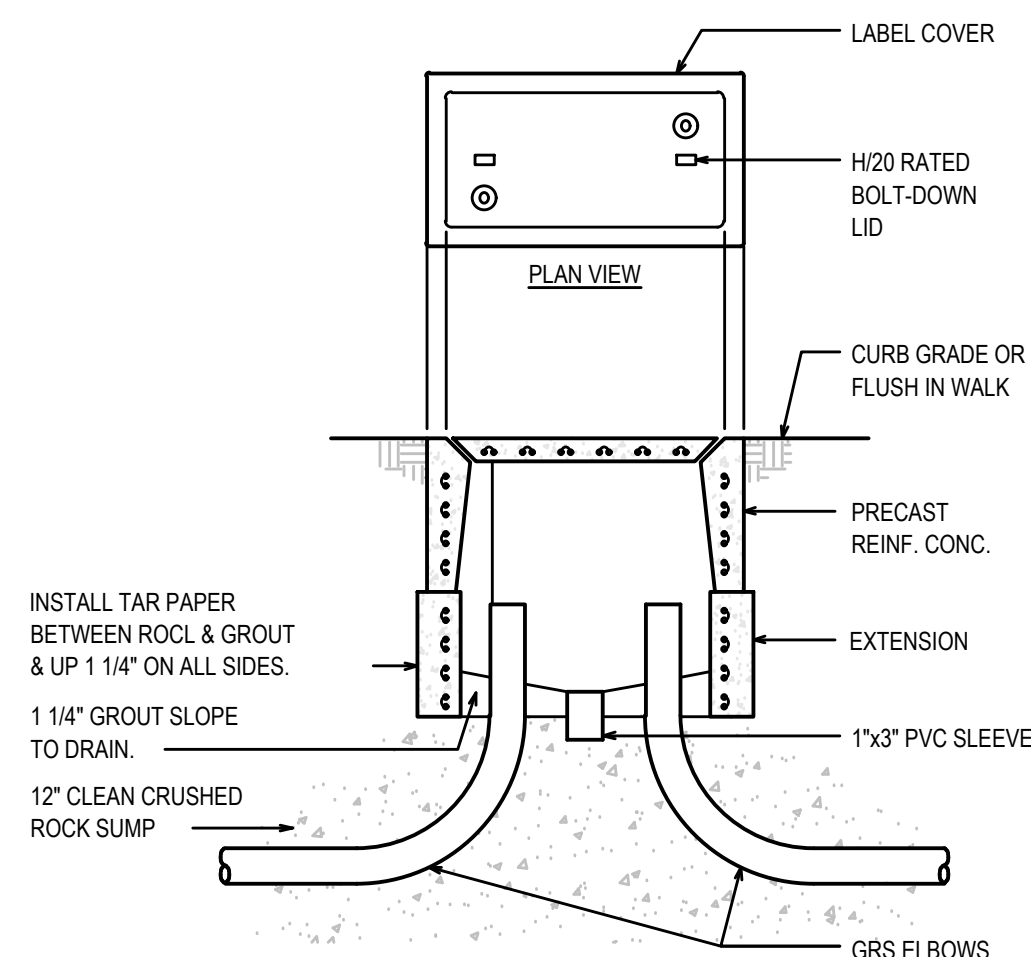
5 Trenching Detail

No Scale



4 Scoreboard Grounding Detail

No Scale



6 Pull Box Detail

No Scale

KEY NOTES

1.

EXISTING SPORTS LIGHTING POLE TO REMAIN. SHOWN FOR REFERENCE. EXERCISE CAUTION WHEN EXCAVATING IN THE SCOREBOARD AREA FOR UNDERGROUND CONDUITS. PROVIDE LOCATION SERVICE OF PRIVATE CONDUITS. SPORTS LIGHTING IS FED FROM THE EXISTING ELECTRICAL SERVICE SOUTHEAST OF THE STADIUM.
2.

EXISTING MAIN SWITCHBOARD 'MSBB' TO REMAIN. 120/208V, 3Ø, 4W, 10KAIC, NEMA 3R.
3.

VERIFY EXISTING PULL BOX TO REMAIN. VERIFY LOCATION IN FIELD. IF PULLBOX IS NOT PRESENT, INTERCEPT EXISTING FEEDER AND/OR COMMUNICATION CONDUIT AND PROVIDE NEW 10X17 PULL BOX WITH 12" EXTENSION AND LID, H20-RATED. SEE DETAIL 6/E-1.
4.

EXISTING SCOREBOARD PANEL FEEDER TO REMAIN. 1-1/2"C. 3Ø4, 1Ø8G. EXTEND FEEDER IF REQUIRED. PROVIDE SUBMERSIBLE SPLICES.
5.

EXISTING SCOREBOARD DATA/CONTROL CONDUIT TO PRESS BOX. REPLACE EXISTING CONDUCTORS WITH NEW.
6.

DEMO EXISTING PANEL 'BBSB' FROM STEEL SCOREBOARD SUPPORT COLUMN. DISCONNECT EXISTING FEEDER AND PULL BACK TO PULL BOX PP-7 AND PRESERVE FOR RECONNECTION TO NEW PANEL.
7.

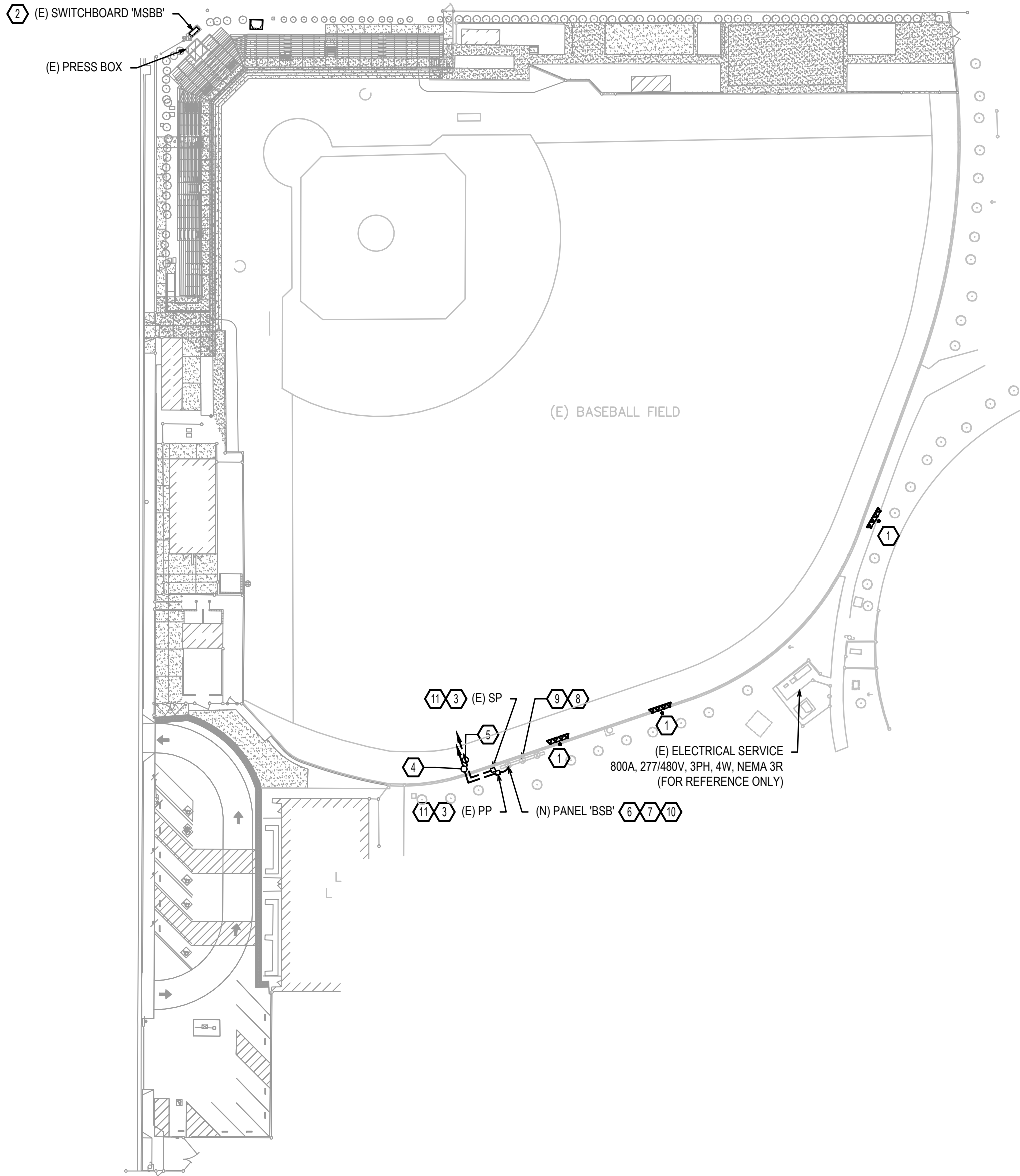
PROVIDE AND INSTALL NEW LOAD CENTER 'BBSB'. 70A, 4 CKT, 120/208V,
- 10, 10KAIC, NEMA 3R. S.Q.D #Q024L70RB (OR APPROVED EQUAL) PROVIDE (2) 20A/1P CIRCUIT BREAKERS. RECONNECT EXISTING CONDUCTORS. SEE DETAIL 1/E-1, 2/E-1, 3/E-1, 4/E-1, 5/E-1.
8.

DISCONNECT EXISTING SCOREBOARD POWER AND DATA/CONTROL. REMOVE EXISTING CONDUCTORS AND EXPOSED CONDUIT BACK TO NEAREST PULL BOX.
9.

POWER CONNECTION BY SCOREBOARD SUPPLIER. NEVCO MODEL #1609, 120V, 3.9A. 3/4"C. 2Ø12, 1Ø12G. SEE DETAIL 4/E-1.
10.

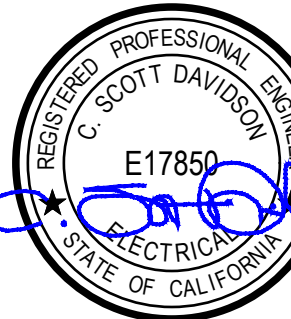
PROVIDE RECEPTACLE -42", 20A, 120V, GFCI-PROTECTED. WEATHER-RESISTANT RECEPTACLE WITH WEATHERPROOF WHILE-IN-USE COVER. 3/4"C. 2Ø12, 1Ø12G. SEE SINGLE LINE DIAGRAM 1/E-1.
11.

RELOCATE/REPLACE EXISTING PULL BOXES IF REQUIRED. SEE DETAIL 6/E-1. IF REPLACING BOXES, REPLACE WITH LIKE SIZE, H20-RATED.



1 PARTIAL ELECTRICAL SITE PLAN

SCALE: 1"= 50'-0"



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SHEET:

E-2

PROJECT 23314