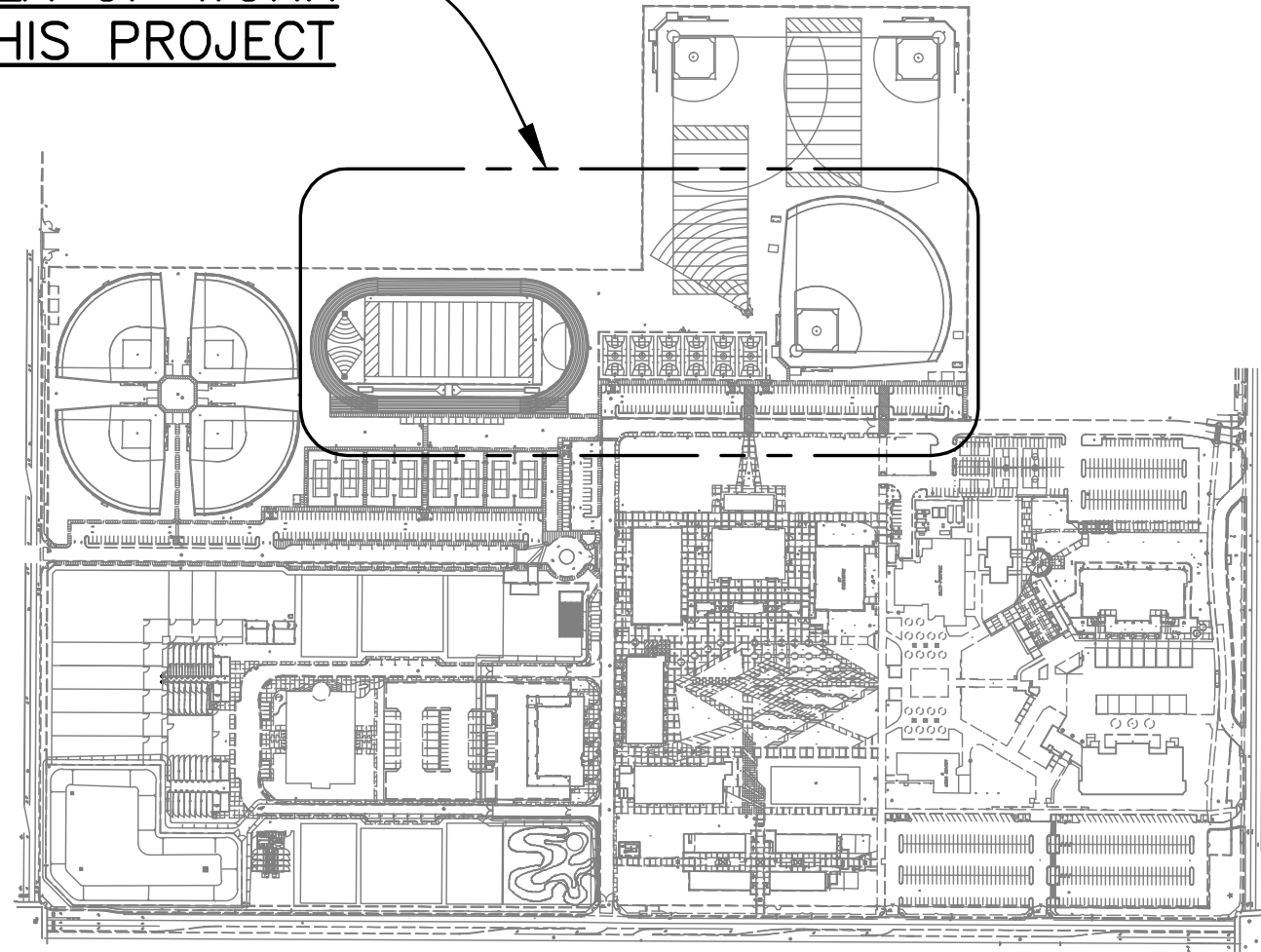


## 10 VICINITY MAP

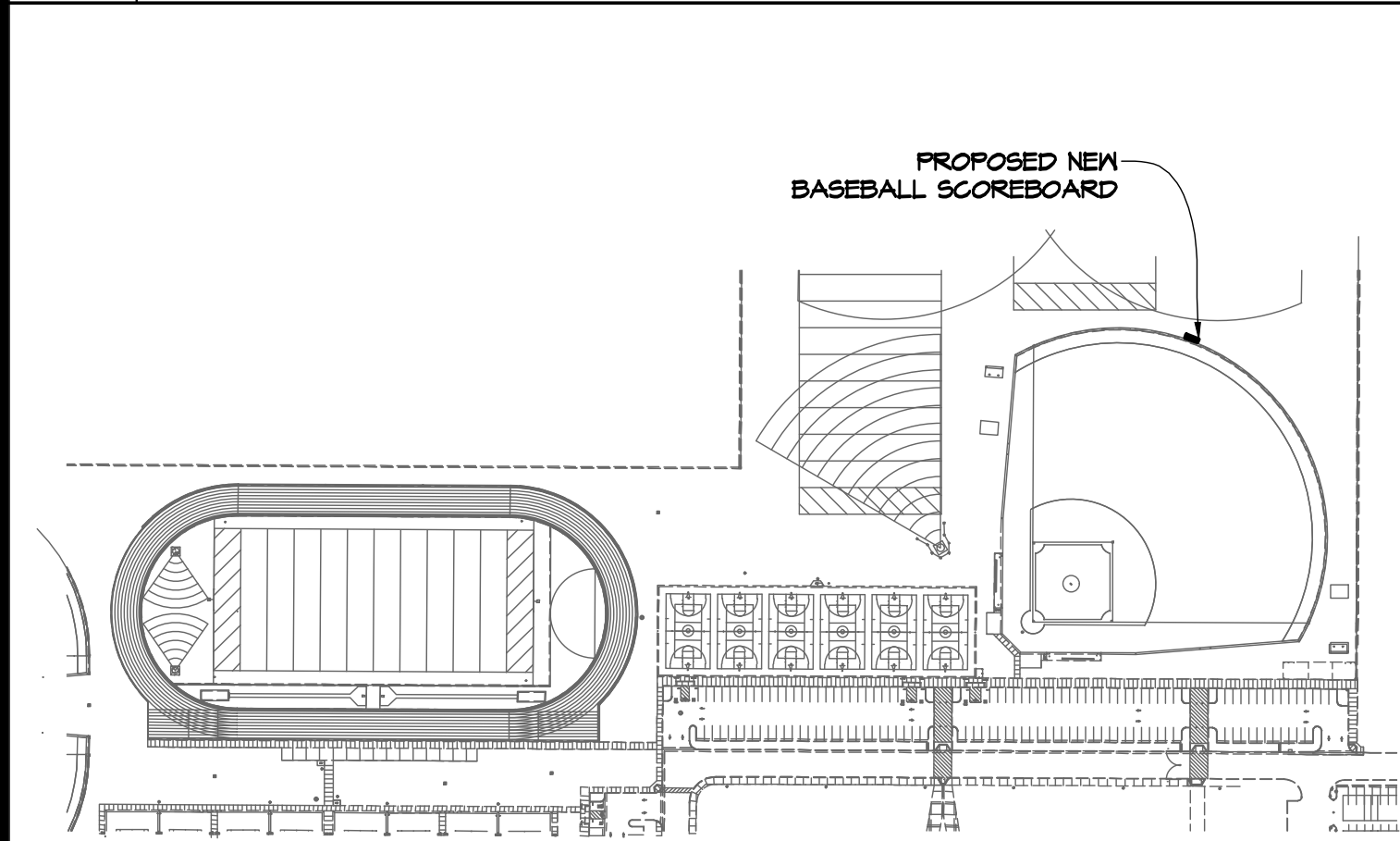
SCALE: N.T.S.

### AREA OF WORK THIS PROJECT



## 11 SITE PLAN

SCALE: N.T.S.



## 12 AREA OF WORK

ITEMS

SCALE: N.T.S.

## 9 CONSULTANTS

SCALE: N.T.S.

### FLOOD HAZARD ZONE INFORMATION:

- FLOOD ZONE DESIGNATION: ZONE X - OTHER AREAS OUTSIDE OF THE 0.2% ANNUAL CHANGE FLOODPLAIN
- FIRM PANEL DESIGNATION: MAP# 06031C0185C
- FIRM EFFECTIVE DATE: JUNE 16, 2009
- BASE FLOOD ELEVATION: N/A

## 10 FLOOD ZONE

ITEMS

SCALE: N.T.S.

Bid No.020624A-MSHS Varsity Baseball Scoreboard



# MADERA UNIFIED SCHOOL DISTRICT

1205 S. Madera Ave. Madera, California 93637  
(559) 675-4548

### PROJECT INFORMATION:

- PROJECT NAME:** MADERA SOUTH HIGH SCHOOL
- LOCATION:** 705 W. PECAN AVE.  
MADERA, CALIFORNIA 93637
- PROJECT DESCRIPTION:** PROVIDE NEW MODEL NEVCO 1606-PC  
OUTDOOR BASEBALL SCOREBOARD

### SCOPE OF WORK:

- CONSTRUCT SCOREBOARDS STRUCTURAL SUPPORTS AND FOOTINGS.
- INSTALL OWNER FURNISHED SCOREBOARD.
- MAKE ELECTRICAL POWER CONNECTION FROM EXISTING ELECTRICAL PULL BOX, INCLUDING DISCONNECT.
- PAINT EXPOSED STEEL.

### GENERAL NOTES:

- ALL WORK SHALL BE DONE IN ACCORDANCE WITH THESE CONSTRUCTIONS DRAWINGS, THE CONTRACT SPECIFICATIONS AND, WHERE APPLICABLE, THE CITY OF HANFORD AND THE STATE OF CALIFORNIA STANDARD SPECIFICATIONS.
- THE CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION ACTIVITIES WITH THE SCHOOL DISTRICT'S USE OF THE FACILITIES AND OTHER CONTRACTORS WHO MAY BE DOING CONSTRUCTION WITHIN THE PROJECT SITE.
- THE CONTRACTORS SHALL CONTACT DISTRICT OFFICIALS FOR DETERMINATION OF DEPTH AND LOCATION OF UNDERGROUND UTILITIES PRIOR TO EXCAVATION IN THE PROJECT SITE.
- BEFORE COMMENCING WORK, THE CONTRACTOR SHALL NOTIFY ALL UTILITY AUTHORITIES OR UTILITY COMPANIES HAVING POSSIBLE INTEREST IN THE WORK OF THE CONTRACTOR'S INTENTION TO EXCAVATE PROXIMATE TO EXISTING FACILITIES AND THE CONTRACTOR SHALL VERIFY THE LOCATION OF ANY UTILITIES IN THE WORK AREA, NOTIFY U.S.A. AT 1(800) 642-2444, TWO (2) DAYS PRIOR TO EXCAVATION.
- CONTRACTOR SHALL PROVIDE 6' HIGH TEMPORARY CHAIN LINK FENCE AROUND THE PERIMETER OF THE WORK AREAS EXCEPT WHERE ENCLOSED BY EXISTING FENCING.
- ALL CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 CALIFORNIA BUILDING CODE (CBC).
- CHANGE TO THE APPROVED DRAWINGS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
- A "DSA CERTIFIED" CLASS 3 PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
- FIRE SAFETY DURING DEMOLITION AND CONSTRUCTION SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF CHAPTER 33 OF THE 2022 CALIFORNIA BUILDING CODE AND THE APPLICABLE PROVISIONS OF CHAPTER 33 OF THE CALIFORNIA FIRE CODE.

## 6 GENERAL DESIGN NOTES

ITEMS

SCALE: N.T.S.

### SHEET INDEX:

SHT. NO. TITLE

#### GENERAL

- C-1 COVER SHEET  
SP-1 STRUCTURAL PARTIAL SITE PLAN

#### NEVCO DSA P.C. 04-122317

- SBO.1 COVER SHEET  
SBO.2 STRUCTURAL NOTES  
~~SBO.3 EXAMPLE DSA 103-TESTING AND INSPECTIONS~~  
~~SB1.1 MARQUEE CAISSON - EMBEDDED~~  
~~SB1.2 MARQUEE CAISSON - BOLTED~~  
~~SB1.3 MARQUEE MAT FOOTING~~  
~~SB2.1 TWO COLUMN CAISSON - EMBEDDED~~  
~~SB2.2 TWO COLUMN CAISSON - BOLTED~~  
~~SB2.3 TWO COLUMN MAT FOOTING~~  
~~SB3.1 THREE COLUMN CAISSON - EMBEDDED~~  
~~SB3.2 THREE COLUMN CAISSON - BOLTED~~  
~~SB3.3 THREE COLUMN MAT FOOTING~~  
~~SB4.1 FOUR COLUMN CAISSON - EMBEDDED~~  
~~SB4.2 FOUR COLUMN CAISSON - BOLTED~~  
~~SB4.3 FOUR COLUMN MAT FOOTING~~  
SB5.1 ATTACHMENT DETAILS  
SB5.2 OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAILS  
SB5.3 DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS  
~~SB5.4 DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS & 10mm VIDEO BOARD~~  
~~SB6.1 INDOOR WALL MOUNTED SCOREBOARD~~

#### ELECTRICAL

- E1 ELECTRICAL SYMBOLS, NOTES AND DETAILS  
E2 ELECTRICAL PARTIAL SITE PLAN

TOTAL SHEET COUNT: 24

## 3 SHEET INDEX

SCALE: N.T.S.

## 4 DEFERRED SUBMITTAL

SCALE: N.T.S.



APPROVALS:

APPLICATION #  
02-121895

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT

APP: 02-121895 INC:

REVIEWED FOR

SS ☒ FLS ☒ ACS ☐

DATE: 01/24/2024

DATE: 11/22/2023

COVER SHEET  
MADERA SOUTH HIGH SCHOOL  
BASEBALL SCOREBOARD  
MADERA, CA 93637

REVISIONS  
1  
2  
3  
4  
5

BrooksRansom  
ASSOCIATES  
7415 N. PALM AVE. STE. 100 FRESNO, CA 93711  
(559) 449-8444 OFFICE | (559) 449-8404 FAX

SHEET:

C-1

PROJECT 23314






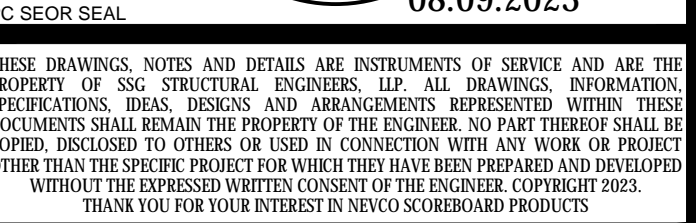




DSA P.C. 04-122317



**SSG**  
structural engineers



301 East Harris Avenue, Greenville, Illinois 62246

[www.nevco.com](http://www.nevco.com)

SA STAMP

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application for construction is required.



## HEET INFORMATION

18.09.2023

CHECKED \_\_\_\_\_

JMK

S23109

SHEET

# SB0.1

SCOREBOARD ASSEMBLY WORKSHEET (TABLE A, C & D) INSTRUCTIONS	
STEP 1:	DETERMINE DESIRED SCOREBOARD ASSEMBLY. FILL OUT SCOREBOARD ASSEMBLY TABLE (TABLE A BELOW). PROVIDE NEVCO PART NUMBERS, PART HEIGHT, PART WIDTH, AND PART WEIGHTS.
STEP 2:	DETERMINE TOTAL ASSEMBLY HEIGHT, WIDTH, AND WEIGHT, TABLE A
STEP 3:	BASED ON TOTAL ASSEMBLY WIDTH, DETERMINE THE NUMBER OF REQUIRED COLUMNS. SEE SHEETS SB1.X FOR 1 COLUMN ASSEMBLY OPTIONS SB2.X FOR 2 COLUMN ASSEMBLY OPTIONS SB3.X FOR 3 COLUMN ASSEMBLY OPTIONS SB4.X FOR 4 COLUMN ASSEMBLY OPTIONS SB6.1 FOR WALL MOUNTED ASSEMBLY OPTIONS (SKIP STEPS 4, 5, & 7)
STEP 4:	PICK FOUNDATION TYPE (CAISSON WITH EMBEDDED COLUMN, CAISSON WITH BOLTED COLUMN, OR MAT FOOTING) . MARK APPLICABLE SHEET ON SHEET INDEX, SB0.1
STEP 5:	MARK APPLICABLE CHECK BOX FOR SCOREBOARD SIZE ON DETAIL 'A' OF SELECTED COLUMN/FOUNDATION OPTION (SHEETS SB1.X, SB2.X, SB3.X OR SB4.X)
STEP 6:	FILL IN SITE SPECIFIC SEISMIC AND WIND VALUES TABLE C ON SB0.1.
STEP 7:	FILL IN SITE SPECIFIC FLOOD ZONE AS REQUIRED, TABLE D ON SB0.1
STEP 8:	VERIFY ALL APPLICABLE SHEETS ARE MARKED ON SHEET INDEX, SB0.1. INCLUDE ONLY MARKED SHEETS AS PART OF DSA SUBMITTAL
SITE SPECIFIC SUBMITTAL REQUIREMENTS	
SEE DSA POLICY PL 07-02 FOR ADDITIONAL INSTRUCTIONS REGARDING USE AND APPLICATION OF THIS PRE-CHECK DOCUMENT. ALL SITE SPECIFIC SUBMITTALS SHALL INCLUDE:	
1.	COMPLETED DSA 1 APPLICATION, DSA3, DSA 103, AND FILING FEE AND COPY OF THE PRE-CHECK DOCUMENT WITH APPLICABLE DESIGN OPTION MARKED ON THE MARQUEE, TWO COLUMN, THREE COLUMN, FOUR COLUMN, OR WALL ASSEMBLY SCHEDULES.
2.	SITE PLAN OF FACILITY IDENTIFYING ALL STRUCTURES BY DSA APPLICATION NUMBER. LOCATION OF SCOREBOARD SHALL BE IDENTIFIED. ELECTRICAL PANEL SERVING THE SCOREBOARD SHALL BE LOCATED AND IDENTIFIED.
3.	WHERE WIRELESS CONTROLLERS ARE NOT SPECIFIED, AN ACCESSIBLE PATH OF TRAVEL AND ACCESSIBLE SEATING FOR THE SCOREBOARD OPERATOR SHALL BE IDENTIFIED AND PROVIDED.
4.	PROVIDE AN ELEVATION OF PROPOSED SCOREBOARD IDENTIFYING ALL INSTALLED DISPLAY COMPONENTS, SIGNAGE, TRUSSES, AND ADDITIONAL COMPONENTS IN THE PRE-CHECK DOCUMENT. ALL ELEMENT WEIGHTS SHALL BE SPECIFIED.
5.	THE APPLICABLE SHEETS SHALL BE IDENTIFIED BY MARKING APPROPRIATE CHECK BOX ON THIS SHEET.
6.	THE APPLICABLE CONFIGURATION SHALL BE IDENTIFIED BY MARKING APPROPRIATE CHECK BOX ON THE 'A' DETAILS ON THE APPLICABLE SHEET.
7.	PROVIDE CUT SHEETS OF THE BOARDS, BOXES, AND EQUIPMENT TO BE MOUNTED ON THE STRUCTURE. CUT SHEETS SHALL INCLUDE WEIGHTS AND DIMENSIONS
8.	SITE SPECIFIC SEISMIC DESIGN CRITERIA SHALL BE PROVIDED IN THE DRAWINGS.
9.	SITE SPECIFIC BASIC DESIGN WINDSPEED AND SITE EXPOSURE SHALL BE PROVIDED ON THE DRAWINGS, SEE TABLE C.
10.	STEEL COATING SPECIFICATIONS FOR WEATHER PROTECTION IF DIFFERENT THAN NOTED ON SB0.3

CHECK ALL THAT APPLY	SHEET INDEX	
<input checked="" type="checkbox"/> (REQ'D)	SB0.1	COVER SHEET
<input checked="" type="checkbox"/> (REQ'D)	SB0.2	STRUCTURAL NOTES
<input type="checkbox"/>	<del>SB0.3</del>	<del>EXAMPLE DSA 103 - TESTING AND INSPECTIONS</del>
<input type="checkbox"/>	<del>SB1.1</del>	<del>MARQUEE CAISSON - EMBEDDED</del>
<input type="checkbox"/>	<del>SB1.2</del>	<del>MARQUEE CAISSON - BOLTED</del>
<input type="checkbox"/>	<del>SB1.3</del>	<del>MARQUEE MAT FOOTING</del>
<input type="checkbox"/>	<del>SB2.1</del>	<del>TWO COLUMN CAISSON - EMBEDDED</del>
<input type="checkbox"/>	<del>SB2.2</del>	<del>TWO COLUMN CAISSON - BOLTED</del>
<input type="checkbox"/>	<del>SB2.3</del>	<del>TWO COLUMN MAT FOOTING</del>
<input type="checkbox"/>	<del>SB3.1</del>	<del>THREE COLUMN CAISSON - EMBEDDED</del>
<input checked="" type="checkbox"/>	SB3.2	THREE COLUMN CAISSON - BOLTED
<input type="checkbox"/>	<del>SB3.3</del>	<del>THREE COLUMN MAT FOOTING</del>
<input type="checkbox"/>	<del>SB4.1</del>	<del>FOUR COLUMN CAISSON - EMBEDDED</del>
<input type="checkbox"/>	<del>SB4.2</del>	<del>FOUR COLUMN CAISSON - BOLTED</del>
<input type="checkbox"/>	<del>SB4.3</del>	<del>FOUR COLUMN MAT FOOTING</del>
<input checked="" type="checkbox"/>	SB5.1	ATTACHMENT DETAILS
<input checked="" type="checkbox"/>	SB5.2	OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAILS
<input type="checkbox"/>	<del>SB5.3</del>	<del>DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS</del>
<input type="checkbox"/>	<del>SB5.4</del>	<del>DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS &amp; 10mm VIDEO BOARD</del>
<input type="checkbox"/>	<del>SB6.1</del>	<del>INDOOR WALL MOUNTED SCOREBOARD</del>

CODE INFORMATION
<p><u>2022 CALIFORNIA BUILDING STANDARDS CODE (TITLE 24, CCR):</u></p> <p>2022 ADMINISTRATIVE CODE, PART 1, TITLE 24 CODE OF REGULATIONS (CCR)</p> <p>2022 CALIFORNIA BUILDING CODE VOLUMES 1 &amp; 2, PART 2, TITLE 24 CCR</p> <p>2022 CALIFORNIA ELECTRICAL CODE, PART 3, TITLE 24 CCR</p> <p>2022 CALIFORNIA MECHANICAL CODE, PART 4, TITLE 24 CCR</p> <p>2022 CALIFORNIA PLUMBING CODE, PART 5, TITLE 24 CCR</p> <p>2022 CALIFORNIA ENERGY CODE, PART 6, TITLE 24 CCR</p> <p>2022 CALIFORNIA FIRE CODE, PART 9, TITLE 24 CCR</p> <p>2022 CALIFORNIA GREEN BUILDING STANDARDS CODE, PART 11, TITLE 24 CCR</p> <p>2022 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24 CCR</p> <p>REFERENCED CODE SECTIONS FOR APPLICABLE STANDARDS:</p> <p>2022 CALIFORNIA BUILDING CODE, CHAPTER 35</p> <p>2022 CALIFORNIA FIRE CODE, CHAPTER 80</p>

TABLE B - STRUCTURAL DESIGN VALUES	
All values reported are unfactored and strength level, unless noted otherwise	
Gravity Design Data	
Dead Loads:	Value
Sign Dead Load	PER SCHEDULE
Snow Loads:	
Ground Snow Load, $P_g$ (Maximum)	30 psf
Deflection Criteria:	
Sign, Wind Load	H/240
Wind Design Data	
Design Wind Speed (3-sec gust), $V_{HT}$	Value
Design Wind Speed (3-sec gust), $V_{AS}$	100 mph
Risk Category	II
Exposure Category	C
Applicable Internal Pressure Coefficient	$\pm 0.18$
Design Wind Pressure(s) for Components & Cladding Not specifically designed by the Registered Design Professional, and to be modified by applicable factors per ASCE 7)	$q_s = 21.8k \times ps$ $k, VARES$
Earthquake Design Data	
Risk Category	II
Importance Factor, $I_e$	1.0
Mapped Spectral Response Accelerations (Maximum)	$S_w = 3.73 g$ $S_u = 1.0 g$
Site Class	A through E
Spectral Response Coefficients (Maximum)	$S_w = 2.49 g$ $S_u = 1.0 g$
Seismic Design Category	E
Analysis Procedure Used	Equivalent Lateral Force Procedure (ASCE 7, 12.8)
Basic Seismic Force Resisting System   Non-Building Structure, ASCE 7-16 Chapter 15	
Response Modification Factor, Signs and Billboards Table 15.4-2	$R = 3.0$
Seismic Response Coefficient	$C = 0.83$
Design Base Shear	$V = C_s w_p$
Flood Design	
When the scoreboard is located in a flood zone other than Zone X, a letter stamped and signed from a Geotechnical Engineer is needed to validate allowable soil values specified in the PC are still applicable.	
Geotechnical Design Data	
Geotechnical Design Based on: 2022 California Building Code, Chapter 18A, Table 1806A.2 (Class 5 Material)	Value
Allowable Soil Bearing Pressure (DL + 1L)	1,500 psf
Design Passive Pressure, $P_p$ (Tabular value has been increased per CBC Section 1806A.3.4 for pier design)	100 pcf
Design Skin Friction, $f_s$	100 psf

11. A GEOHAZARD REPORT IS NOT REQUIRED PER IR A-4.13. IF A SCOREBOARD IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED AND SIGNED BY A GEOTECHNICAL ENGINEER IS REQUIRED VALIDATING THE ALLOWABLE SOIL VALUES, PROVIDE INFORMATION IN TABLE D.

12. PROVIDE A SITE SPECIFIC DESIGN FOR STRUCTURES THAT DO NOT MEET THE MINIMUM SETBACK REQUIREMENTS.

13. PROVIDE A SITE SPECIFIC DESIGN FOR STRUCTURES LOCATED IN AN AREA WITH LIQUEFIABLE SOIL OR SITE CLASS F.

14. FOR WALL MOUNTED ASSEMBLIES (SB6.1), STRUCTURAL ANALYSIS AND JUSTIFICATION THAT THE WALL FRAMING IS CAPABLE OF SUPPORTING THE ASSEMBLY FOR VERTICAL AND LATERAL LOADS.

TABLE A - SCOREBOARD ASSEMBLY WORKSHEET <sup>(1)</sup>			
Nevo Part No.or Description	Part Height [ft.]	Part Width [ft]	Part Weight [lb]
1606-PC-W	8	24	570
ADO 24-2	2	24	106
Total			
TOTAL ASSEMBLY DIMENSIONS & WEIGHT <sup>(2)</sup>			
Total Assembly Height =	_10_ ft. _0_ in.		
Total Assembly Width =	_24_ ft. _0_ in.		
Total Assembly Weight =	_676_ lbs.		
Distance from Finish Grade to Bottom of Sign =	_10_ ft. _0_ in.	Total Height = Total Assembly Height + Distance from Finish Grade to Bottom of Sign =	_20_ ft. _0_ in.

SCOREBOARD ASSEMBLY FOOTNOTES

- Verify part number, dimensions, and weight with Nevo
- See Step 3 of Scoreboard Assembly Worksheet Instructions

## GENERAL NOTES AND MATERIAL SPECIFICATIONS

### GENERAL REQUIREMENTS

1. THE ARCHITECT OR PROFESSIONAL ENGINEER IN GENERAL RESPONSIBLE CHARGE SHALL SIGN AND SEAL ALL DRAWINGS AND SPECIFICATIONS PER TITLE 24, PART 1, SECTIONS 4-316(E) AND 4-317 (H).
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA, OR CONSTRUCTION CHANGE DOCUMENTS APPROVED BY THE DIVISION OF THE STATE ARCHITECT (DSA), AS REQUIRED BY TITLE 24, PART 1, SECTION 4-338.
3. THE DISTRICT SHALL EMPLOY A CLASS 2 PROJECT INSPECTOR WHEN OVERALL STRUCTURE HEIGHT IS 35 FEET OR GREATER, OTHERWISE A CLASS 3 PROJECT INSPECTOR MAY BE USED. THE PROJECT INSPECTOR SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK , AND SHALL SUBMIT VERIFIED REPORTS ON A DSA-6 FORM. THE DUTIES OF THE PROJECT INSPECTION ARE DEFINED IN TITLE 24, PART 1, SECTION 4-342.
4. ALL SCOREBOARD CONTROLS SHALL BE FULLY ACCESSIBLE VIA WIRELESS CONTROL OR COMPLETE DESIGN SHALL BE DEMONSTRATED IN THE SITE-SPECIFIC APPLICATION.
5. ALL ASSEMBLIES SHALL HAVE ELECTRICAL DISCONNECT PER CEC 600.6 AND BE ELECTRICALLY GROUNDED PER CEC 600.7, SEE DETAIL B/SB5.1
6. IN FLOOD ZONES, LOCATION OF ELECTRICAL ELEMENTS SHALL CONFORM TO ASCE 24, SECTION 7.2 PER DSA PR-14-01 SECTION 1.2.1.
7. SEE PAGE, SB0.2, FOR ALL MATERIAL SPECIFICATIONS AND NOTES.
8. PROJECT DESIGN PROFESSIONAL OF RECORD IS RESPONSIBLE FOR PREPARATION OF THE PROJECT SPECIFIC DSA 103 AND IS RESPONSIBLE FOR ALL SHOP DRAWING AND SUBMITTAL REVIEWS. SEE SB0.3 FOR EXAMPLE DSA 103



STRUCTURAL NOTES

GENERAL NOTES

1.

The following notes, typical details and schedules shall apply to all phases of this project unless otherwise shown or noted.

2.

Specific notes and details shall take precedence over general notes and typical details.

3.

All materials and workmanship shall conform to the minimum standards of the 2022 edition Title 24 of the California Building Code (CBC) and such other regulating agencies exercising authority over any portion of the work. The contractor shall have a current copy of the CBC on the job site.

4.

The "Contract or Construction Documents" shall consist of these notes, details, schedules, plans, and drawings.

5.

All specifications, including but not limited to materials and products, shall be those put forth in the "Contract or Construction Documents". No substitutions shall be permitted to be used or assumed to be used in the bidding or construction process without written approval by the Structural Engineer of Record.

6.

The contractor shall examine the "Contract or Construction Documents" and shall notify the Architect or Structural Engineer of Record of any discrepancies he may find before proceeding with the work.

7.

All information on existing conditions shown on drawings are based on best present knowledge available, but without guarantee of accuracy. The Contractor shall verify and be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.

8.

The Contractor shall immediately notify the Architect or Structural Engineer of Record of any condition which in his opinion might endanger the stability of the structure or cause distress of the structure.

9.

All work shall conform to the best practice prevailing in the various trades comprising work. The Contractor shall be responsible for coordinating the work of all trades.

10.

These "Contract or Construction Documents" represent the finished structure, and do not indicate the method of construction. The Contractor shall supervise and direct the work and shall be solely responsible for construction means, methods, techniques, sequences and procedures.

11.

Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.

A. Labeling (as required or specified) shall be provided in accordance with CBC Section 1703A.5.

B. Evaluation and follow-up inspection services (as required or specified), shall conform to CBC Section 1703A.6.

12.

The Contractor shall provide temporary bracing and shoring for all structural members as required for structural stability of the structure during all phases of construction.

13.

The Contractor shall take all steps necessary to ensure proper alignment of the structure after the installation of all structural and finish materials. This shall include any necessary preloading of the structure to determine final position of the completed work.

14.

Observation visits to the project site by field representatives of Architect and/or Structural Engineer of Record (support services) shall not include inspections of safety or protective measures, nor construction procedures, techniques or methods. Any support services performed by Architect or Structural Engineer of Record during any phase of construction, shall be distinguished from continuous and detailed inspection services (as required by any regulating governmental agency, e.g. the Authority Having Jurisdiction) provided by others. These support services, whether of material or work, are performed solely for the purpose of assisting in quality control and in achieving conformance with contract documents, but do not guarantee Contractor's performance and shall not be construed as supervision of construction.

15.

These notes, details, drawings and specifications (Contract or Construction Documents) do not carry necessary provisions for construction safety. These documents and all phases of construction hereby contemplated are to be governed, at all times, by applicable provisions of the current California Occupational Safety and Health Act.

16.

Where any conflict occurs between the requirements of federal, state and local laws, codes, ordinances, rules and regulations, the most stringent shall govern.

17.

Written dimensions shall have precedence over scaled dimensions.

18.

Drawings (notes, schedules, details and plans) shall have precedence over Structural Calculations.

19.

In the event that certain features of the construction are not fully shown on the drawings or called for in the General Notes or Specifications, then their construction shall be of the same character as for similar conditions that are shown or called for.

20.

ASTM designation and all standards refer to the latest amendments.

21.

These structural "Contract or Construction Documents" shall not be modified without prior written approval of the Structural Engineer of Record.

22.

Only structural working drawings approved by the Division of the State Architect are permitted to be used for construction on this project. All other drawings or documents are obsolete and are not permitted on the job site, nor shall they be used for any construction purposes. Contractors using unapproved drawings or documents are solely responsible for all work not performed in accordance with the "approved" drawings.

23.

A Division of the State Architect certified project inspector employed by the District (Owner) and approved by the Division of the State Architect shall provide continuous inspection of the work. The duties of the inspector are defined in Section 4-342, Part 1, Title 24 California Code of Regulations.

FOUNDATION NOTES

1.

Basic: See Structural Design Values Chart, Sheet SB0.1 Table B

2.

Unexpected soil conditions: Allowable values and foundation design are based upon the minimum values provided in Table 1806A.2 of the 2022 California Building Code. See SB0.1 for values

3.

Excavate to required depths and dimensions (as indicated in drawings), cut square and smooth with firm level bottoms. Care shall be taken not to over-excavate foundation at lower elevation and prevent disturbing of soils around higher elevation.

4.

Footings shall be poured in neat excavations, without side forms whenever possible.

5.

Carry all foundations to required depths into compacted fill or natural soil (as per Structural Plans and Details).

6.

All foundation excavations shall be inspected and approved by the Inspector of Record or Geotechnical Engineer prior to forming and placement of reinforcing or concrete.

7.

Foundations shall not be poured until all required reinforcing steel, sleeves, inserts, conduits, pipes, etc. and formwork is properly placed and inspected by the Authority having Jurisdiction.

8.

The sides and bottoms of excavations which are to have concrete contact must be moistened several times just prior to pouring upon them.

9.

De-water footings, as required, to maintain dry working conditions.

REINFORCING STEEL

1.

All reinforcing steel shall be deformed intermediate grade bars conforming to ASTM A615, Grade 60 (f<sub>y</sub> = 60 ksi) unless noted otherwise.

2.

Reinforcing steel shall not be welded, unless specifically noted otherwise.

3.

To hold reinforcing bars in their true position and prevent displacement, standard tie and anchorage devices must be provided. Placing of reinforcement shall conform to ACI 318-19 Section 26.6.2.

4.

Shop drawings for fabrication of any reinforcing steel shall be approved by Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record, for their review, prior to fabrication.

5.

Refer to typical details for minimum splice length and minimum radius of bend of reinforcing steel.

6.

All reinforcing steel splices shall be staggered 24", unless specifically noted or detailed otherwise.

7.

All reinforcing bar bends shall be made cold.

8.

Fabrication, erection and placement of reinforcing steel shall conform to Concrete Reinforcing Steel Institute of Standard Practice.

9.

Reinforcing steel shall be clean of rust, grease or other material likely to impair bond.

CONCRETE

1.

All concrete shall have a minimum ultimate compressive strength (f<sub>c</sub>) as outlined below at 28 days. All concrete shall be regular weight (unless specifically noted otherwise).

A. Concrete for footings: 4,500 psi w/c = 0.45 max.

2.

Maximum Fly Ash content shall be 15% by weight, of total cementitious materials and shall conform to ASTM C618.

3.

All concrete work shall comply with CBC Chapter 19A and ACI 318-19 and latest edition of ACI Manual of Concrete Practice.

4.

Special Inspection (as required or specified) shall conform to CBC Chapter 17A.

5.

Cement shall be portland cement Type V and shall conform to ASTM C150.

6.

Aggregates shall conform to ASTM C33, provide aggregates from a single source.

7.

Water shall conform to ASTM C94 and be potable.

8.

Where not specifically detailed, the minimum concrete cover on reinforcing steel shall be:

A. Concrete cast against and permanently exposed to earth or weather: 3"

10.

All reinforcing steel, anchor bolts, dowels, inserts and any other hardware to be set in concrete shall be well secured in position prior to pouring of concrete.

11.

Vibrate all concrete as it is placed, with a mechanical vibrator operated by experienced personnel. The vibrator shall be used to consolidate the concrete, not transport it. Reinforcing and forms shall not be vibrated.

12.

Formwork design and removal shall conform to ACI 318-19 Section 26.11. Remove forms in accordance with the following minimum schedule:

A. Side forms of footings: Minimum 48 hours

B. Column and pier forms: 72 hours & 70% of design strength

15.

Concrete shall not free fall more than six feet. Use tremie, pump or other approved methods.

16.

Concrete shall be maintained in a moist condition for a minimum of 5 days after placement.

17.

The Contractor may use concrete admixtures as a construction means and methods to execute "Contract or Construction Documents". Use of admixture is solely the responsibility of the Contractor.

18.

Mix designs shall be prepared by an approved testing laboratory, signed by a licensed engineer and shall be submitted to the Project Specific Design Professional of Record for approval. SSG is not responsible for review or approval of site specific concrete mix design.

19.

Only one grade of concrete shall be allowed on project site at any one time

20.

Concrete strength shall be verified by standard cylinder tests (in accordance with CBC Section 1905A.1.16) made by an approved testing laboratory.

21.

Concrete placed when the air temperature has fallen to, or is expected to fall below 40° shall conform to ACI 318-19 Section 26.5.4, and ACI 306R-16.

22.

Concrete placed during hot weather shall conform to ACI 318-19 Section 26.5.5, and ACI 305R-14.

23.

Conduits and sleeves placed within structural concrete shall not be tied directly to structural reinforcement.

A. 1" concrete cover shall be maintained around all reinforcement.

24.

No stakes shall be permitted within the footing section.

25.

Concrete shall reach minimum 75% design strength or cure for 3 days minimum prior to installation of steel columns and scoreboard components.

DRILLED CAISSON/PIER AND GRADE BEAM NOTES

1.

Excavations for drilled caissons/pier shall be performed in compliance with local grading codes and ordinances as well as CBC Chapters 18A and 33A.

2.

Provide Special Inspection in accordance with CBC Section 1705A.8 and Table 1705A.8.

3.

Excavations for all drilled caissons/piers shall be approved by the Project Geotechnical Engineer or Project Special Inspector prior to placing of concrete.

4.

Reinforcement for drilled caissons/pier shall be approved by the Structural Engineer of Record prior to placing in caisson/pier excavation.

5.

De-water caisson/pier footings and building excavation as required to maintain dry working conditions.

6.

Caisson/piers are to be poured within 24 hours after completion of drilling operation. Shoring requirements shall be determined by contractor. Contractor shall provide fall protection and safety barriers at and near the drilled hole as required by OSHA and the Authority Having Jurisdiction.

7.

The Contractor shall be responsible for all shoring, bracing, etc. necessary to support cut and/or fill banks, and existing structures during excavation, and the forming and placement of concrete.

8.

Bottom of caissons/piers shall be thoroughly cleaned prior to placement of concrete.

STRUCTURAL STEEL AND WELDING

1.

All structural steel construction shall conform to AISC 360-16 and AISC 341-16.

A. Fabrication of all structural steel shall be done in the shop of an approved fabricator. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.

2.

All structural steel shall conform to the following specifications:

A. Angles, channels, plates, bars, rounds, and other miscellaneous shapes: Shall conform to ASTM A36 and shall have a minimum yield stress (F<sub>y</sub>) of 36 ksi.

B. Wide-flange shapes: Shall conform to ASTM A992 and shall have a minimum yield stress (F<sub>y</sub>) of 50 ksi.

C. Structural tubes: Shall be ASTM A500, Grade C, and shall have a min. yield stress (F<sub>y</sub>) of 50ksi.

3.

All structural-steel fasteners shall conform to the following specifications:

A. Bolts shall conform to ASTM A307

B. Anchor Bolts shall conform to ASTM F1554, Grade as noted in drawings

C. Carbon steel nuts shall conform to ASTM A563

D. Stainless steel nuts shall conform to ASTM F594

E. Washers shall conform to ASTM F436

4.

Special Inspection shall be provided for all structural steel and welding, in accordance with CBC Chapter 17A.

5.

All structural steel shall be fabricated, erected and welded in accordance with AISC Specifications for Structural Steel Buildings (AISC 360-16) and Code of Standard Practice for Steel Buildings and Bridges (AISC 303-16).

6.

All welding shall be done by qualified and certified welders.

7.

Shop drawings for the fabrication of any structural steel shall be approved by the Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record for their review, prior to fabrication.

8.

No holes other than those specifically detailed shall be allowed through structural steel members. Burning of holes is not permitted.

9.

All welding shall conform to 'AWS D1.1' specifications for welding. (E-70XX Electrodes).

10.

Where fillet weld size is not indicated, use 'AWS' minimum size based on the thickness of the thinner part being welded, as specified in AISC Specifications for Structural Steel Buildings (AISC 360-10), Section J2.2.

11.

All butt welds to be complete joint penetration, unless specifically noted otherwise.

12.

Welder qualification requirements, welding procedure and welding electrodes for all structural steel (except structural sheet steel, see steel decking) shall conform to CBC Sections 1705A.2.1 and 2204A.1.

13.

Provide 3" minimum concrete cover around all structural steel below grade.

14.

Structural steel embedded into concrete shall be uncoated.

15.

Structural steel shall be hot-dip galvanized (minimum ASTM A123 or A153 Class D) or painted with zinc-rich primer, undercoat, and finish coat; or equivalent paint system.

16.

All exposed steel fasteners, including cast-in-place anchor bolts/rods, shall be stainless steel (Type 304 minimum), hot-dip galvanized (ASTM A153, Class D minimum or ASTM F2329), or protected with corrosion preventive coating that demonstrated no more than 2% of red rust in minimum 1,000 hours of exposure in salt spray test per ASTM B117. Zinc plated fasteners do not comply with this requirement.

ABBREVIATIONS

A.B.

Anchor Bolt

ABV.

Above

ACI

American Concrete Institute

HORIZ.

Horizontal

ADJ.

Adjacent

HSS

Hollow Steel Section

AHJ

Division of the State Architect

HT.

Height

ASC

American Institute of Steel Construction

ICC

International Building Code

AOR

Architect of Record

ICC

International Code Council

APPROX.

Approximately

ID

Inside Diameter

ASCE

American Society of Civil Engineers

IN.

Inch, Inches

ARCH.

Architect, Architecture

INT.

Interior

ASTM

American Society of Testing and Materials

ksi

Kips per Square Inch

ATR

All Thread Rod

LL

Live Load

AWS

American Welding Society

MAX.

Maximum

B.O.

Bottom of \_\_\_\_\_

MB

Machine Bolt

BOT.

Bottom

MFR.

Manufactured, Manufacturer

b/t

Between

MIN.

Minimum

MPH

Miles per Hour

CAC

California Administrative Code

N/R

Not Required

CBC

California Building Code

N.T.S.

Not to Scale

CIP

Cast-in-place

CJP

Complete Joint Penetration

o.c.

On Center

CL

Centerline

o/

Over

CLR

Clear

OD

Outside Diameter

COL.

Column

CONC.

Concrete

PEN.

Penetration

CONN.

Connection

PL

Plate

CONST.

Construction

PJP

Partial Joint Penetration

CONT.

Continue, Continuous

psi

Pounds per Square Inch

PSF

Pounds per Square Foot

Ø

Diameter

DBL

Double

REBAR

Reinforcing Bar

DET.

Detail

REINF.

Reinforcement

DL

Dead Load

REQ'D

Required

DSA

Division of State Architect

DWGS.

Drawings

S.F.

Square Feet

EA

Each

SHT

Sheet

E.F.

Each Face

SIM.

Similar

ELEC.

Electric, Electrical

SMS

Sheet Metal Screw

ELEV.

Elevation

SQ.

Square

EMBED.

Embedded, Embedment

STAGCD

Staggered

EQ.

Engineer of Record

STD.

Standard

EQUIP.

Equipment

STL

Steel

E.S.

Each Side

SEOR

Structural Engineer of Record

E.W.

Each Way

EXT.

Exterior

T.O.

Top of \_\_\_\_\_

TYP.

Typical

FAB.

Fabricated

FDN.

Foundation

U.N.O.

Unless Noted Otherwise

F.G.

Finish Grade

F.O.

Face of \_\_\_\_\_

VERT.

Vertical

FRMG.

Framing

VIF

Verify in Field

FT.

Foot-Feet

FTC.

Footing

w/

With

w/c

Water/Cement Ratio

GA.

Gauge

WELDED

Welded Steel Stud

GALV.

Galvanized

WT.

Weight

GEOR.

Geotechnical Engineer of Record

POST INSTALLED ANCHOR & TESTING

1.

At post-installed anchors are to be tension tested with the exception that torque testing is allowed if the anchors are specifically designed as torque controlled

2.

Test quantity of post-installed anchors as noted below:

Application	Quantity
Non-structural (Equipment Anchorage, etc.)	50%
Structural	100%

3.

Apply proof test loads to anchors without removing the nut if possible. If not, remove nut and install a threaded coupler to the same tightness of the original nut using a torque wrench and apply load.

4.

All tests shall be performed in the presence of the inspector.

5.

Reaction loads from test fixtures may be applied close to the anchor being tested, provided the anchor is not restrained from withdrawing or restricted from a concrete shear cone type failure mechanism.

6.

Test equipment is to be calibrated by an approved testing laboratory in accordance with standard recognized procedures.

7.

The following criteria apply for the acceptance of installed anchors:

A. Hydraulic ram method: anchors tested with a hydraulic jack or spring loaded devices shall maintain the test load for a minimum of 15 seconds and shall exhibit no discernable movement during the tension test, e.g. as evidenced by loosening of the washer under the nut.

B. Torque wrench method: anchors tested with a calibrated torque wrench must attain the manufacturer recommended torque within ½ turn of the nut.

Exceptions:

- Wedge or sleeve type: one-quarter turn of the nut from 3/8" sleeve anchor only.
- Threaded type: one-quarter turn of the screw after initial seating of the screw head.

8.

If any anchor fails testing, test all anchors of the same type not previously tested until twenty consecutive anchors pass, then resume the initial test frequency. If the anchors are used for the support and bracing of non-structural components (pipe, duct or conduit), the twenty shall be only those anchors installed by the same trade.

9.

Test loads per ICC ESR, IAPMO, OR UES report

10.

When installing drilled-in anchors and/or powder driven pins in existing non-prestressed reinforced concrete, use care and caution to avoid cutting or damaging the existing reinforcing bars. When installing them into existing prestressed concrete (pre- or post-tensioned) locate the prestressed tendons by using a non-destructive method prior to installation. Exercise extreme care and caution to avoid cutting or damaging the tendons during installation. Maintain a minimum clearance of one inch between the reinforcement and the drilled-in anchor and/or pin.

ANCHOR TORQUE TEST VALUES

Anchor Diameter	CONCRETE		MASONRY	
	HILTI KB TZ 2	SIMPSON STRONG BOLT Z	HILTI KB TZ 2	SIMPSON STRONG BOLT Z
	ESR-4266	ESR-3037	ESR-4561	ER-240
3/8"	30 ft-lb	30 ft-lb	15 ft-lb	20 ft-lb
1/2"	50 ft-lb	60 ft-lb	25 ft-lb	35 ft-lb
5/8"	40 ft-lb	90 ft-lb	30 ft-lb	55 ft-lb
3/4"	110 ft-lb	150 ft-lb	50 ft-lb	100 ft-lb

If the manufacturer's recommended installation torque is less than the test torque noted in the table, the manufacturer's recommended installation torque should be used in lieu of the tabulated values.

See manufacturer's ESR report for Maximum Impact Wrench Torque Rating.

APPLICATION # 02-121895

IDENTIFICATION STAMP

DIV. OF THE STATE ARCHITECT

APP: 02-121895 INC:

REVIEWED FOR

SS ☒ FLS ☒ ACS ☐

DATE: 01/24/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER

STATE OF CALIFORNIA

NO. 5405

DATE RECD: 08.09.2023

PC SEOR SEAL

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APPROVED

DIV. OF THE STATE ARCHITECT

APP: 04-122347 PC

REVIEWED FOR

SS ☒ FLS ☒ ACS ☒ CG ☐

DATE: 09/20/2023

OSA STAMP

PRE-CHECK (PC) DOCUMENT

CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER

STATE OF CALIFORNIA

NO. 52386

EXPIRY: 08/31/2024

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

SSG JOB # S23109

SHEET SB0.2



# SB0.3

SHEET INFORMATION	
DATE	08.09.2023
DRAWN	JMK

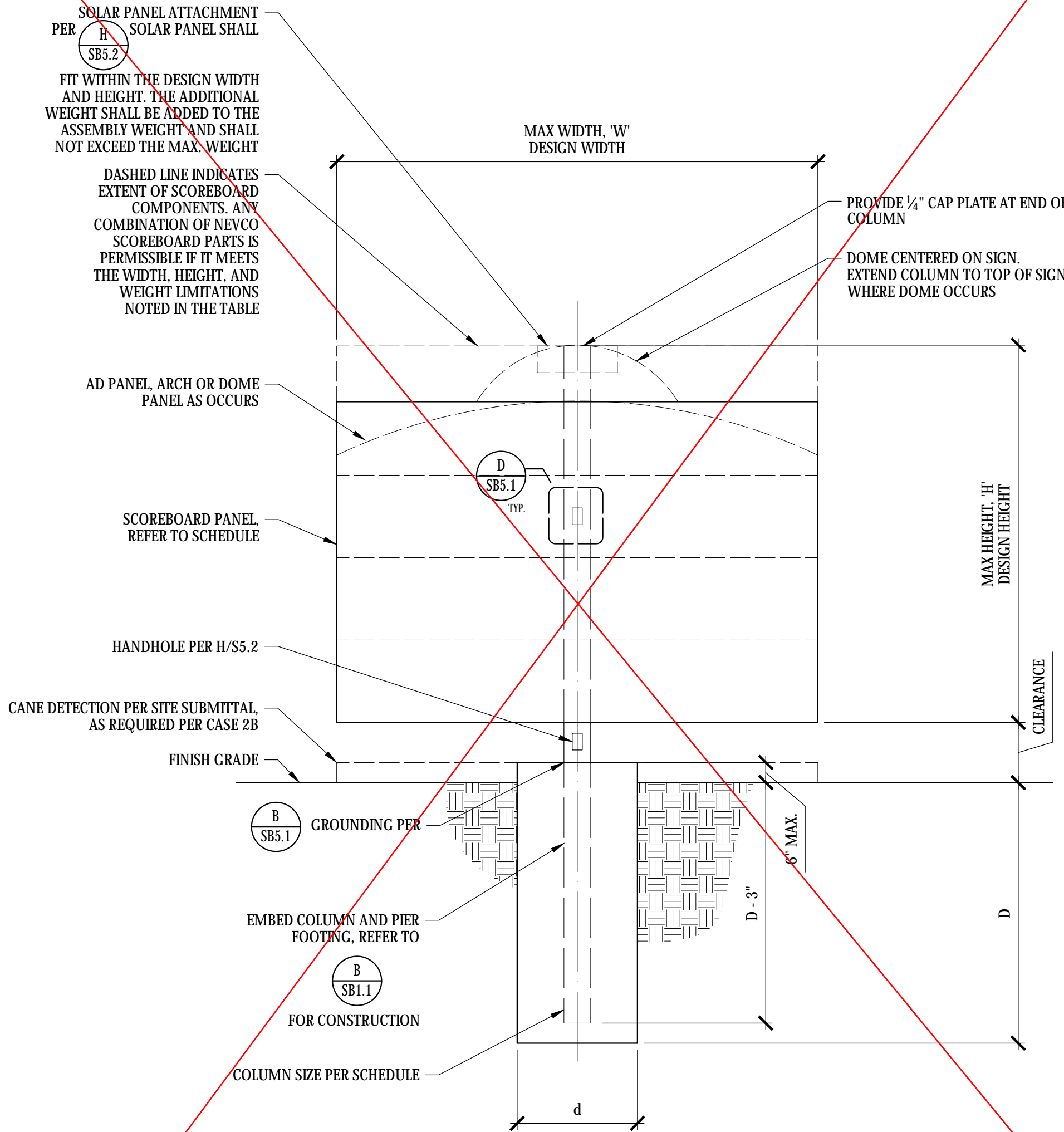


ONE COLUMN ASSEMBLY - CASE 1							
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	6'-6"	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	9'-0"	N/R

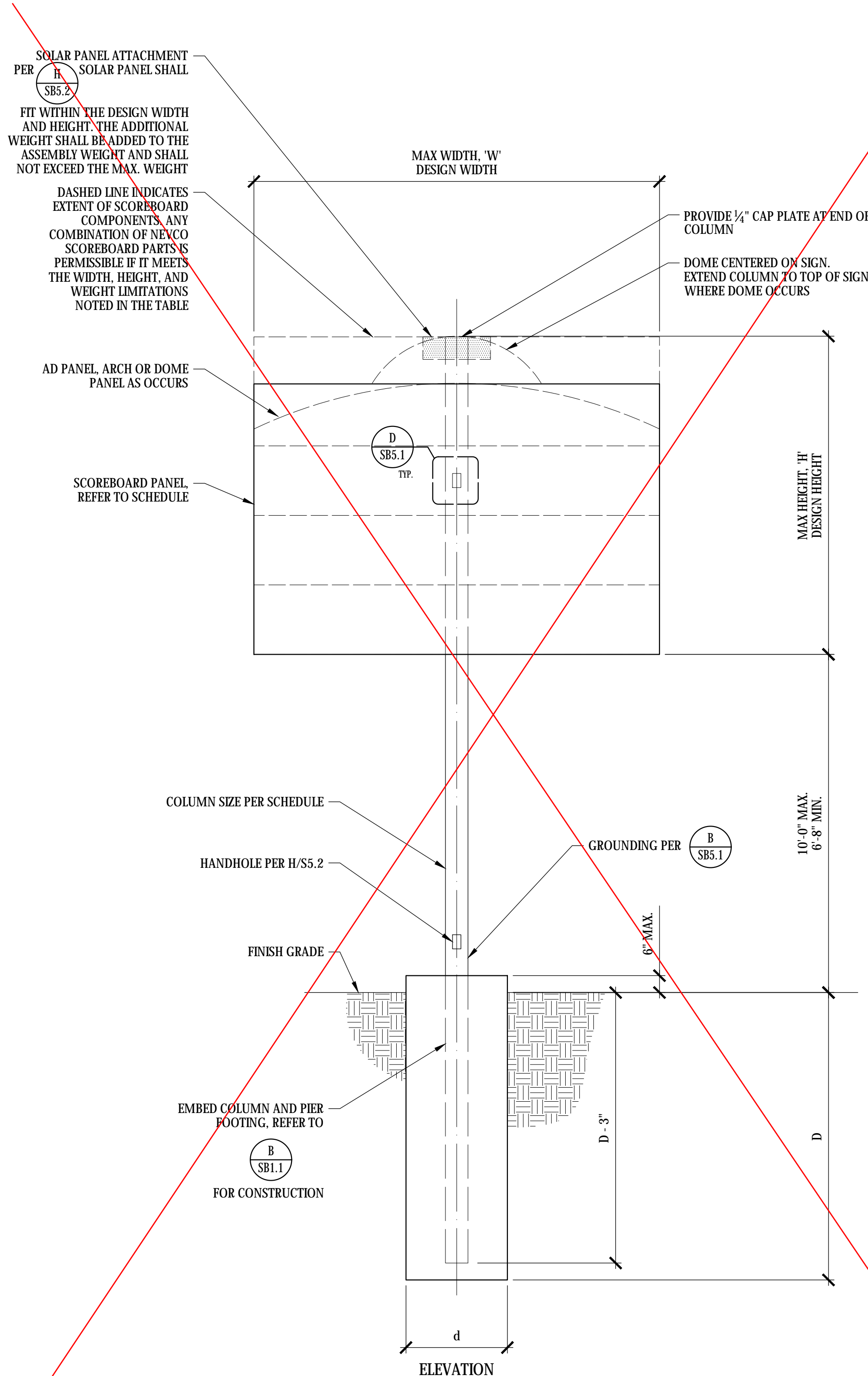
NOTES:  
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06

ONE COLUMN ASSEMBLY - CASE 2A <sup>(2)</sup> AND CASE 2B <sup>(2)(3)</sup>							
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	5'-3"	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	6'-6"	N/R

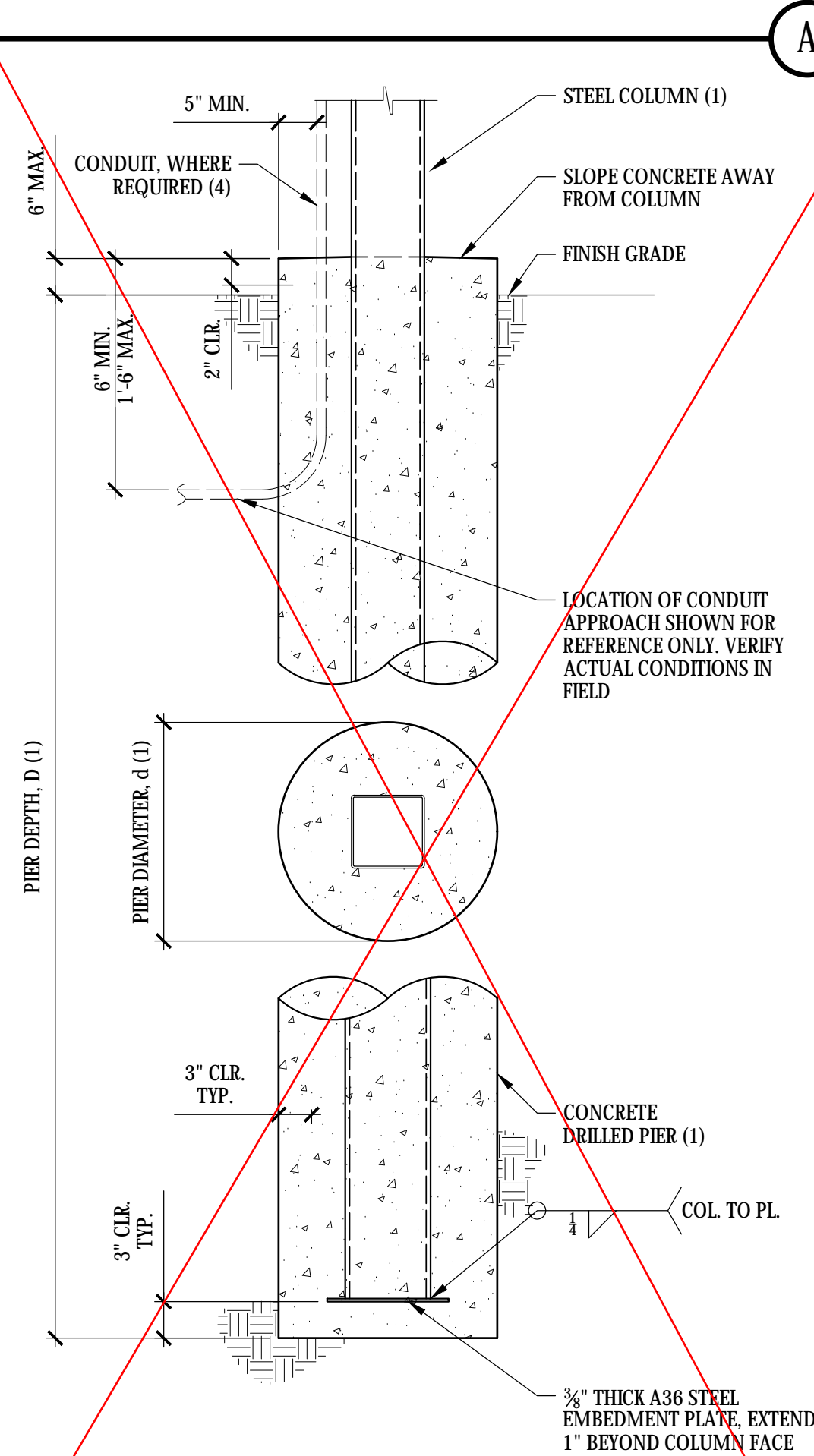
NOTES:  
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06  
2. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
3. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD



ELEVATION  
CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"  
CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3' < CLEARANCE < 6'-8"



ELEVATION  
CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE



NOTES: (4)  
1/ SEE ELECTRONIC SIGN ELEVATION, A/S81.1  
2. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2)-2\"/>

ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION  
N.T.S.

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-121895 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

structural engineers

PC SEOR REAL  
DATE SIGNED: 08.09.2023

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SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 09/20/2023

DSA STAMP  
PRE-CHECK (PC) DOCUMENT  
CODE: 2022  
A separate project application for construction is required.

MARQUEE  
CAISSON -  
EMBEDDED

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB1.1

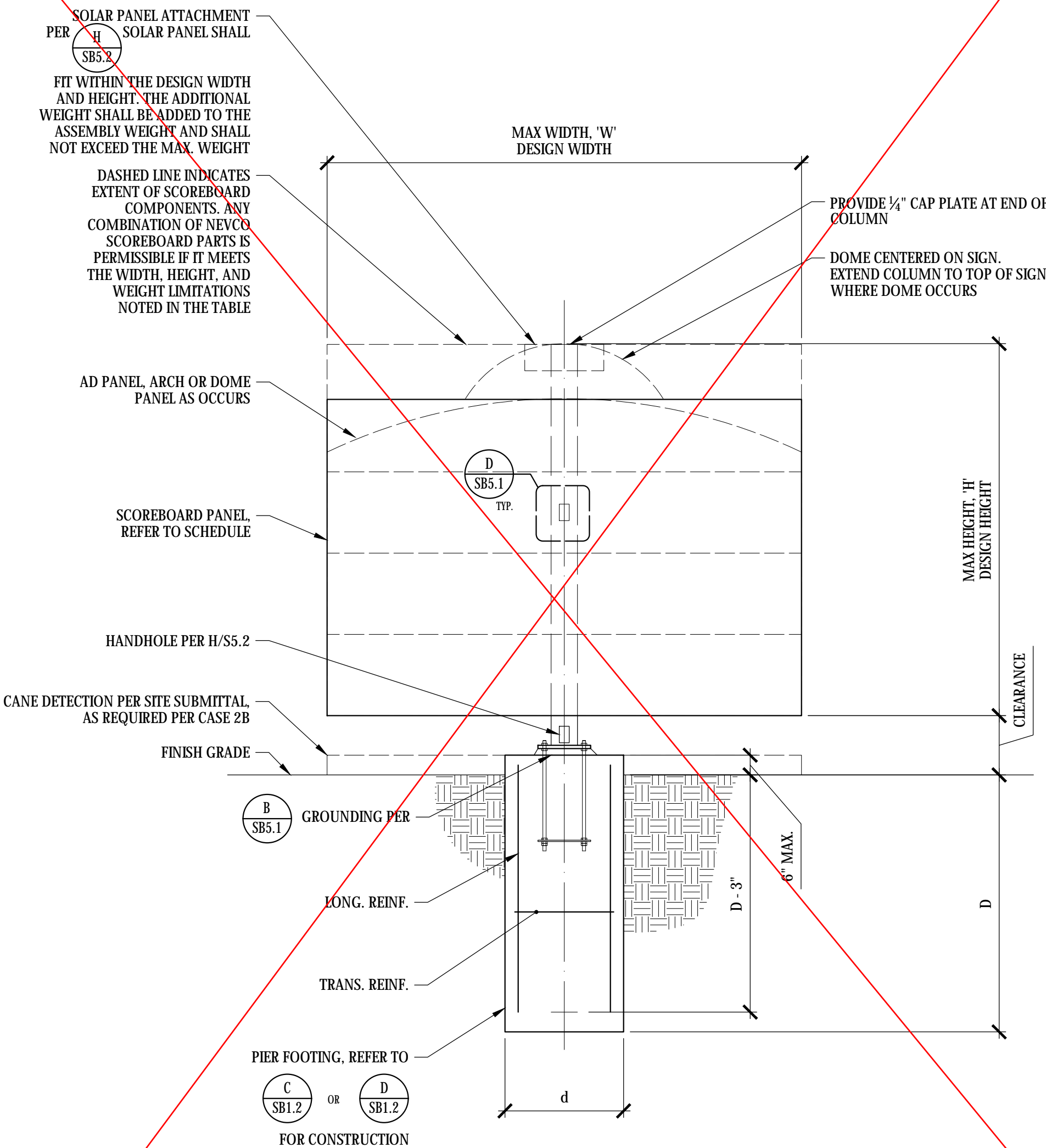


ONE COLUMN ASSEMBLY - CASE 1							
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	6'-6"	8 - #6
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	9'-0"	8 - #6

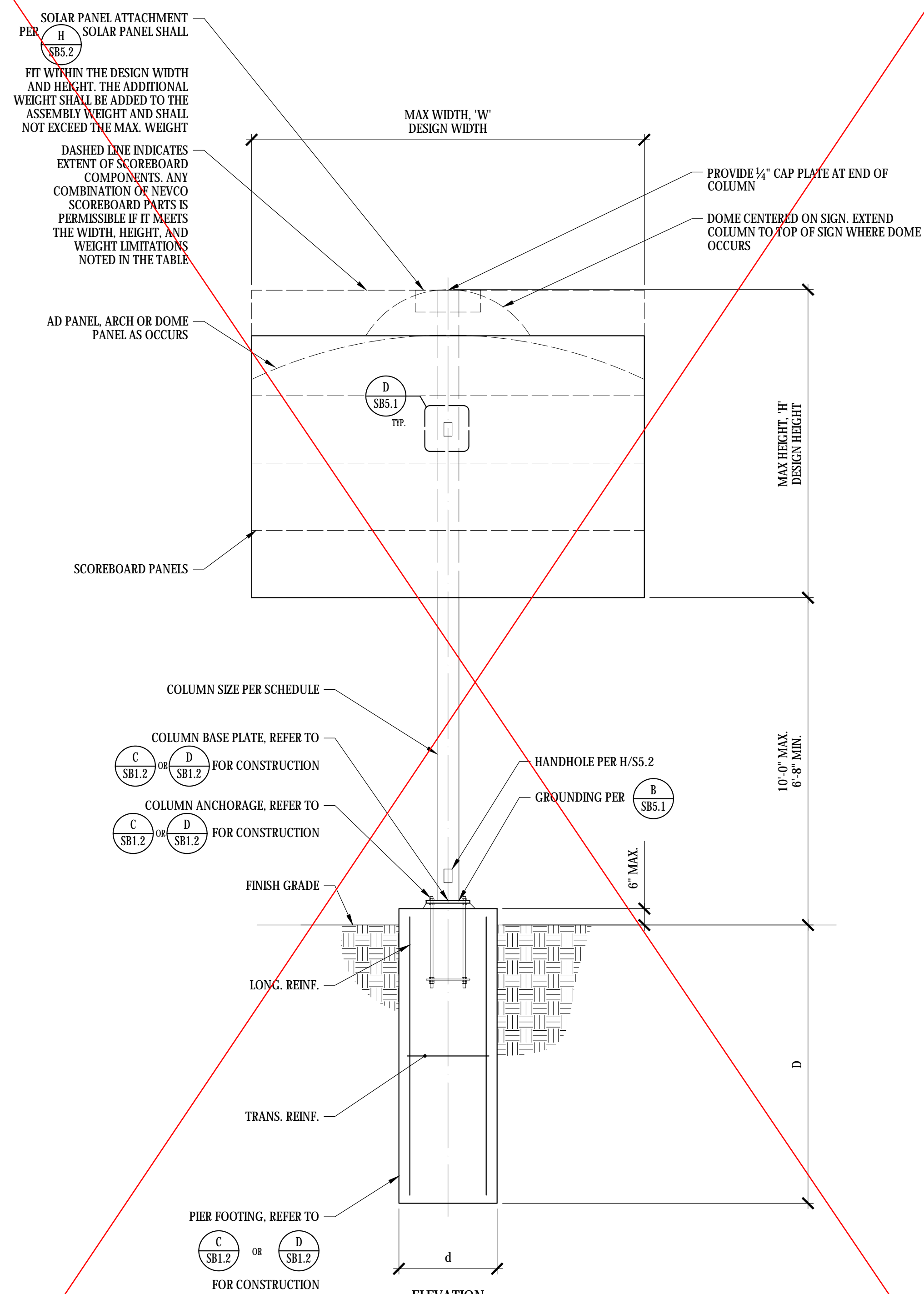
NOTES: (#)  
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.  
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.

ONE COLUMN ASSEMBLY - CASE 2A <sup>(3)</sup> AND CASE 2B <sup>(3)(4)</sup>							
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	5'-3"	8 - #6
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	6'-6"	8 - #6

NOTES: (#)  
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.  
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.  
3. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
4. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD.



CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"  
CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3" < CLEARANCE < 6'-8"



CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE

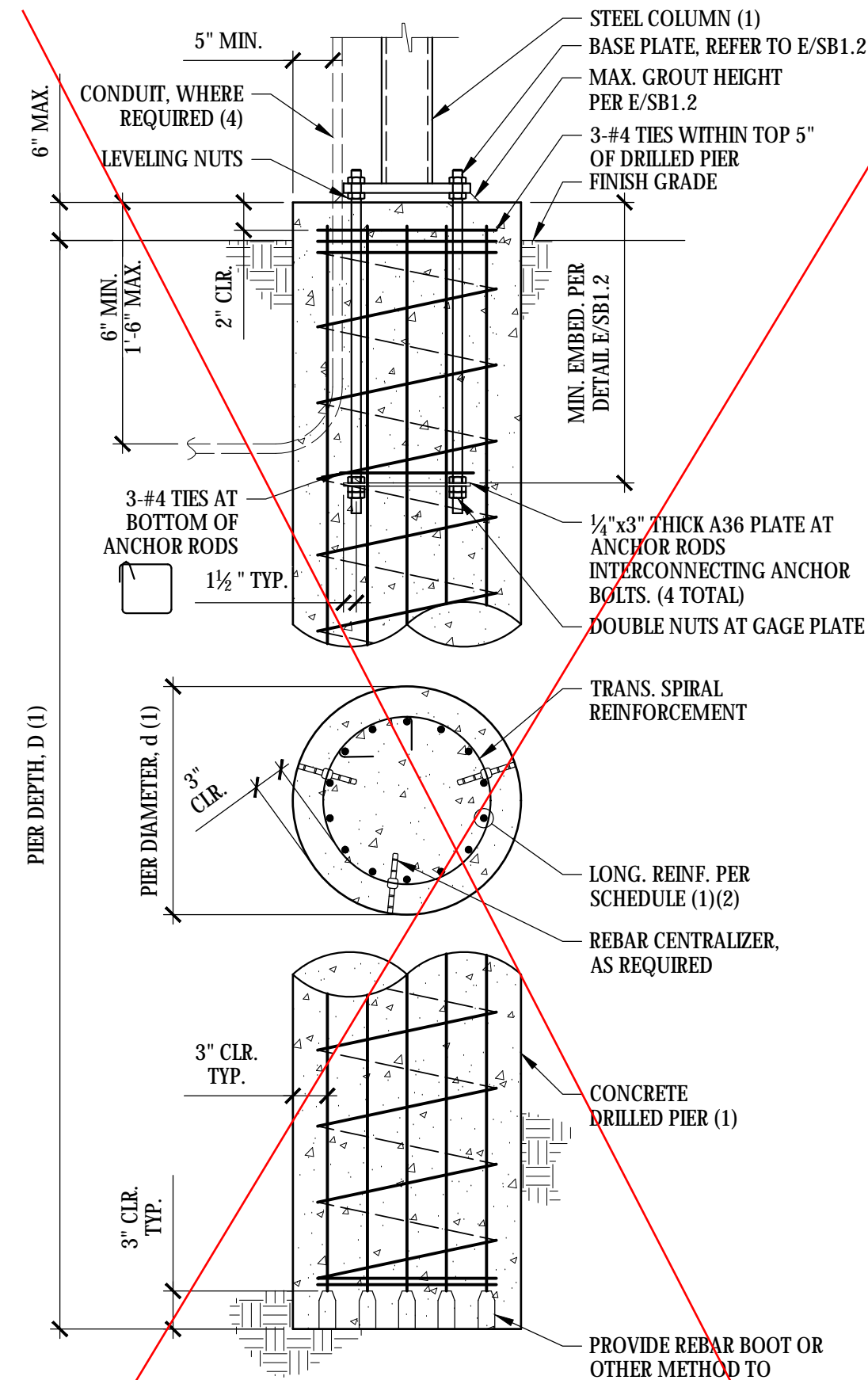
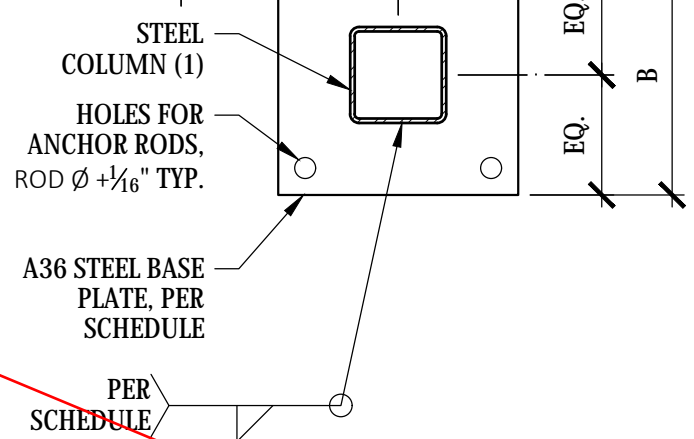
## ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

NTS.

COLUMN SIZE	BASE PLATE				WELD	ANCHOR ROD			
	THICKNESS, t	WIDTH, B	LENGTH, L	GRADE		QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	EMBED.
HSS8x8x $\frac{3}{8}$	1"	16"	16"	3/8" FILLET		4-1 $\frac{1}{2}$ "Ø	F1554, A36	2 $\frac{1}{2}$ "	1 $\frac{1}{2}$ "

NOTES: (#)  
1. SEE SCOREBOARD ELEVATION, A/SB1.2  
NTS.

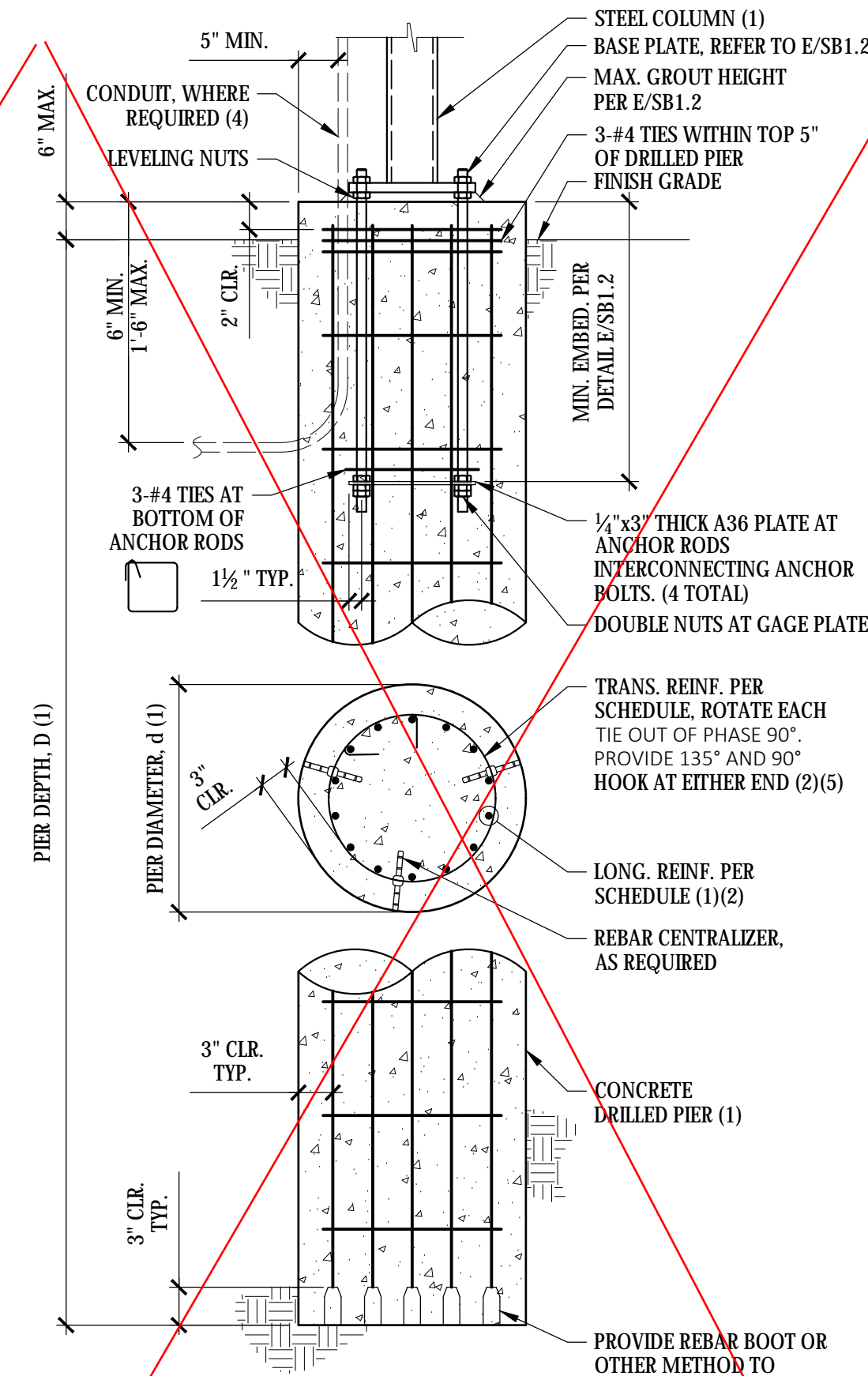
### BASE PLATE



NOTES: (#)  
1. SEE ELECTRONIC SIGN ELEVATION, A/SB1.2  
2. SEE DETAILS B/SB1.2 FOR REINFORCEMENT BEND & TIE REQUIREMENTS  
3. DO NOT SPLICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2)-2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.

### CONCRETE DRILLED PIER

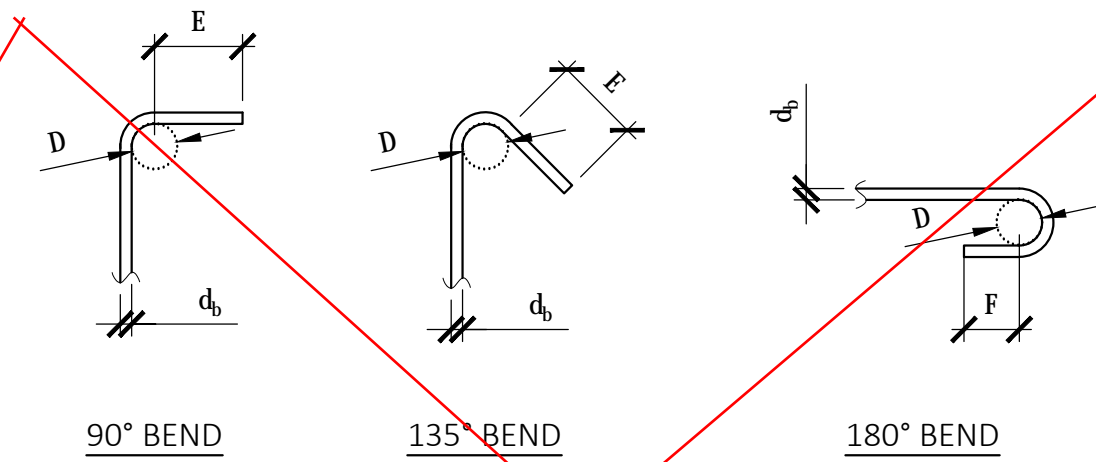
NTS.



NOTES: (#)  
1. SEE ELECTRONIC SIGN ELEVATION, A/SB1.2  
2. SEE DETAILS B/SB1.2 FOR REINFORCEMENT BEND & TIE REQUIREMENTS  
3. DO NOT SPLICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2)-2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. THE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

### CONCRETE DRILLED PIER

NTS.



Bar Size	Dimension of Standard Bends		
	D	E	(6d) [3" min.]
#3	1 $\frac{1}{2}$ "Ø	3"	3"
#4	2"Ø	3"	3"
#5	2 $\frac{1}{2}$ "Ø	3 $\frac{3}{4}$ "	3 $\frac{3}{4}$ "

### TIE AND STIRRUP BENDS

NTS.

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APP: 02-121895 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

SSG  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. PARSONS  
No. 5485  
STATE OF CALIFORNIA  
DATE SIGNED: 08.09.2023  
PC SEOR REAL

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DIV. OF THE STATE ARCHITECT  
APP: 04-122347 PC  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application  
for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
GAILFORD R. RAJIC  
No. 52356  
EXPIRES: 08/31/2025  
STATE OF CALIFORNIA

MARQUEE  
CAISSON -  
BOLTED

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

SSG JOB # S23109

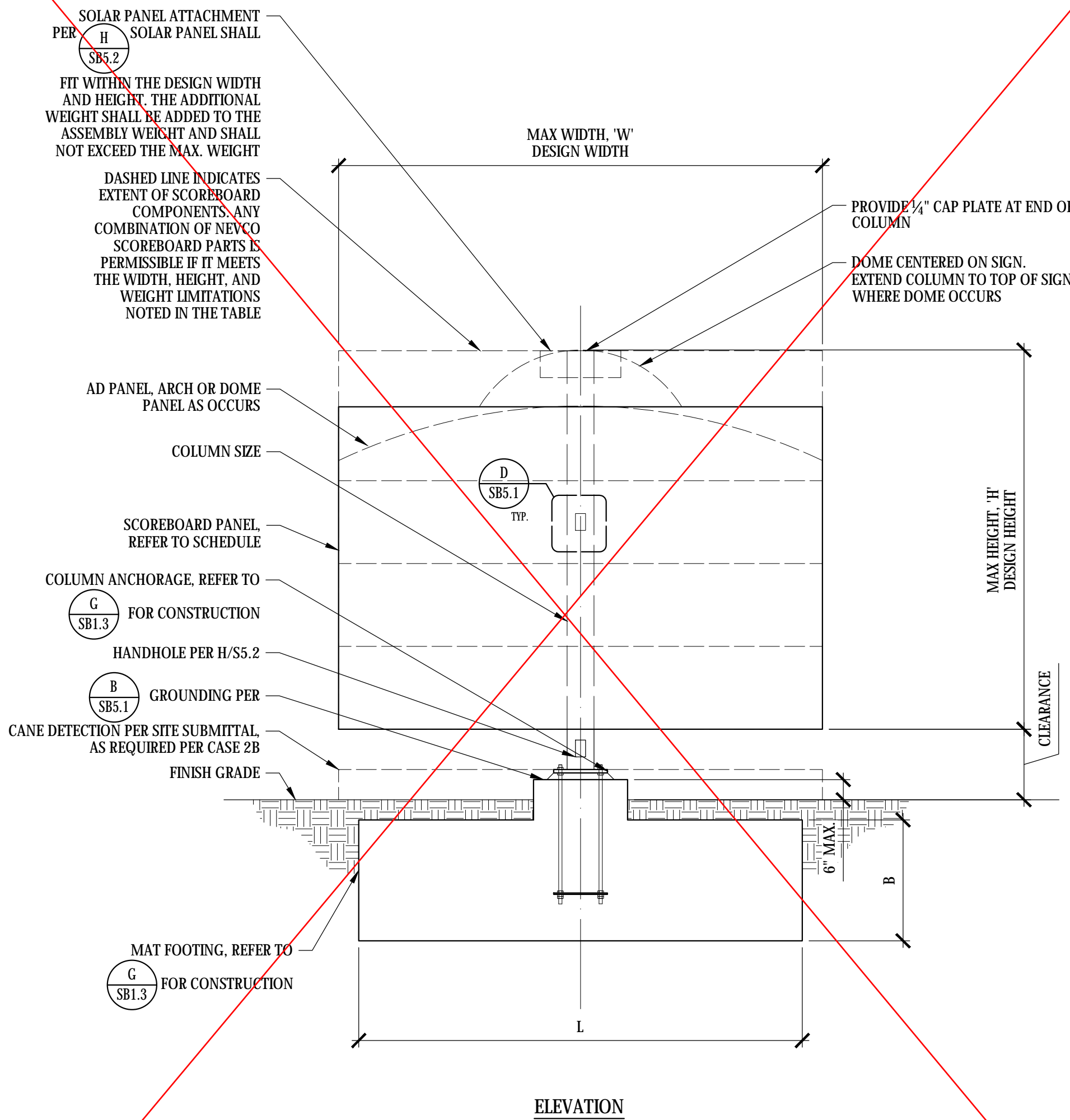
SHEET SB1.2



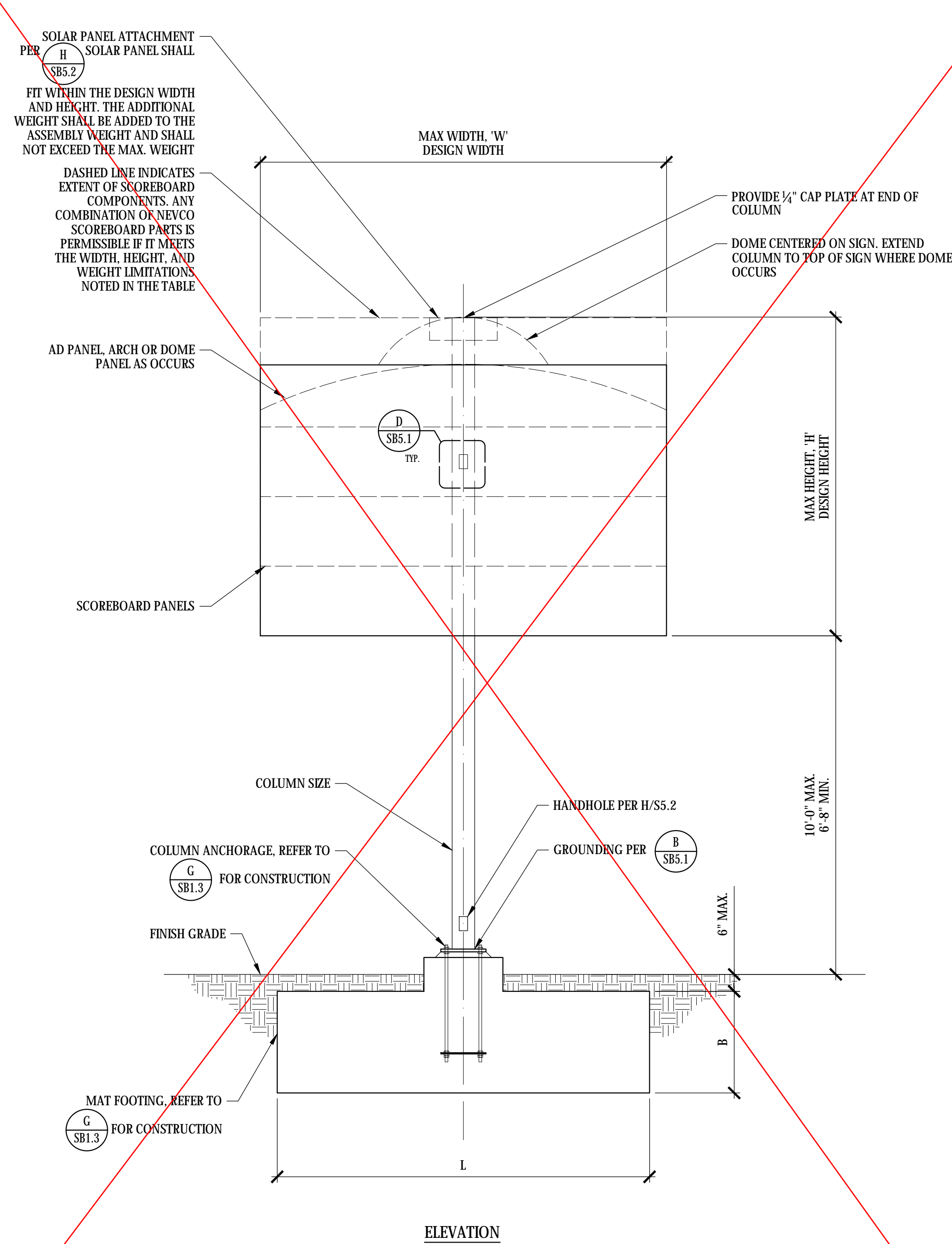
ONE COLUMN ASSEMBLY - CASE 1							
ASSEMBLY CRITERIA				MAT FOOTING CRITERIA			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	WIDTH, A	DEPTH, B	LENGTH, L
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	5'-0"	3'-0"	5'-0"
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	7'-0"	3'-0"	7'-0"

ONE COLUMN ASSEMBLY - CASE 2A <sup>(2)</sup> AND CASE 2B <sup>(2)(3)</sup>							
ASSEMBLY CRITERIA				MAT FOOTING CRITERIA			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	WIDTH, A	DEPTH, B	LENGTH, L
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	4'-6"	2'-6"	4'-6"
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	6'-0"	2'-6"	6'-0"

NOTES: (1) CASE 2 MAY NOT BE INSTALLED IN PATH OF TRAVEL. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
 (2) CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
 (3) CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD



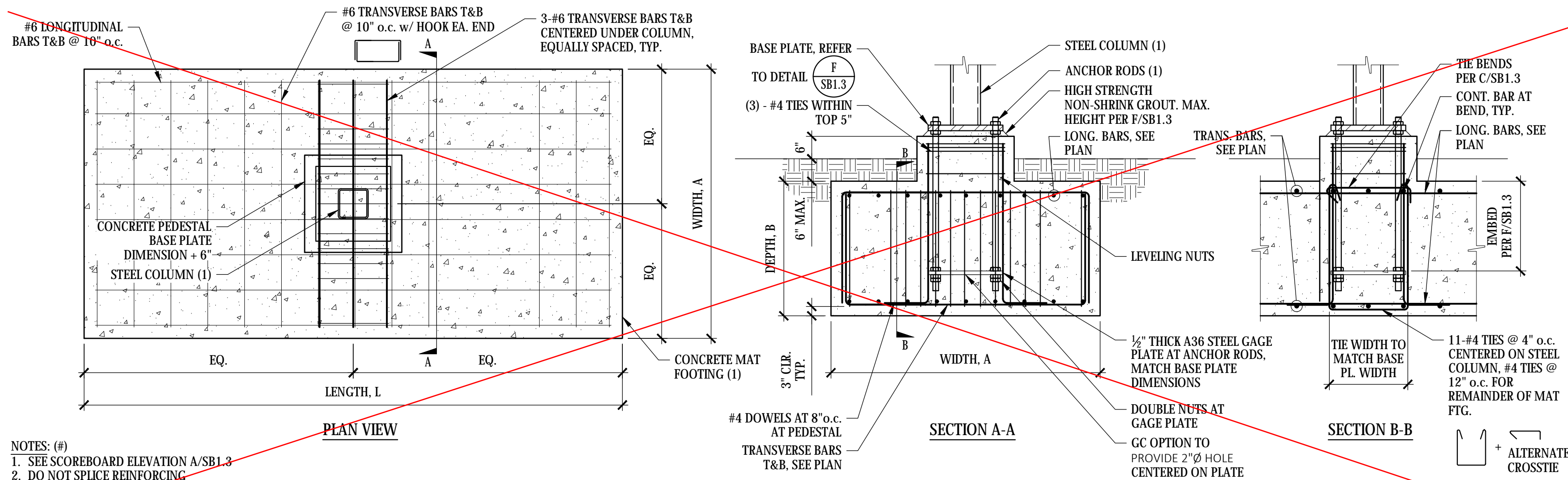
CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"  
 CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3" < CLEARANCE < 6'-8"



CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE

## ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

N.T.S.



NOTES: (1) SEE SCOREBOARD ELEVATION A/SB1.3  
 (2) DO NOT SPLICE REINFORCING

### MAT FOOTING CONSTRUCTION AND ANCHORAGE

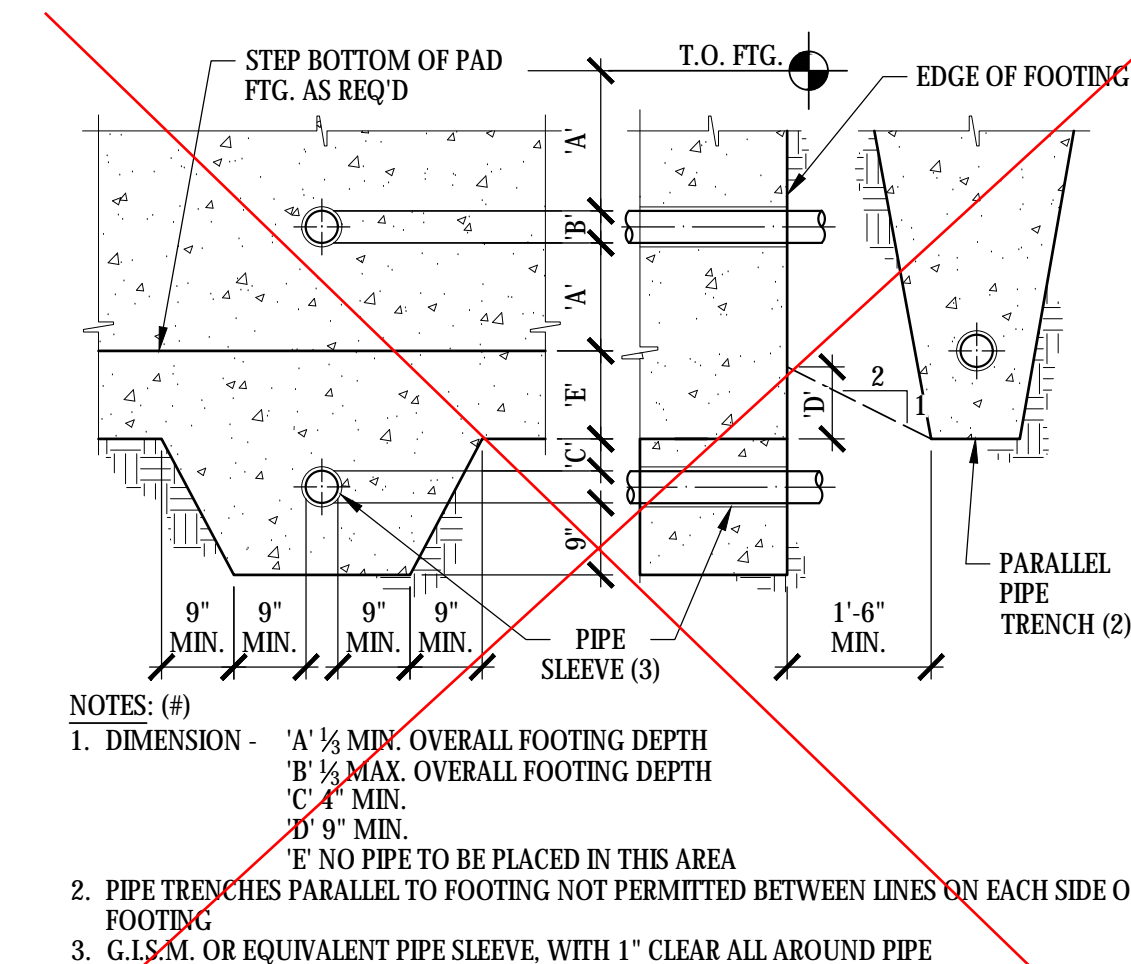
N.T.S.

BASE PLATE SCHEDULE									
COLUMN SIZE	BASE PLATE			WELD	ANCHOR ROD				
	THICKNESS, t	WIDTH, B	LENGTH, L		QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED.
HSS8x8x $\frac{5}{8}$	1"	16"	16"	$\frac{3}{8}$ " FILLET	4-1 $\frac{1}{2}$ " $\phi$	F1554 - Gr.36	2 $\frac{1}{2}$ "	1 $\frac{1}{2}$ "	30"

NOTES: (1) SEE SCOREBOARD ELEVATION, A/SB1.3

### BASE PLATE

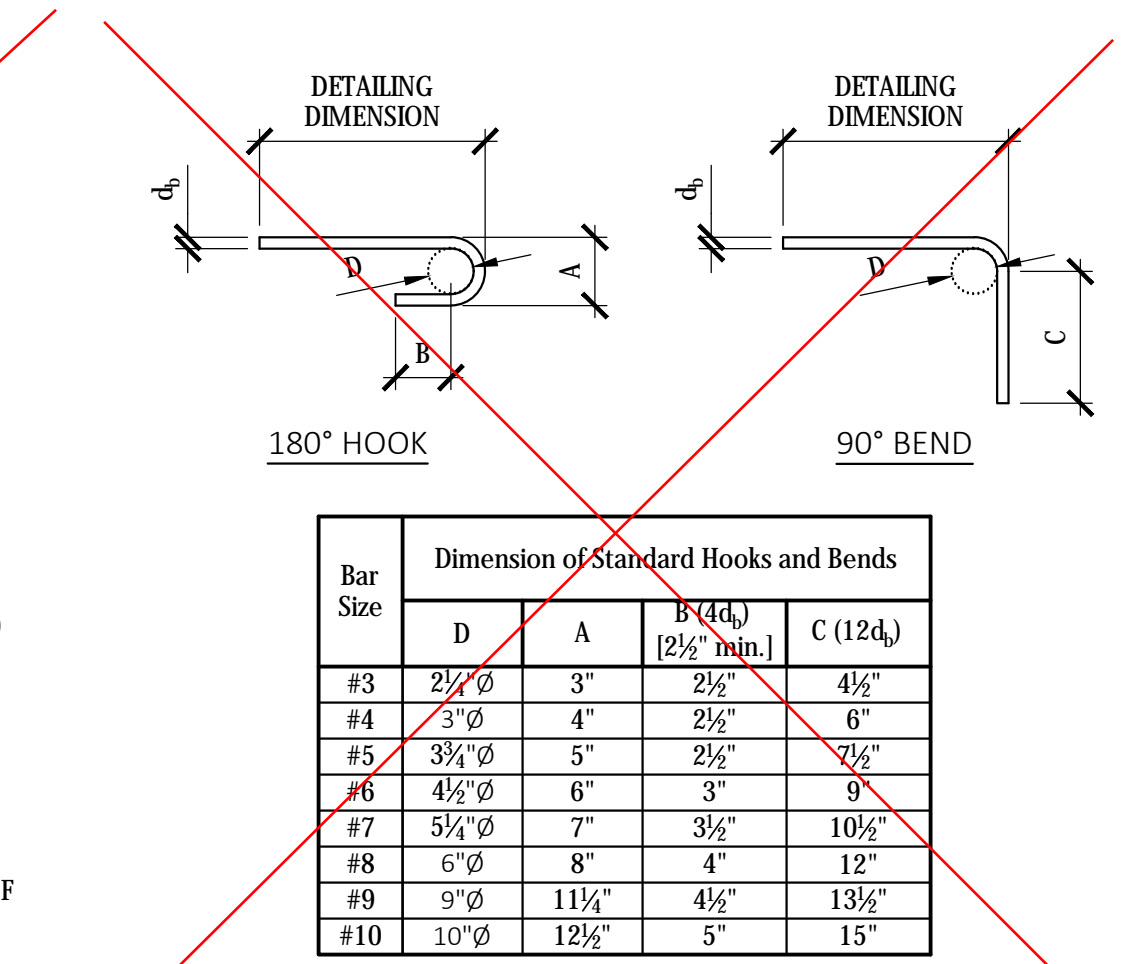
N.T.S.



NOTES: (1) DIMENSION - A'  $\frac{1}{4}$ " MIN. OVERALL FOOTING DEPTH  
 B'  $\frac{1}{4}$ " MAX. OVERALL FOOTING DEPTH  
 C' 4" MIN.  
 D' 9" MIN.  
 E' NO PIPE TO BE PLACED IN THIS AREA  
 F' G.L.S.M. OR EQUIVALENT PIPE SLEEVE, WITH 1" CLEAR ALL AROUND PIPE

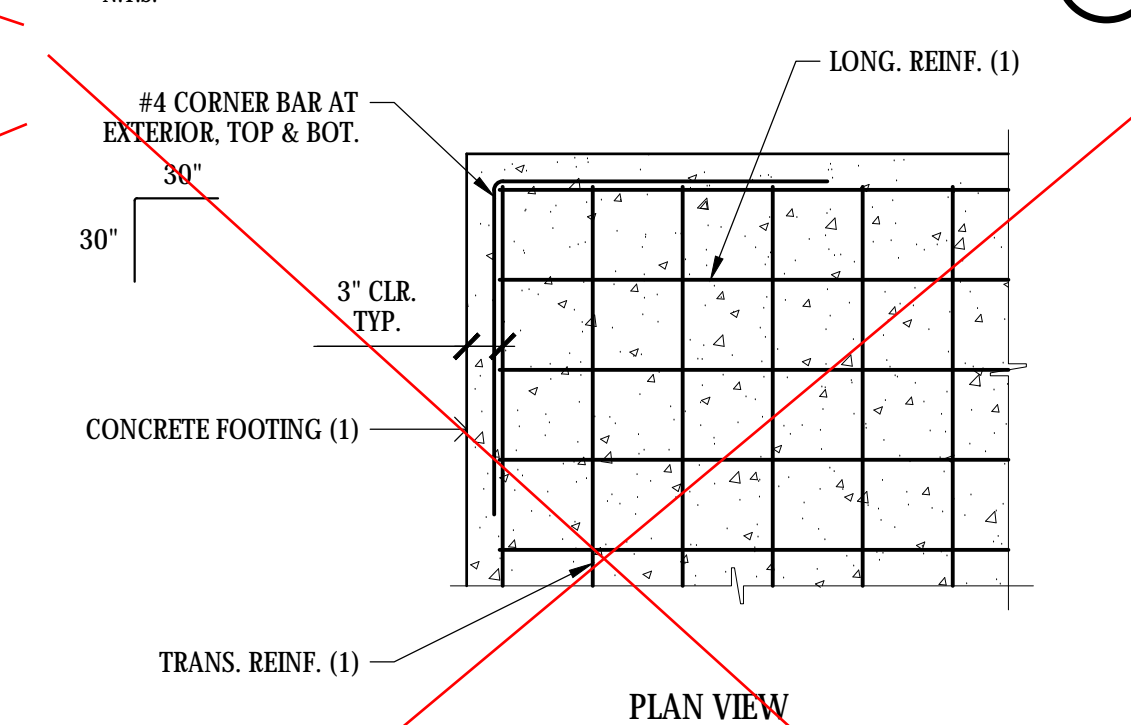
### PIPE THROUGH FOOTING

N.T.S.



### REBAR HOOKS & BENDS

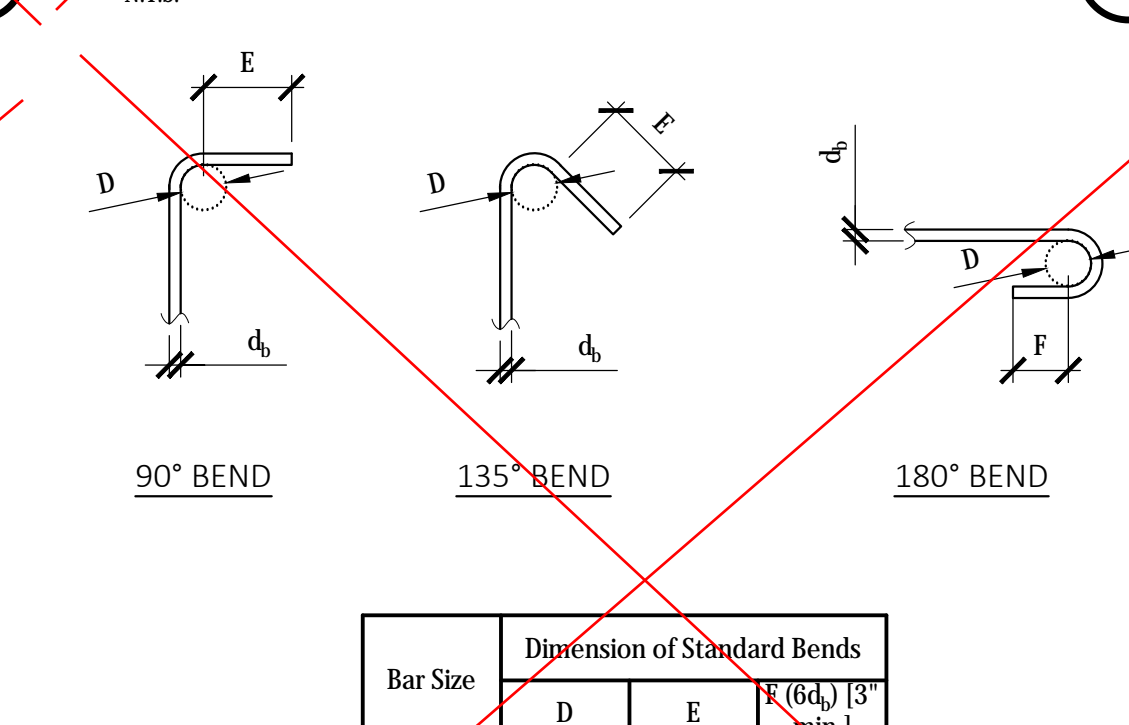
N.T.S.



NOTES: (1) SEE A/SB1.3

### TYP. FOOTING CORNER

N.T.S.



### TIE AND STIRRUP BENDS

N.T.S.

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 APP: 02-121895 INC.  
 REVIEWED FOR  
 SS ☒ FLS ☒ ACS ☐  
 DATE: 01/24/2024

**SSG**  
 structural engineers

REGISTERED PROFESSIONAL ENGINEER  
 MICHAEL E. PARSONS  
 No. 5405  
 STRUCTURAL  
 STATE OF CALIFORNIA  
 DATE SIGNED: 08.09.2023  
 PC SEOR SEAL

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 THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

**nevco**

301 East Harris Avenue, Greenville, Illinois 62246  
 Phone: 618-664-0960

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APPROVED  
 DIV. OF THE STATE ARCHITECT  
 APP: 04-122347 PC  
 REVIEWED FOR  
 SS ☒ FLS ☒ ACS ☐ CG ☐  
 DATE: 09/20/2023

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PRE-CHECK (PC) DOCUMENT  
 CODE: 2022

A separate project application  
 for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
 CALIFORNIA R. PARSONS  
 No. 52386  
 EXPIRES 12/31/2024  
 STATE OF CALIFORNIA

MARQUEE  
 MAT  
 FOOTING

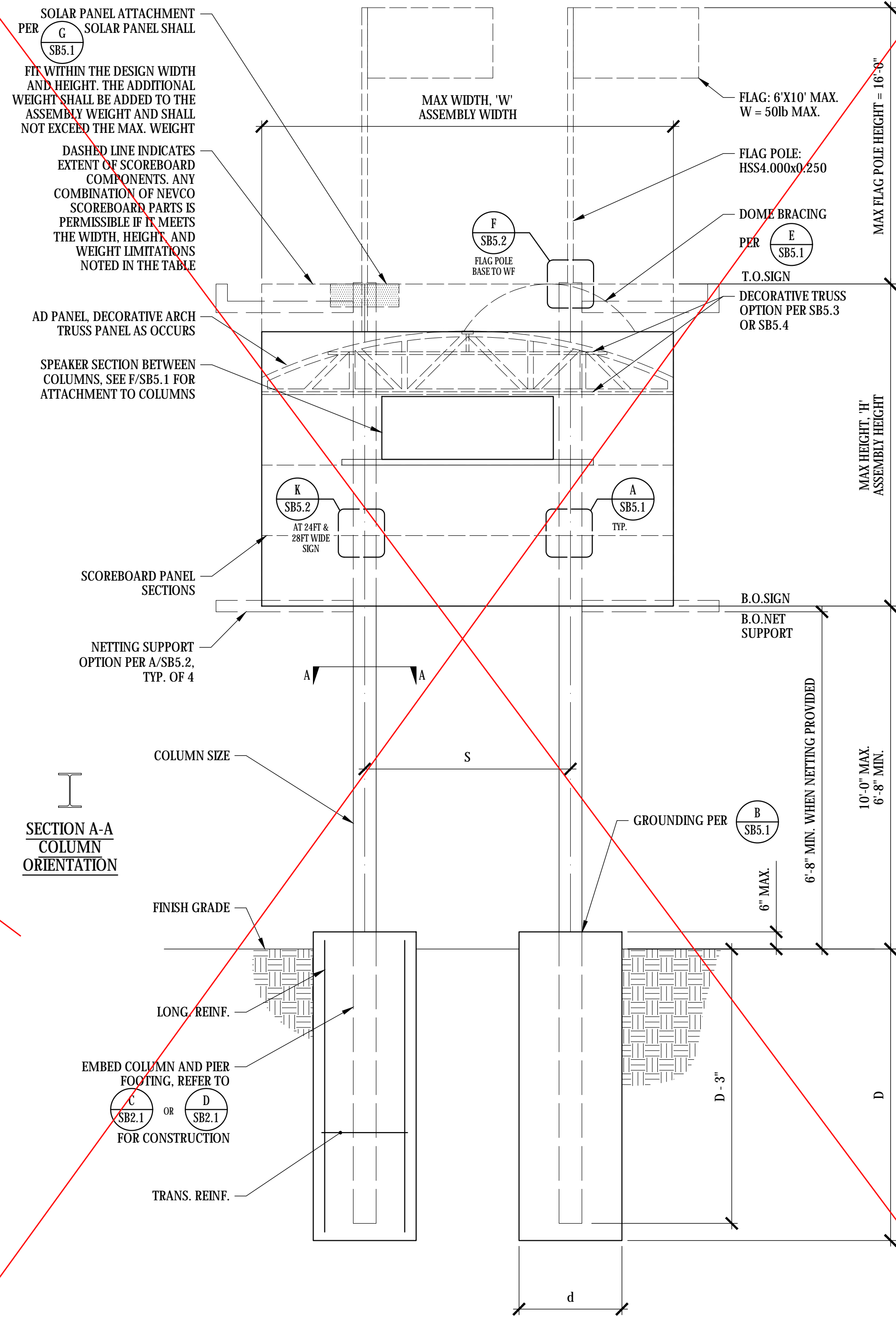
SHEET INFORMATION

DATE: 08.09.2023  
 DRAWN: JMK  
 CHECKED: MEP  
 SSG JOB #: S23109  
 SHEET: SB1.3



TWO COLUMN ASSEMBLY										
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA			PIER FOOTING CRITERIA (3)					
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF. (3)	TRANS. REINF. (2)
8'-0"		770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	30"Ø	7'-6"	N/R	N/R
		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	30"Ø	8'-9"	N/R	N/R
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	36"Ø	9'-0"	N/R	N/R
		1,920 lbs.	≤ 20'-0"	6'-0"	W14x61	W14x61	36"Ø	10'-3"	4-#5	#4 @ 8"o.c.
9'-0"		870 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	30"Ø	7'-9"	N/R	N/R
		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	30"Ø	8'-9"	N/R	N/R
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x40	36"Ø	9'-3"	4-#5	#4 @ 8"o.c.
		2,160 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	36"Ø	10'-3"	4-#5	#4 @ 8"o.c.
10'-0"		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	30"Ø	8'-0"	N/R	N/R
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-6"	N/R	N/R
		1,920 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	36"Ø	9'-9"	4-#5	#4 @ 8"o.c.
		2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	9'-9"	4-#5	#4 @ 8"o.c.
12'-0"		1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	30"Ø	8'-6"	N/R	N/R
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	W14x43	36"Ø	9'-3"	N/R	N/R
		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	36"Ø	10'-3"	4-#5	#4 @ 8"o.c.
		2,880 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-3"	4-#5	#4 @ 8"o.c.
16'-0"		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	30"Ø	9'-6"	4-#5	#4 @ 8"o.c.
		2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	36"Ø	10'-3"	4-#5	#4 @ 8"o.c.
		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	36"Ø	12'-0"	4-#5	#4 @ 8"o.c.
		3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	4-#6	#4 @ 8"o.c.
18'-0"		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	30"Ø	10'-0"	4-#5	#4 @ 8"o.c.
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x43	W14x43	36"Ø	10'-9"	4-#5	#4 @ 8"o.c.
		3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	48"Ø	10'-9"	4-#6	#4 @ 8"o.c.
		4,320 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x77	48"Ø	12'-0"	4-#6	#4 @ 8"o.c.
24'-0"		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	36"Ø	9'-9"	4-#5	#4 @ 8"o.c.
		3,490 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	36"Ø	11'-6"	4-#5	#4 @ 8"o.c.
		4,610 lbs.	≤ 16'-0"	14'-0"	W16x67	W16x67	48"Ø	11'-9"	4-#6	#4 @ 8"o.c.
		5,760 lbs.	≤ 20'-0"	14'-0"	W18x86	W18x86	48"Ø	13'-3"	4-#6	#4 @ 8"o.c.
28'-0"		6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	W18x119	48"Ø	14'-6"	4-#8	#4 @ 8"o.c.
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	48"Ø	16'-0"	4-#8	#4 @ 6"o.c.
		2,690 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	42"Ø	10'-0"	4-#6	#4 @ 6"o.c.
		4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	48"Ø	11'-3"	4-#6	#4 @ 6"o.c.
		5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x67	48"Ø	12'-9"	4-#6	#4 @ 6"o.c.
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	48"Ø	14'-3"	4-#8	#4 @ 6"o.c.
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	48"Ø	15'-9"	4-#8	#4 @ 6"o.c.
		9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	48"Ø	17'-0"	4-#8	#4 @ 6"o.c.

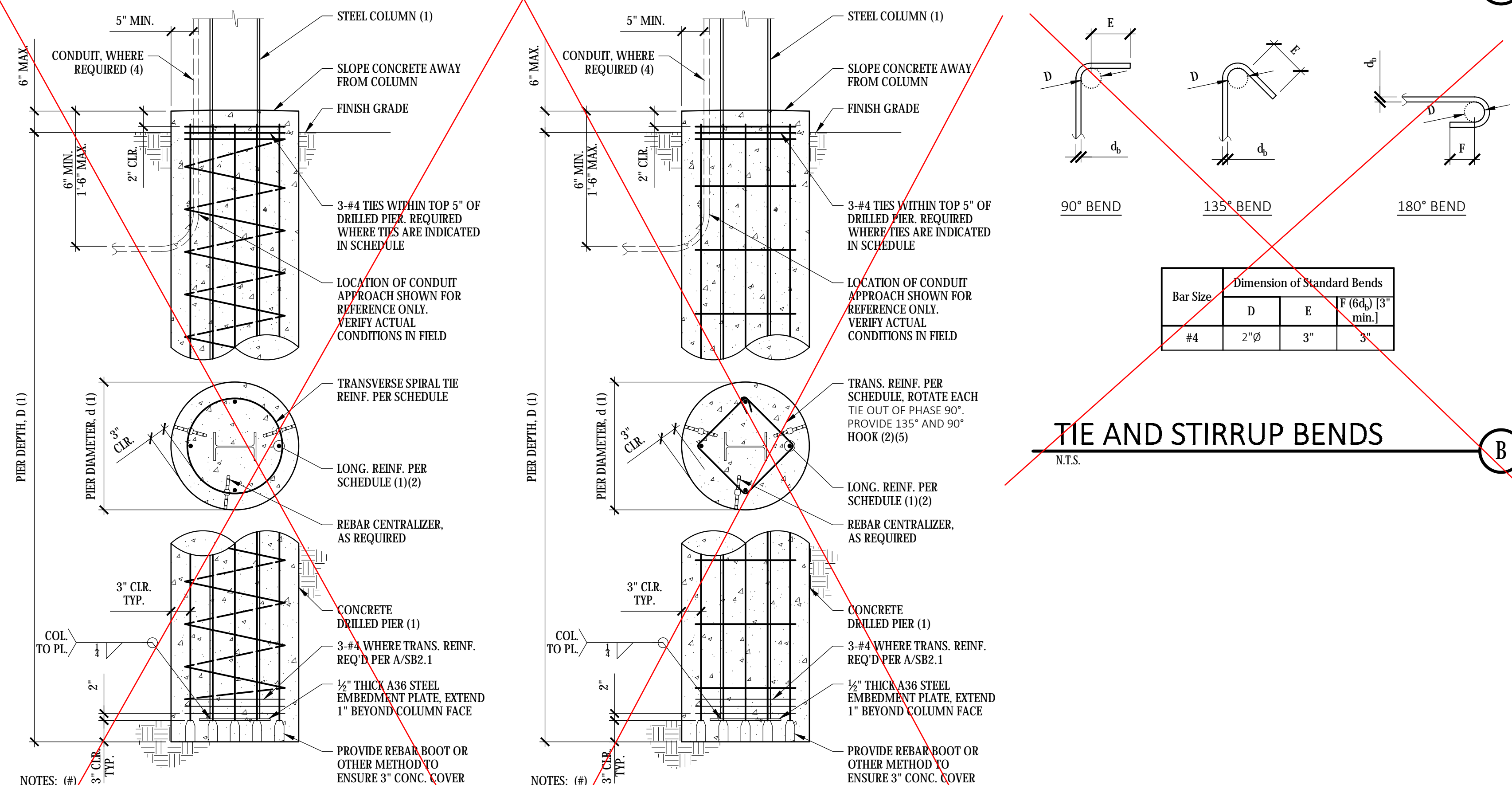
NOTES: (H)  
1. N/R - REINFORCEMENT NOT REQUIRED PER ISA RU 09-06.  
2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB2.1 FOR THE OPTION. SEE D/SB2.1 FOR SPIRAL OPTION.  
3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



ELEVATION

## TWO COLUMN SCOREBOARD INSTALLATION

N.T.S.



NOTES: (H)  
1. SEE ELEVATION SIGN ELEVATION, A/SB2.1  
2. SEE DETAILS B/SB2.1 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPLICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.a. MIN  
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

### CONCRETE DRILLED PIER

N.T.S.

NOTES: (H)  
1. SEE ELEVATION SIGN ELEVATION, A/SB2.1  
2. SEE DETAILS B/SB2.1 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPLICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.a. MIN  
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

### CONCRETE DRILLED PIER

N.T.S.

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APP: 02-121895 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

SSG  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. PAVONI  
No. 5405  
STATE OF CALIFORNIA  
PC SEOR REAL  
08.09.2023

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Phone: (618) 664-0360

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DATE: 09/20/2023

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for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
GAILARD R. PAVONI  
No. 52356  
EXPIRES 12/31/2024  
STATE OF CALIFORNIA

TWO COLUMN  
CAISSON -  
EMBEDDED

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

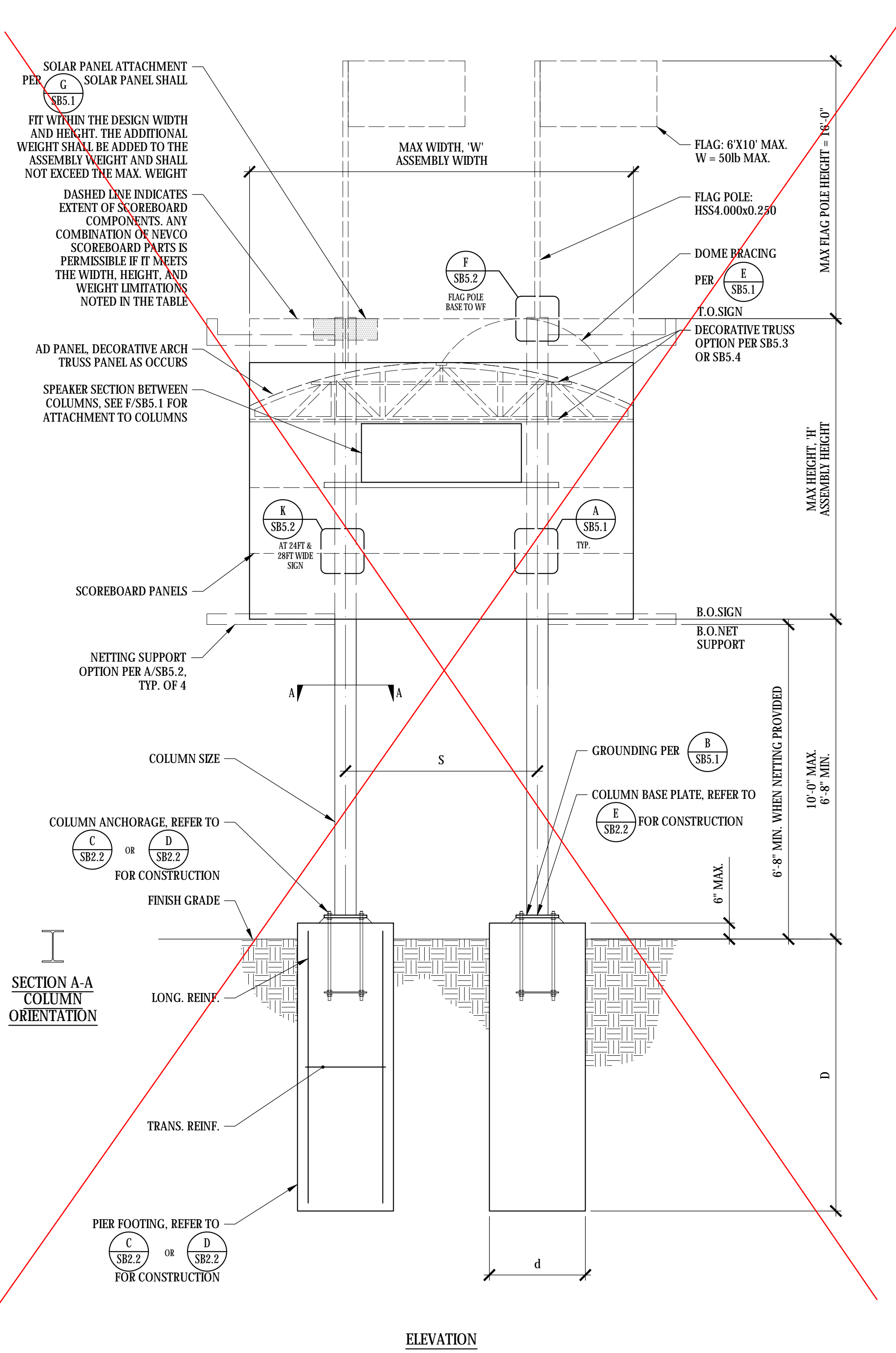
SSG JOB # S23109

SHEET SB2.1



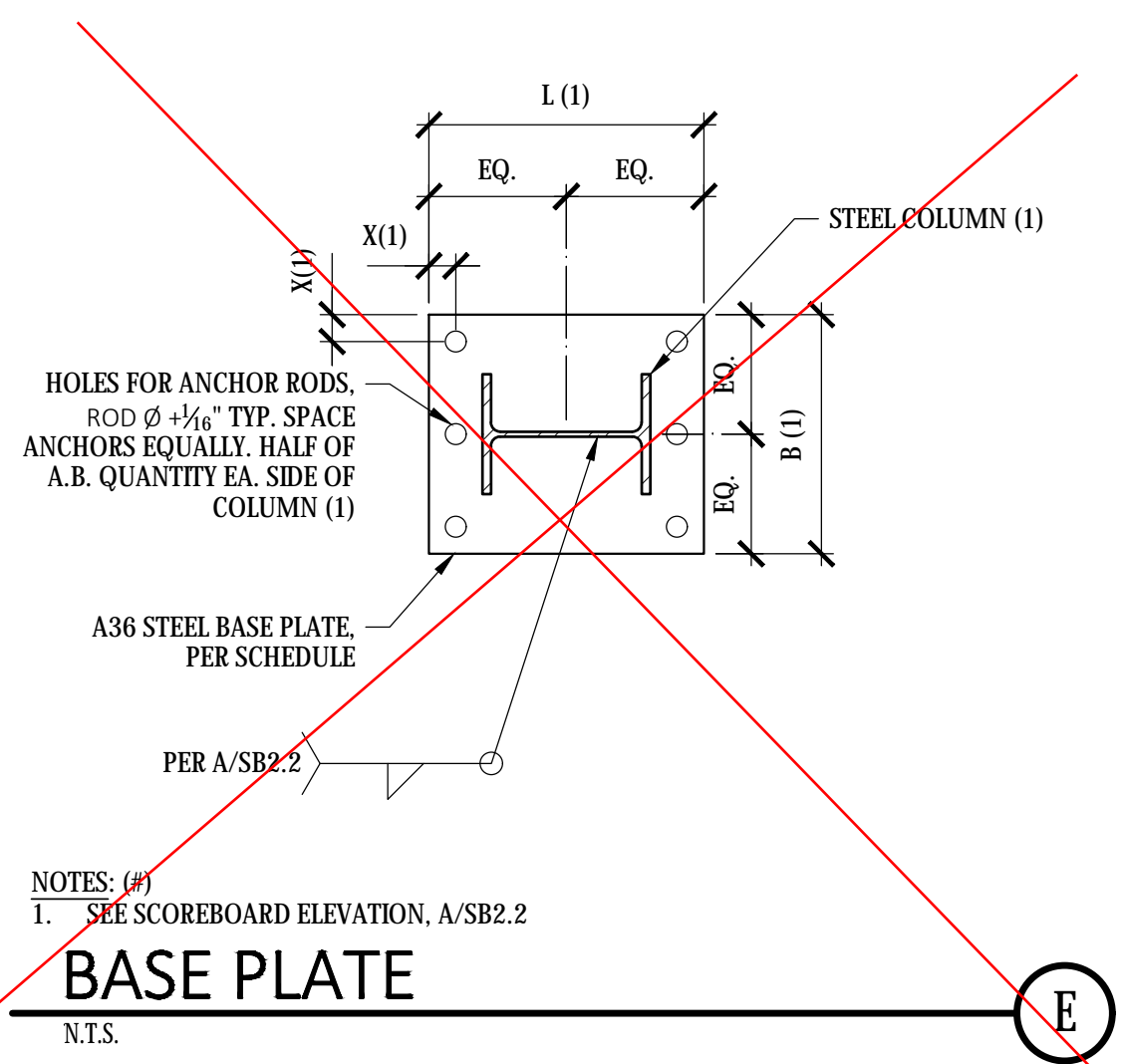
TWO COLUMN ASSEMBLY																							
ASSEMBLY WIDTH, W		CHECK OPTION THIS APPLICATION		ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)				BASE PLATE				QUANTITY & DIAMETER		ANCHOR RODS		GROUT HEIGHT		EMBED	
				MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	GRADE	EDGE DISTANCE, X					
8'-0"				770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	36"Ø	7'-0"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	36"Ø	8'-0"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	36"Ø	9'-0"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"		
				1,920 lbs.	≤ 20'-0"	6'-0"	W14x61	W14x61	42"Ø	9'-9"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"		
9'-0"				870 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	36"Ø	7'-3"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-3"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x40	36"Ø	9'-3"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"		
				2,160 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	42"Ø	10'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"		
10'-0"				960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	36"Ø	7'-6"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-6"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,920 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	42"Ø	9'-9"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.105	2½"	2"	48"		
				2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	9'-9"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.105	2½"	2"	64"		
12'-0"				1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-0"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	W14x43	42"Ø	9'-3"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"		
				2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	42"Ø	10'-3"	8- #8	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"		
				2,880 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-3"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"		
16'-0"				1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-9"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"		
				2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	36"Ø	10'-3"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	48"		
				3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	12'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"		
				3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	12- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.105	2½"	2"	64"		
18'-0"				1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	36"Ø	9'-0"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"	F1554 - GR.36	2½"	2"	48"		
				2,600 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	42"Ø	10'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"		
				3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	48"Ø	10'-9"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"		
				4,320 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x77	48"Ø	13'-0"	12- #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"		
24'-0"				2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	36"Ø	9'-9"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	48"		
				3,460 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	36"Ø	11'-6"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"	F1554 - GR.55	2½"	2"	64"		
				4,610 lbs.	≤ 16'-0"	14'-0"	W16x67	W16x67	48"Ø	11'-9"	12- #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"		
				5,760 lbs.	≤ 20'-0"	14'-0"	W18x86	W18x86	48"Ø	13'-3"	12- #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"		
28'-0"				6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	W18x119	48"Ø	14'-6"	12- #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 2"Ø	F1554 - GR.105	3"	2"	64"		
				8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	54"Ø	16'-0"	12- #8	#4 @ 6" o.c.	2½"	24"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2"	64"		
				2,690 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	42"Ø	10'-0"	8- #7	#4 @ 4½" o.c.	1½"	24"	24"	⅜	(4) - 1½"	F1554 - GR.55	2½"	2"	64"		
				4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	48"Ø	11'-3"	8- #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(4) - 1½"	F1554 - GR.55	3"	2"	64"		
28'-0"				5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x67	48"Ø	12'-9"	12- #8	#4 @ 6" o.c.	2"	24"	30"	⅜	(6) - 1½"	F1554 - GR.55	3"	2"	64"		
				6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	48"Ø	14'-3"	12- #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"	F1554 - GR.105	3"	2"	64"		
				8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	54"Ø	15'-9"	12- #8	#4 @ 6" o.c.	2½"	24"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	64"		
				9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	54"Ø	16'-6"	14- #8	#4 @ 6" o.c.	3"	24"	36"	CJP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	64"		

NOTES: (4)  
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB2.2 FOR TIE OPTION. SEE D/SB2.2 FOR SPIRAL OPTION.  
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIER AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.

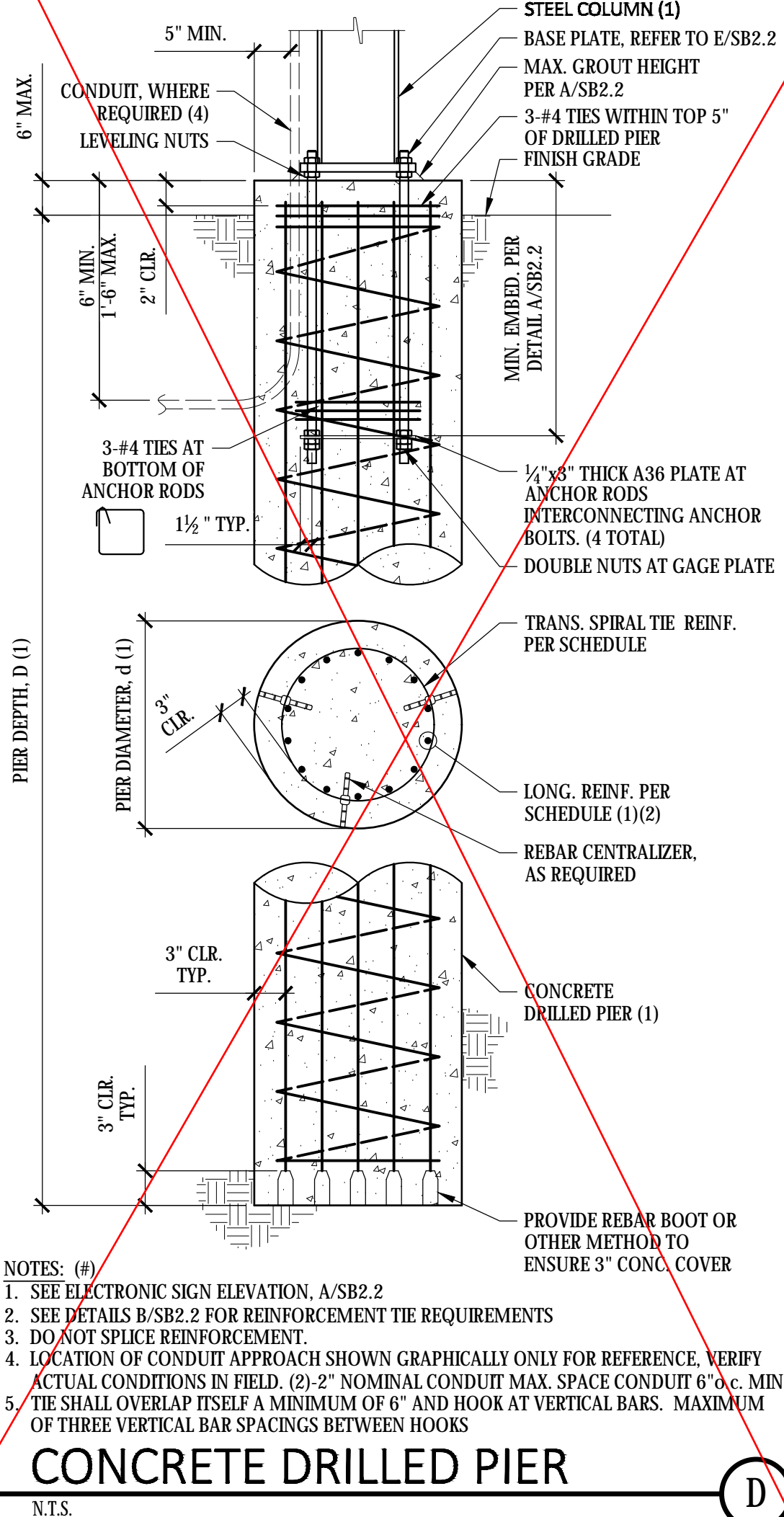


## TWO COLUMN SCOREBOARD INSTALLATION

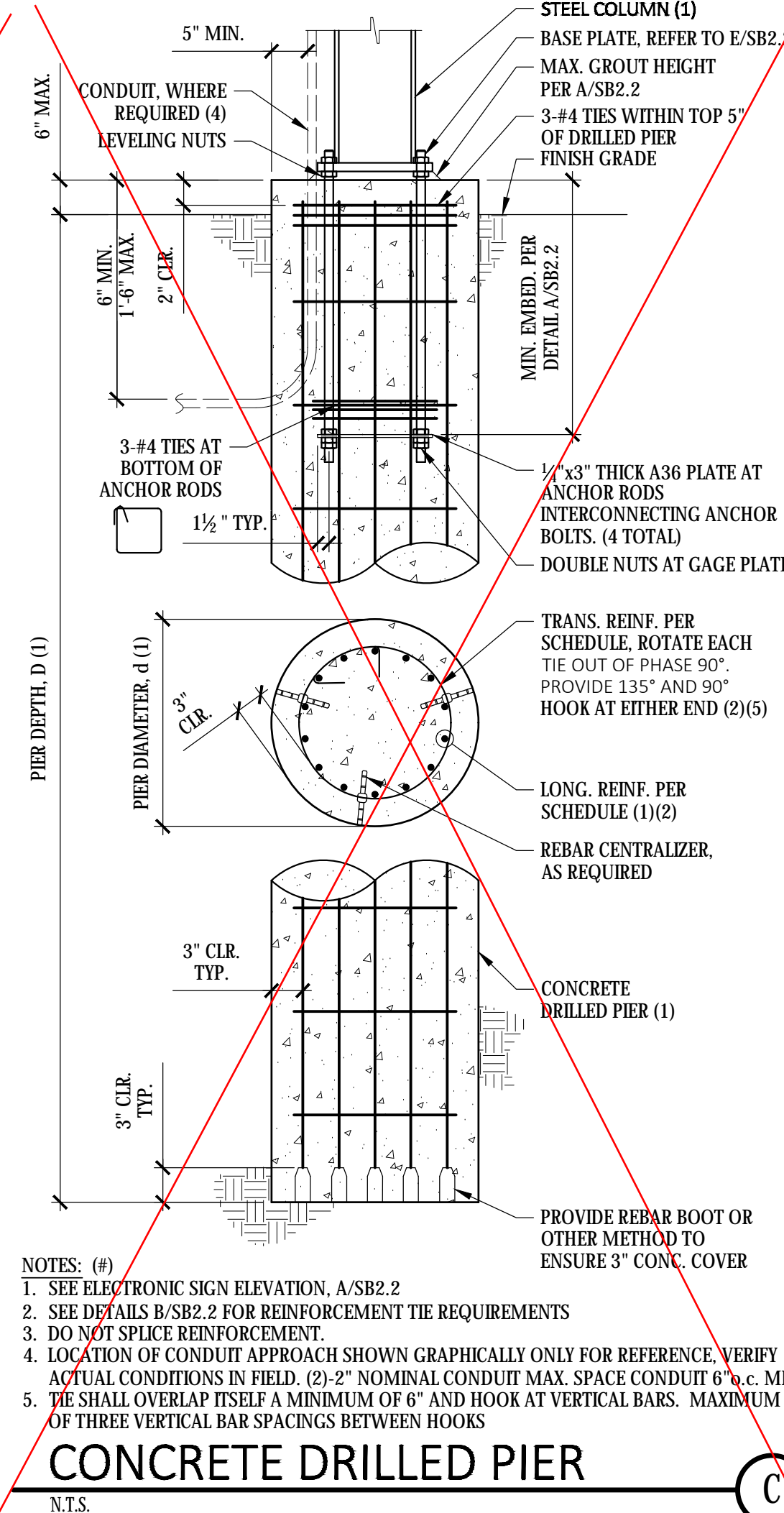
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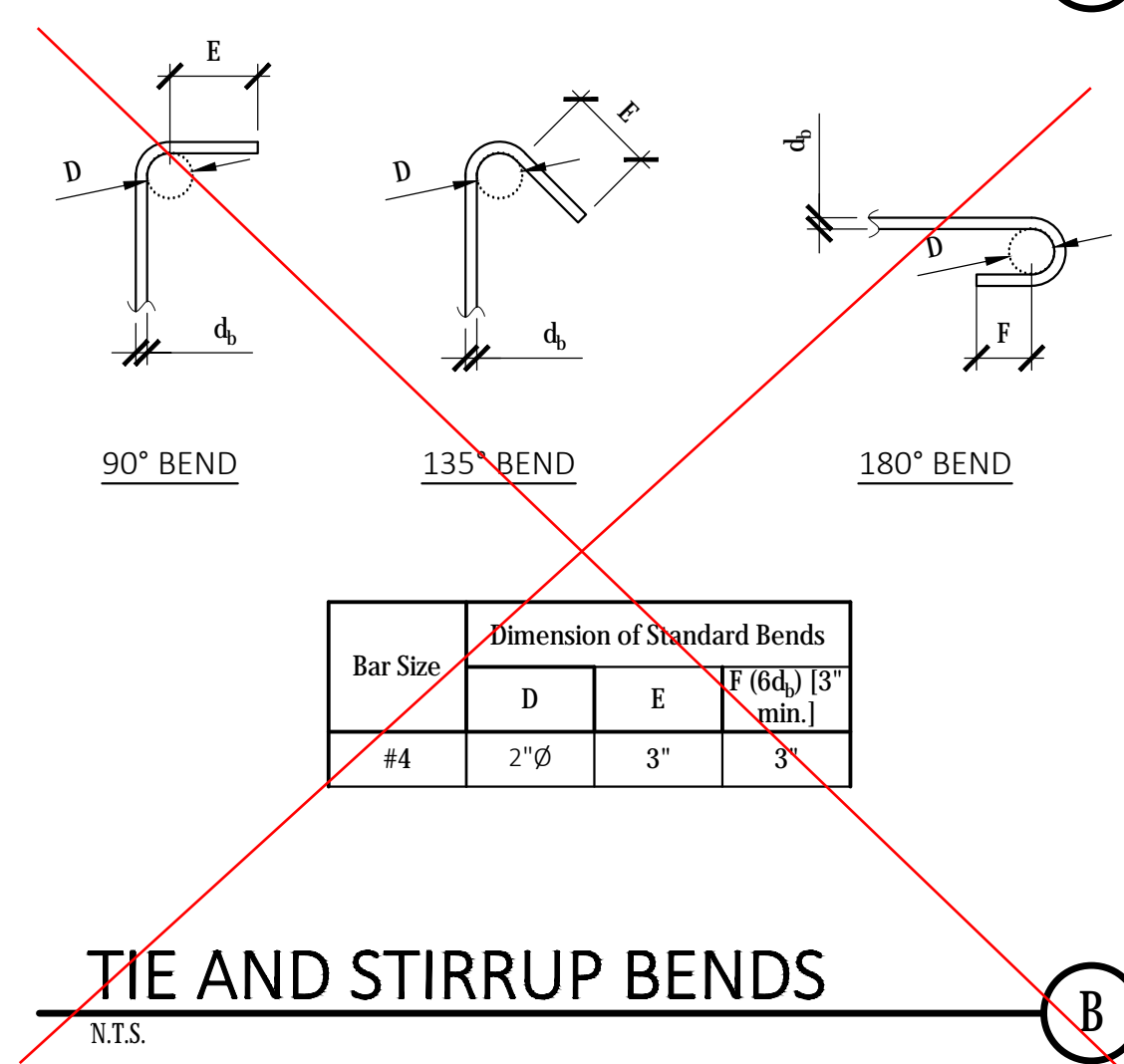
NOTES: (4)  
1. SEE SCOREBOARD ELEVATION, A/SB2.2  
2. SEE DETAILS B/SB2.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. THE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS



NOTES: (4)  
1. SEE ELECTRONIC SIGN ELEVATION, A/SB2.2  
2. SEE DETAILS B/SB2.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. THE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS



NOTES: (4)  
1. SEE ELECTRONIC SIGN ELEVATION, A/SB2.2  
2. SEE DETAILS B/SB2.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. THE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS



Dimension of Standard Bends		Dimension of Standard Bends	
Bar Size	D	E	F (6d) [3" min.]
#4	2"Ø	3"	3"

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DATE: 01/24/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. PARSONS  
No. 5405  
STATE OF CALIFORNIA  
08.09.2023

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS.

nevco  
301 East Harris Avenue, Greenville, Illinois 62246  
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APP: 04-122347 PC  
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DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT  
CODE: 2022  
A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
GAILLARD R. RAJIC  
No. 52386  
EXPIRES 12/31/2024  
STATE OF CALIFORNIA

TWO COLUMN  
CAISSON -  
BOLTED

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB2.2



**#6 LONGITUDINAL BARS T&B @ 10" o.c.**

**#6 TRANSVERSE BARS T&B @ 10" o.c. w/ HOOK EA. END**

**3-#6 TRANSVERSE BARS T&B CENTERED UNDER COLUMN, EQUALLY SPACED, TYP.**

**PEDESTAL**

**BASE PLATE**

**STEEL COLUMN (I)**

**EQ.**

**A**

**COLUMN SPACING, S**

**EQ.**

**LENGTH, L**

**WIDTH, A**

**CONCRETE MAT FOOTING (1)**

**PLAN VIEW**

**SECTION A-A**

**SECTION B-B**

**BASE PLATE, REFER TO DETAIL**

**(3) - #4 TIES WITHIN TOP 5"**

**8"**

**6" MAX. DEPTH, B**

**3" CLR. TYP.**

**#4 DOWELS AT 8" o.c. AT PEDESTAL TRANSVERSE BARS T&B, SEE PLAN**

**STEEL COLUMN (I)**

**ANCHOR RODS w/ LEVELING NUTS (1)**

**HIGH STRENGTH NON-SHRINK GROUT, MAX. HEIGHT PER A/SB2.3**

**LONG. BARS, SEE PLAN**

**TRANS. BARS, SEE PLAN**

**TIE BENDS PER D/SB2.3**

**CONT. BAR AT BEND, TYP.**

**LONG. BARS, SEE PLAN**

**1/2" THICK A36 STEEL GAGE PLATE AT ANCHOR RODS, MATCH BASE PLATE DIMENSIONS**

**DOUBLE NUTS AT GAGE PLATE**

**GC OPTION TO PROVIDE 2"x6" HOLE, CENTERED ON PLATE**

**11-#4 TIES @ 4" o.c. CENTERED ON STEEL COLUMN, #4 TIES @ 12" o.c. FOR REMAINDER OF MAT FTG.**

**ALTERNATE CROSS-TIE**

**NOTES:** (0)  
1. SEE SCOREBOARD ELEVATION A/SB2.3

# MAT FOOTING CONSTRUCTION AND ANCHORAGE

Bar Size	Dimension of Standard Hooks and Bends			
	D	A	B (4d <sub>b</sub> ) (2 1/2" min.)	C (12d <sub>b</sub> )
#3	2 1/4" Ø	3"	2 1/2"	4 1/2"
#4	3" Ø	4"	2 1/2"	6"
#5	3 3/8" Ø	5"	2 1/2"	7 1/2"
#6	4 1/8" Ø	6"	3"	9"
#7	5 1/4" Ø	7"	3 1/2"	10 1/2"
#8	6" Ø	8"	4"	12"
#9	9" Ø	11 1/4"	4 1/2"	13 1/2"
#10	10" Ø	12 1/2"	5"	15"

Bar Size	Concrete Reinforcing Splices (1)(2)		
	Class A (3) Splice	Class B (3) Splice	$f_y$ (Min.)
#3	16"	21"	40 ksi
#4	21"	27"	60 ksi
#5	40"	51"	60 ksi
#6	47"	61"	60 ksi
#7	63"	81"	60 ksi
#8	72"	93"	60 ksi
#9	81"	105"	60 ksi
#10	91"	118"	60 ksi

NOTES: (#)  
1. LAP LENGTHS LISTED APPLY TO HORIZONTAL REINFORCEMENT  
2. WHERE BARS OF A DIFFERENT SIZE ARE LAPPED, THE LAP LENGTH SHALL BE THE LENGTH REQUIRED BY THE LARGER BAR  
3. ALL SPLICES SHALL BE CONSIDERED CLASS B UNLESS SPECIFICALLY NOTED OTHERWISE

NOTES: (#)  
1. SEE G/SB2.3

#10	10"Ø	12½"	5"
-----	------	------	----

**BEAR HOOKS & BENDS**

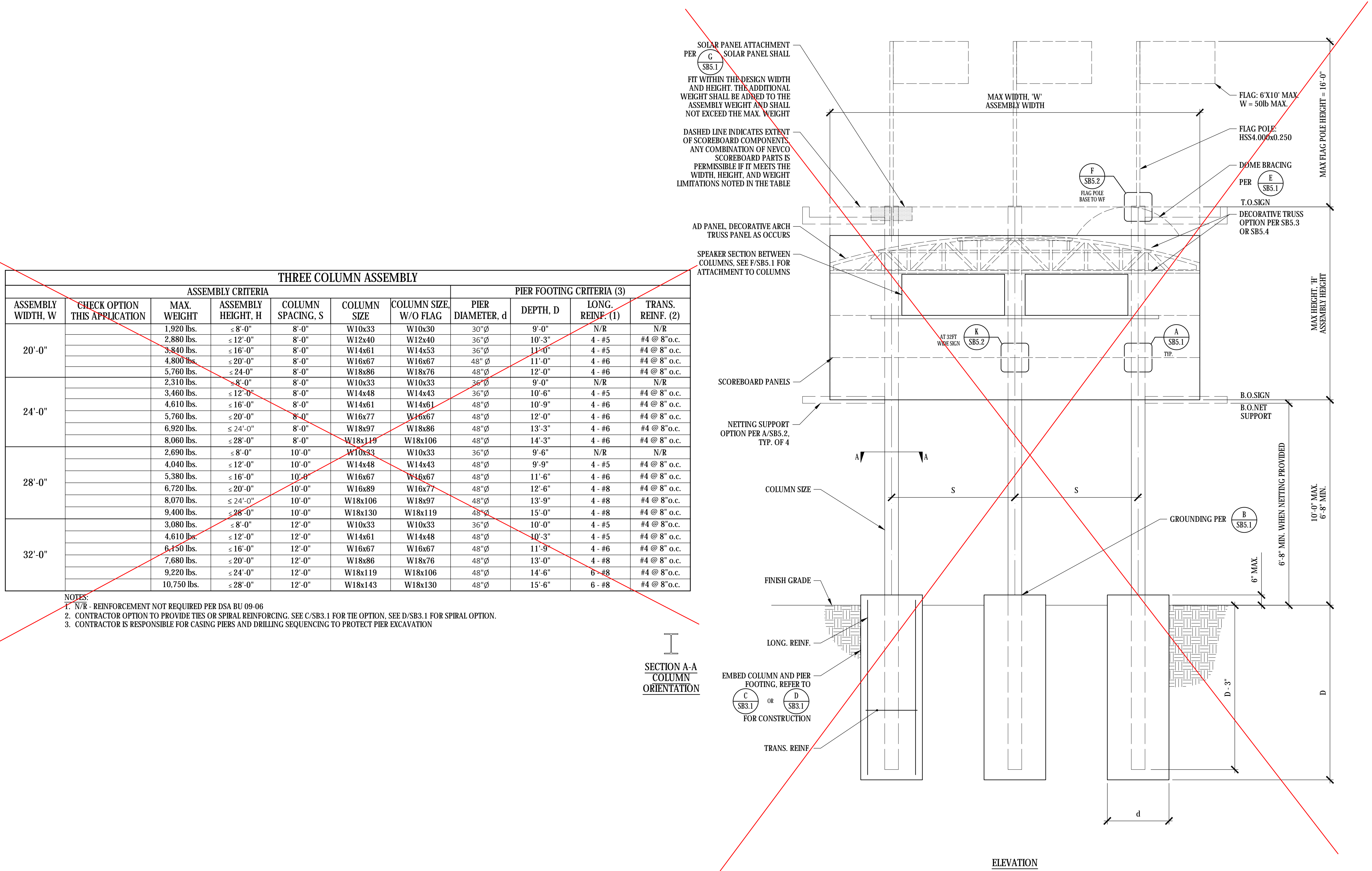
REQUIRED BY THE LARGER BAR  
3. ALL SPLICES SHALL BE CONSIDERED CLASS B UNLESS

BASE PLATE

## PIPE THROUGH FOOTING

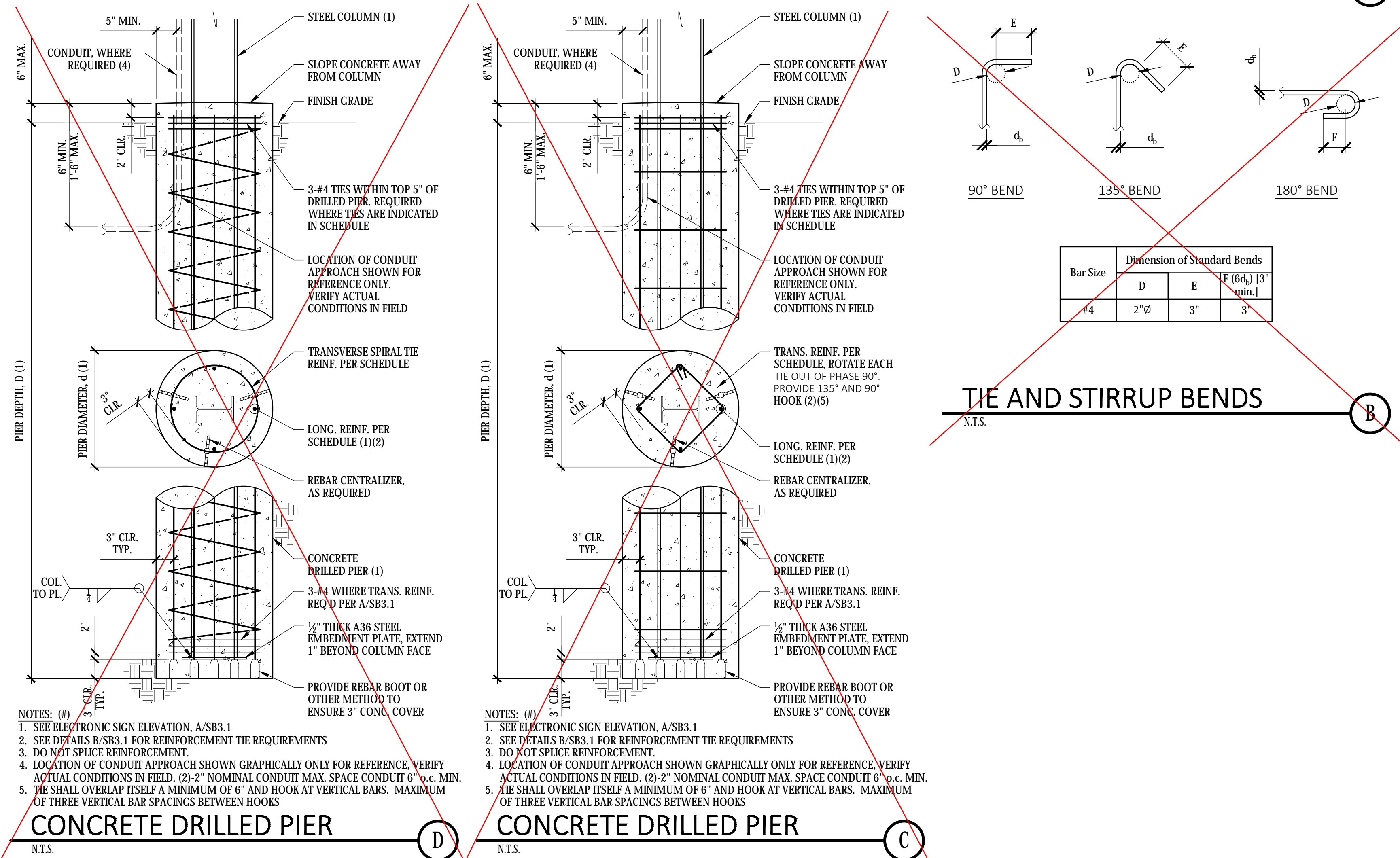
## TIE AND STIRRUP BEND





### THREE COLUMN SCOREBOARD INSTALLATION

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GAILORD R. RAJIC  
No. 52356  
EXPIRES 12/31/2024  
STATE OF CALIFORNIA

THREE COLUMN  
CAISSON -  
EMBEDDED

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

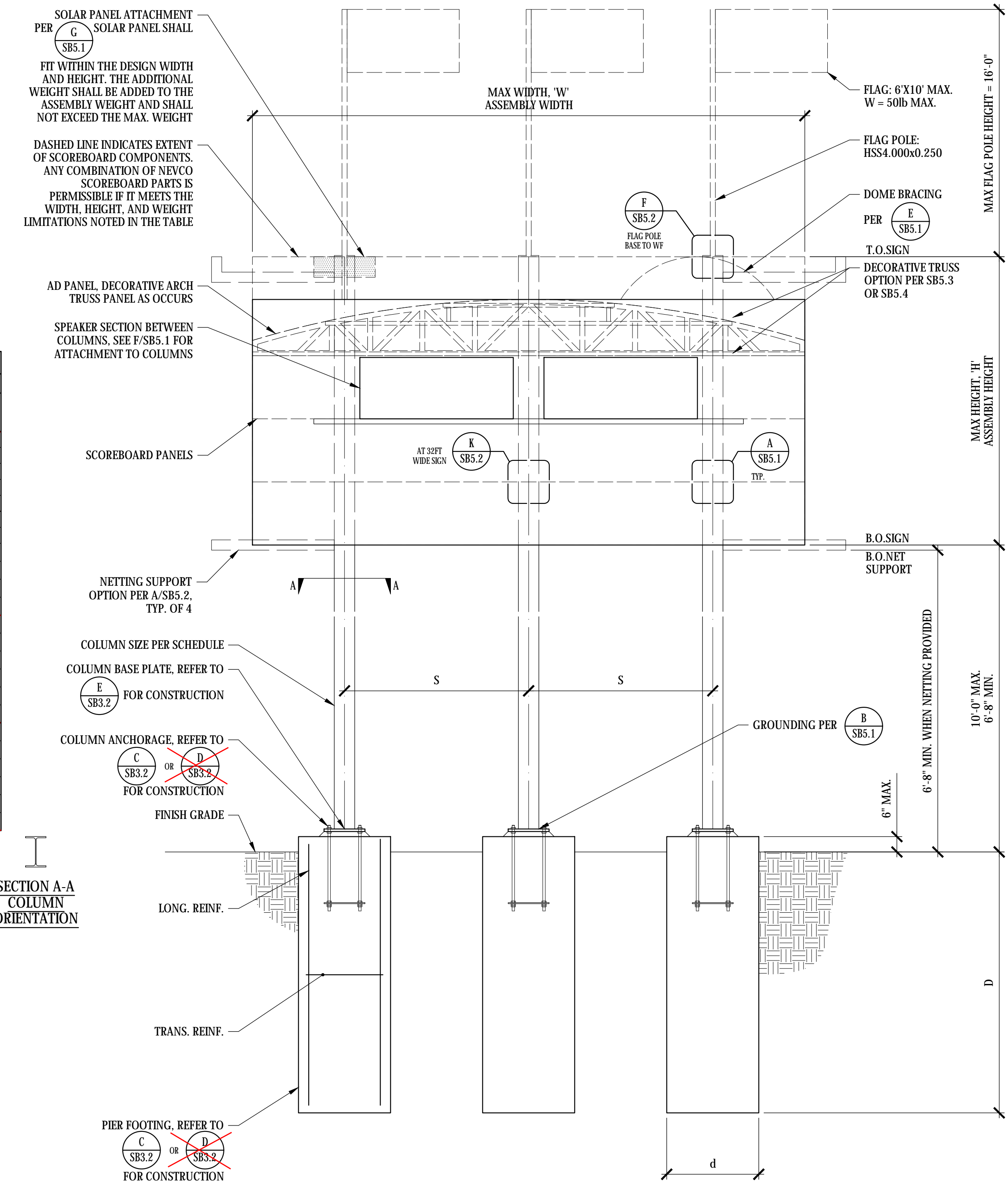
SSG JOB # S23109

SHEET SB3.1



		THREE COLUMN ASSEMBLY															
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)				BASE PLATE			ANCHOR RODS				
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"Ø	F1554 - GR.36	2½"
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	36"Ø	10'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(6) - 1½"Ø	F1554 - GR.55	2½"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	42"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"Ø	F1554 - GR.55	2½"
		4,800 lbs.	≤ 20'-0"	8'-0"	W16x67	W16x67	48"Ø	11'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"Ø	F1554 - GR.55	3"
		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	48"Ø	12'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
24'-0"		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"Ø	F1554 - GR.55	2½"
	X	3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	36"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	⅜	(6) - 1½"Ø	F1554 - GR.55	2½"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-0"	12 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"Ø	F1554 - GR.55	2½"
		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"Ø	F1554 - GR.105	2½"
		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	48"Ø	13'-3"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
28'-0"		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	48"Ø	14'-3"	14 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"Ø	F1554 - GR.55	2½"
		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	48"Ø	9'-9"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"Ø	F1554 - GR.55	2½"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-6"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(6) - 1½"Ø	F1554 - GR.55	3"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	14 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
32'-0"		8,070 lbs.	≤ 24'-0"	10'-0"	W18x106	W18x97	48"Ø	13'-9"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	48"Ø	15'-0"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.105	3"
		3,080 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	⅜	(4) - 1½"Ø	F1554 - GR.55	2½"
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-3"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	⅜	(6) - 1½"Ø	F1554 - GR.55	2½"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	11'-9"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	⅜	(4) - 1½"Ø	F1554 - GR.55	3"
32'-0"		7,680 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	48"Ø	13'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	54"Ø	15'-6"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.35	4"

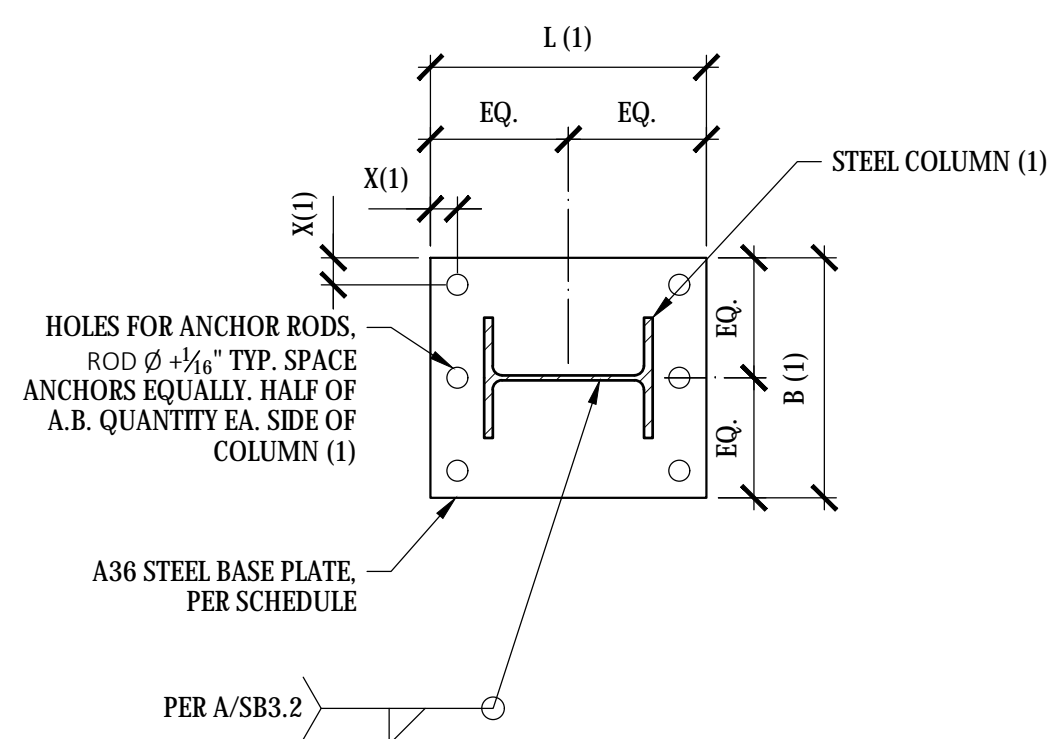
NOTES:  
1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB3.2 FOR THE OPTION, SEE D/SB3.2 FOR SPIRAL OPTION.  
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



ELEVATION

## THREE COLUMN SCOREBOARD INSTALLATION

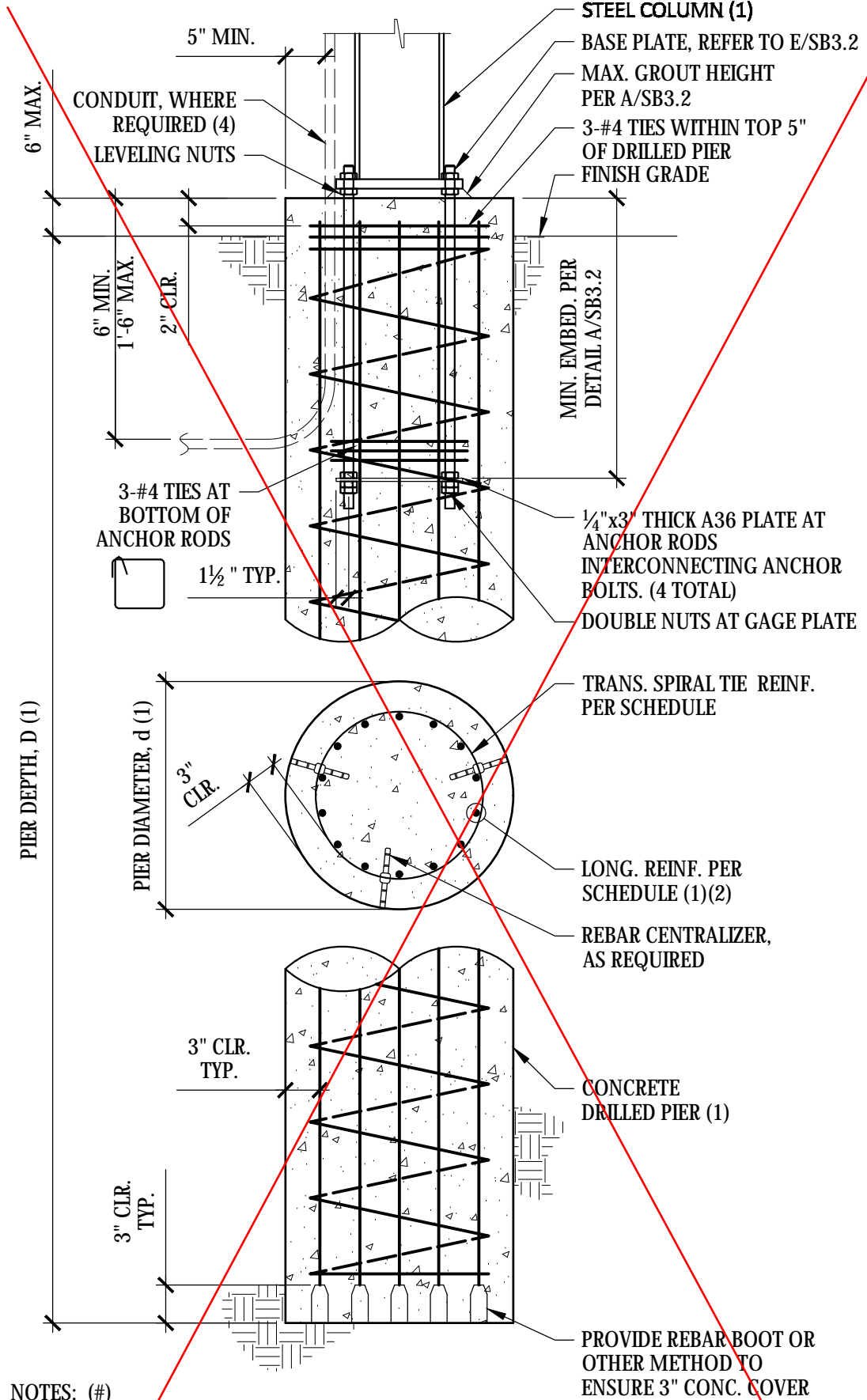
N.T.S.



NOTES: (#)  
1. SEE SCOREBOARD ELEVATION, A/SB3.2

### BASE PLATE

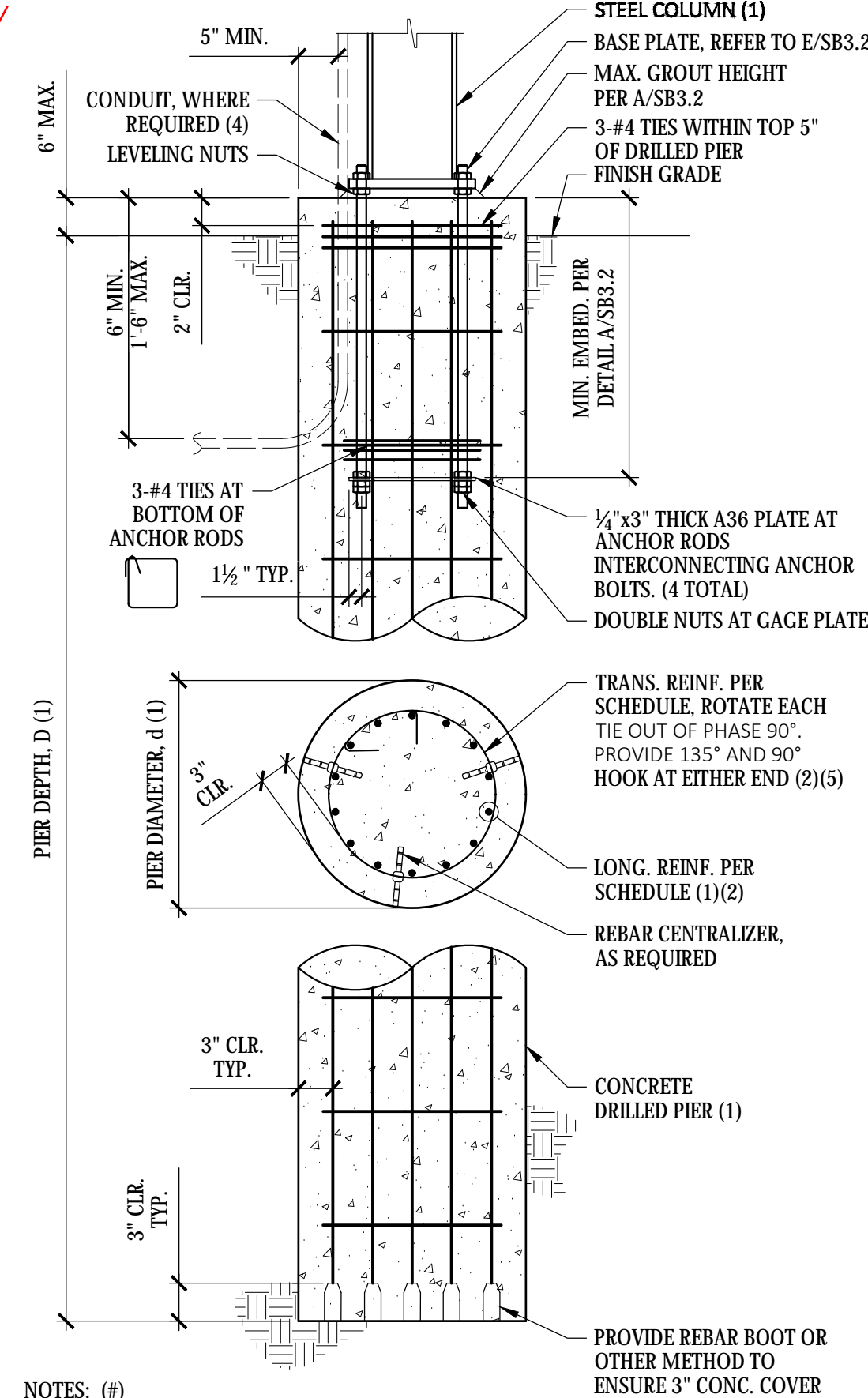
N.T.S.



NOTES: (#)  
1. SEE ELECTRONIC SIGN ELEVATION, A/SB3.2  
2. SEE DETAILS B/SB3.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPICE REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. TIE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

### CONCRETE DRILLED PIER

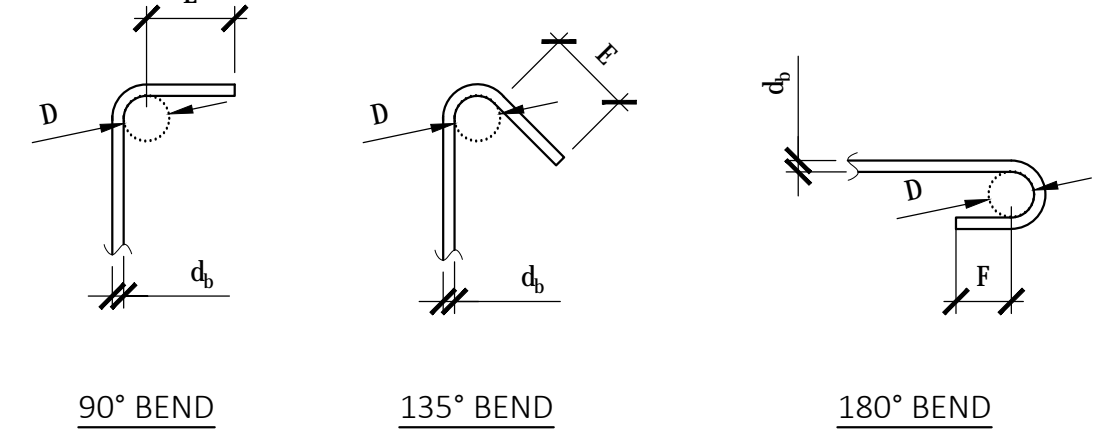
N.T.S.



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### CONCRETE DRILLED PIER

N.T.S.



Bar Size	Dimension of Standard Bends		
	D	E	F (6d) [3" min.]
#4	2"Ø	3"	3"

### TIE AND STIRRUP BENDS

N.T.S.

APPLICATION #  
02-121895

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A separate project application  
for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
GAILARD R. RAJIC  
No. 52386  
EXPIRES: 12-31-2024  
STATE OF CALIFORNIA

THREE COLUMN  
CAISSON -  
BOLTED

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

SSG JOB # S23109

SHEET

SB3.2



SOLAR PANEL ATTACHMENT PER SRS.1  
 SOLAR PANEL SHALL FIT WITHIN THE DESIGN WIDTH AND HEIGHT. THE ADDITIONAL WEIGHT SHALL BE ADDED TO THE ASSEMBLY WEIGHT AND SHALL NOT EXCEED THE MAX. WEIGHT.

DASHED LINE INDICATES EXTENT OF SCOREBOARD COMPONENTS. ANY COMBINATION OF NEVCO SCOREBOARD PARTS IS PERMISSIBLE IF IT MEETS THE WIDTH, HEIGHT, AND WEIGHT LIMITATIONS NOTED IN THE TABLE.

AD PANEL, DECORATIVE ARCH TRUSS PANEL AS OCCURS.

SPEAKER SECTION BETWEEN COLUMNS, SEE F/SRS.1 FOR ATTACHMENT TO COLUMNS.

SCOREBOARD PANELS.

NETTING SUPPORT OPTION PER A/SRS.2, TYP. OF 4.

COLUMN BASE PLATE. REFER TO H/SRS.3 FOR CONSTRUCTION.

COLUMN ANCHORAGE. REFER TO G/SRS.3 FOR CONSTRUCTION.

FINISH GRADE.

MAT FOOTING. REFER TO C/SRS.3 FOR CONSTRUCTION.

MAX WIDTH, "W" ASSEMBLY WIDTH.

FLAG: 6'X10' MAX. W = 5006 MAX.

FLAG POLE: HSS4.00X60.250.

DOME BRACING PER F/SRS.1.

T.O. SIGN.

DECORATIVE TRUSS OPTION PER SRS.3 OR SRS.4.

AT 20FT. WIDE-SEN.

TYP.

B.O. SIGN.

COLUMN SIZE PER SCHEDULE.

GROUNDING PER B/SRS.1.

6" MAX.

6'-8" MIN. WHEN NETTING PROVIDED.

10'-0" MAX. 6'-0" MIN.

MAX FLAG POLE HEIGHT = 16'-0".

MAX HEIGHT, "H" ASSEMBLY HEIGHT.

ELEVATION.

**PLAN VIEW**

PEDESTAL  
BASE PLATE  
STEEL COLUMN (1)

#6 LONGITUDINAL BARS T&B @ 10" o.c.

#6 TRANSVERSE BARS T&B @ 10" o.c. w/ HOOK EA. END

3 #6 TRANSVERSE BARS T&B CENTERED UNDER COLUMN, EQUALLY SPACED, TYP.

EQ. WIDTH A

EQ. LENGTH L

COLUMN SPACING, S

3" 3" 3"

3" C.R. TYP.

CONCRETE MAT FOOTING (1)

TRANSVERSE REINF. T&B. SEE PLAN

**SECTION A-A**

BASE PLATE. REFER TO DETAIL

(3) - #4 TIES WITHIN TOP 5"

6" MAX. DEPTH, B

WIDTH, A

STEEL COLUMN (1)

ANCHOR RODS w/ LEVELING NUTS (1)

HIGH STRENGTH NON SHRINK GROUT. MAX. HEIGHT PER A/SB3.3

LONG. BARS. SEE PLAN

TRANS. BARS. SEE PLAN

1/2" THK X 36" STEEL GAGE PLATE AT ANCHOR RODS. MATCH BASE PLATE DIMENSIONS

DOUBLE NUTS AT GAGE PLATE

GC OPTION TO PROVIDE 2" Ø HOLE CENTERED ON PLATE

#4 DOWELS AT 8" o.c. AT PEDESTAL

**SECTION B-B**

TIE BENDS PER D/SB3.3

CONT. BAR AT BEND, TYP.

LONG. BARS. SEE PLAN

11-#4 TIES @ 4" o.c. CENTERED ON STEEL COLUMN. #4 TIES @ 12" o.c. FOR REMAINDER OF MAT FTC.

1. SEE SCOREBOARD ELEVATION A/SB3.3

**MAT FOOTING CONSTRUCTION AND ANCHORAGE**

Bar Size	Dimension of Standard Hooks and Bends			
	D	A	B (4d <sub>b</sub> ) [2½" min.]	C (12d <sub>b</sub> )
#3	2½" Ø	3"	2½"	4½"
#4	3" Ø	4"	2½"	6"
#5	3¾" Ø	5"	2½"	7½"
#6	4½" Ø	6"	3"	9"
#7	5½" Ø	7"	3½"	10½"
#8	6" Ø	8"	4"	12"
#9	9" Ø	11½"	4½"	13½"
#10	10" Ø	12½"	5"	15"

Bar Size	Concrete Reinforcing Splices (1)(2)		
	Class A (3) Splice	Class B (3) Splice	$f_y$ (Min.)
#3	16"	21"	40 ksi
#4	21"	27"	60 ksi
#5	40"	51"	60 ksi
#6	47"	61"	60 ksi
#7	63"	81"	60 ksi
#8	72"	93"	60 ksi
#9	81"	106"	60 ksi
#10	91"	118"	60 ksi

Bar Size	Dimension of Standard Bends		
	D	E	F (6d <sub>b</sub> ) [3" min.]
#3	1½"Ø	3"	3"
#4	2"Ø	3"	3"
#5	2½"Ø	3¾"	3¾"



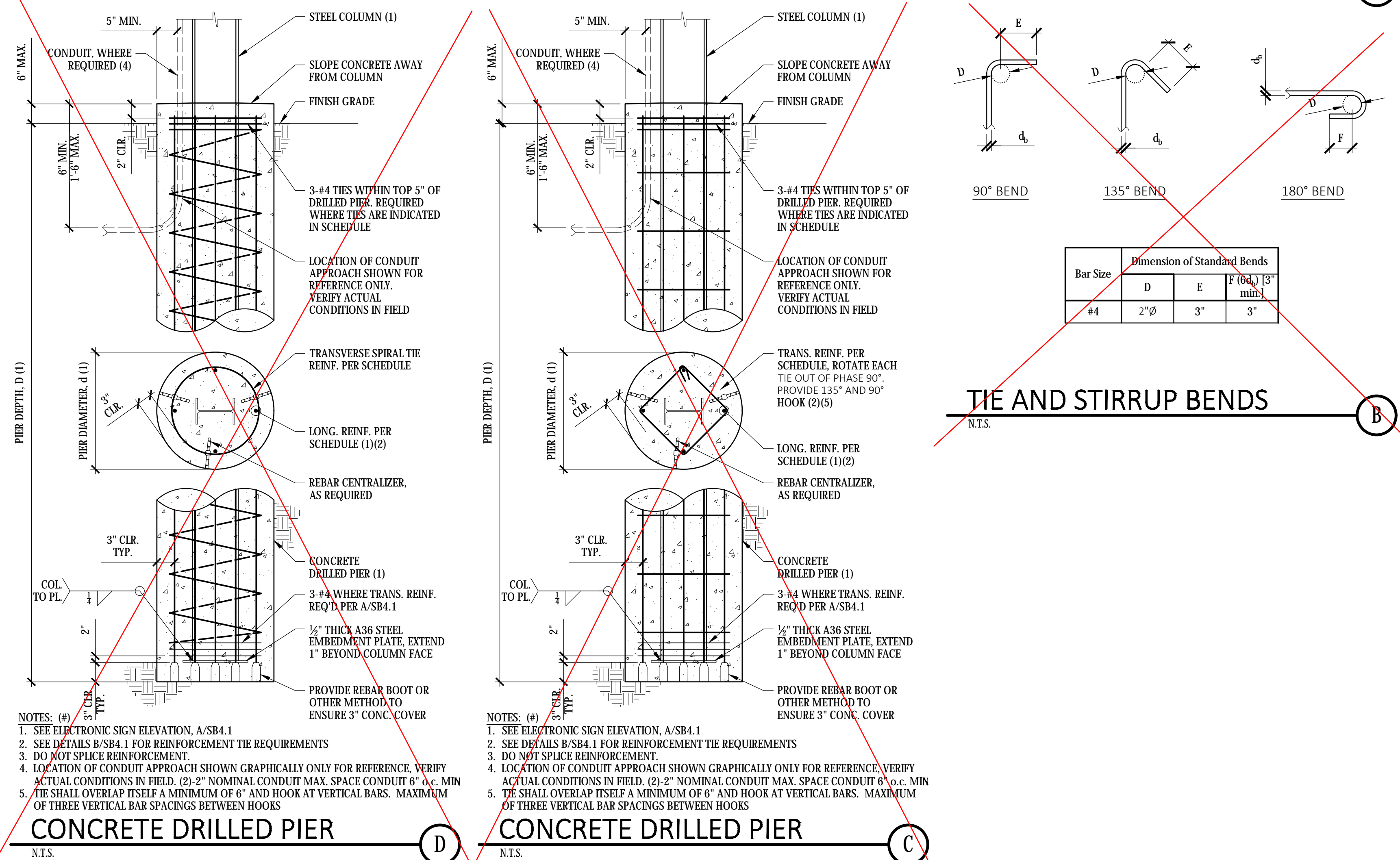
FOUR COLUMN ASSEMBLY										
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA			PIER FOOTING CRITERIA					
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (2)
32'-0"		3,080 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R	N/R
		4,610 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	42"Ø	10'-0"	4-#5	#4 @ 8" o.c.
		6,150 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	42"Ø	11'-6"	4-#5	#4 @ 8" o.c.
		7,680 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	42"Ø	12'-0"	4-#5	#4 @ 8" o.c.
		9,220 lbs.	≤ 24'-0"	8'-0"	W18x106	W18x86	48"Ø	13'-6"	4-#5	#4 @ 8" o.c.
		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x97	48"Ø	14'-8"	4-#5	#4 @ 8" o.c.
36'-0"		12,290 lbs.	≤ 32'-0"	8'-0"	W21x166	W21x132	54"Ø	15'-0"	4-#6	#4 @ 8" o.c.
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	54"Ø	17'-0"	4-#6	#4 @ 8" o.c.
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R	N/R
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	4-#5	#4 @ 8" o.c.
		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-0"	4-#5	#4 @ 8" o.c.
		8,640 lbs.	≤ 20'-0"	10'-0"	W18x89	W16x77	48"Ø	12'-6"	4-#5	#4 @ 8" o.c.
40'-0"		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	48"Ø	13'-6"	4-#5	#4 @ 8" o.c.
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	54"Ø	14'-0"	4-#6	#4 @ 8" o.c.
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	54"Ø	15'-0"	4-#6	#4 @ 8" o.c.
		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	54"Ø	16'-6"	4-#6	#4 @ 6" o.c.
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	10'-0"	N/R	N/R
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	4-#5	#4 @ 8" o.c.
44'-0"		7,680 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	12'-0"	4-#5	#4 @ 8" o.c.
		9,600 lbs.	≤ 20'-0"	10'-0"	W18x89	W16x77	48"Ø	13'-0"	4-#5	#4 @ 8" o.c.
		11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	48"Ø	14'-8"	4-#6	#4 @ 8" o.c.
		13,440 lbs.	≤ 28'-0"	10'-0"	W18x143	W18x119	48"Ø	15'-0"	4-#6	#4 @ 8" o.c.
		15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	54"Ø	16'-0"	4-#6	#4 @ 8" o.c.
		17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	54"Ø	17'-0"	4-#6	#4 @ 8" o.c.
		4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-6"	N/R	N/R
		6,340 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-6"	4-#5	#4 @ 8" o.c.
		8,450 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	12'-0"	4-#5	#4 @ 8" o.c.
		10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	48"Ø	13'-6"	4-#5	#4 @ 8" o.c.
		12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	48"Ø	14'-6"	4-#6	#4 @ 8" o.c.
		14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	54"Ø	15'-6"	4-#6	#4 @ 8" o.c.
		16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	54"Ø	16'-3"	4-#6	#4 @ 8" o.c.
		19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	60"Ø	17'-0"	6-#6	#4 @ 8" o.c.

NOTES:  
1. N/R = REINFORCEMENT NOT REQUIRED PER DSA RU 09-06  
2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB4.1 FOR TIE OPTION. SEE D/SB4.1 FOR SPIRAL OPTION.  
3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

SECTION A-A  
COLUMN  
ORIENTATION

## FOUR COLUMN SCOREBOARD INSTALLATION

N.T.S.



IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-121895 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

SSG  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. PARSONS  
No. 5405  
STRUCTURAL  
STATE OF CALIFORNIA  
DATE SIGNED: 08.09.2023  
PC SEOR SEAL

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nevco

301 East Harris Avenue, Greenville, Illinois 62246  
Phone: (618) 664-0360  
www.nevco.com

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122347 PC  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application  
for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
GAILLARD R. RAJIC  
No. 52356  
EXPIRES 12/31/2024  
STRUCTURAL  
STATE OF CALIFORNIA

FOUR COLUMN  
CAISSON -  
EMBEDDED

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB4.1

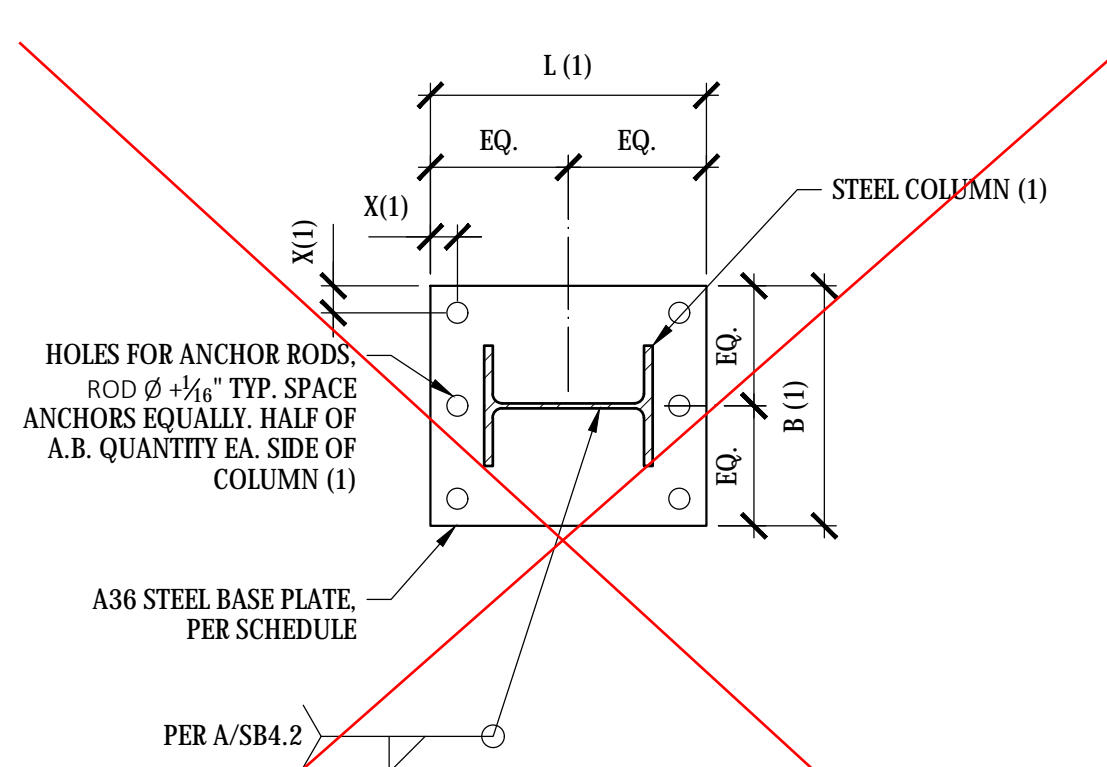


FOUR COLUMN ASSEMBLY																			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA				PIER FOOTING CRITERIA				BASE PLATE				ANCHOR RODS					
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
32'-0"		3,080 lbs.	± 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		4,610 lbs.	± 12'-0"	8'-0"	W14x48	W14x43	42"Ø	10'-0"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,150 lbs.	± 16'-0"	8'-0"	W14x61	W14x61	42"Ø	11'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		7,680 lbs.	± 20'-0"	8'-0"	W16x77	W16x67	42"Ø	12'-9"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		9,220 lbs.	± 24'-0"	8'-0"	W18x106	W18x86	48"Ø	13'-6"	14 - #9	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	70"
		10,760 lbs.	± 28'-0"	8'-0"	W18x130	W18x97	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	70"
36'-0"		12,290 lbs.	± 32'-0"	8'-0"	W21x166	W21x132	54"Ø	15'-0"	18 - #9	#4 @ 6" o.c.	2"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		13,830 lbs.	± 36'-0"	8'-0"	W21x201	W21x166	54"Ø	17'-0"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
		3,460 lbs.	± 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,190 lbs.	± 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,920 lbs.	± 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-0"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(4) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		8,640 lbs.	± 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
40'-0"		10,370 lbs.	± 24'-0"	10'-0"	W18x119	W18x97	48"Ø	13'-6"	14 - #9	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		12,100 lbs.	± 28'-0"	10'-0"	W18x130	W18x106	54"Ø	14'-0"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		13,830 lbs.	± 32'-0"	10'-0"	W21x166	W21x147	54"Ø	15'-0"	18 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		15,560 lbs.	± 36'-0"	10'-0"	W21x223	W21x166	54"Ø	16'-6"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	70"
		3,840 lbs.	± 8'-0"	10'-0"	W10x33	W10x33	36"Ø	10'-0"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,760 lbs.	± 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
44'-0"		7,680 lbs.	± 16'-0"	10'-0"	W16x67	W16x67	48"Ø	12'-0"	10 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		9,600 lbs.	± 20'-0"	10'-0"	W16x89	W16x77	48"Ø	13'-0"	12 - #8	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		11,520 lbs.	± 24'-0"	10'-0"	W18x119	W18x97	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		13,440 lbs.	± 28'-0"	10'-0"	W18x143	W18x119	48"Ø	15'-0"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	64"
		15,360 lbs.	± 32'-0"	10'-0"	W21x182	W21x147	54"Ø	16'-0"	22 - #9	#4 @ 6" o.c.	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
		17,280 lbs.	± 36'-0"	10'-0"	W21x223	W21x182	54"Ø	17'-0"	27 - #9	#4 @ 6" o.c.	3"	30"	42"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2½"	70"
44'-0"		4,230 lbs.	± 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,340 lbs.	± 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		8,450 lbs.	± 16'-0"	12'-0"	W16x67	W16x67	48"Ø	12'-0"	10 - #8	#4 @ 6" o.c.	1½"	24"	30"	½"	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		10,560 lbs.	± 20'-0"	12'-0"	W18x97	W18x76	48"Ø	13'-6"	12 - #8	#4 @ 6" o.c.	1½"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		12,680 lbs.	± 24'-0"	12'-0"	W18x130	W18x106	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
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		19,010 lbs.	± 36'-0"	12'-0"	W21x223	W21x201	60"Ø	17'-0"	27 - #9	#4 @ 6" o.c.	3"	30"	42"	CIP	(10) - 2"Ø	F1554 - GR.105	4"	2½"	70"

NOTES:  
1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB4.2 FOR TIE OPTION. SEE D/SB4.2 FOR SPIRAL OPTION.  
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

## FOUR COLUMN SCOREBOARD INSTALLATION

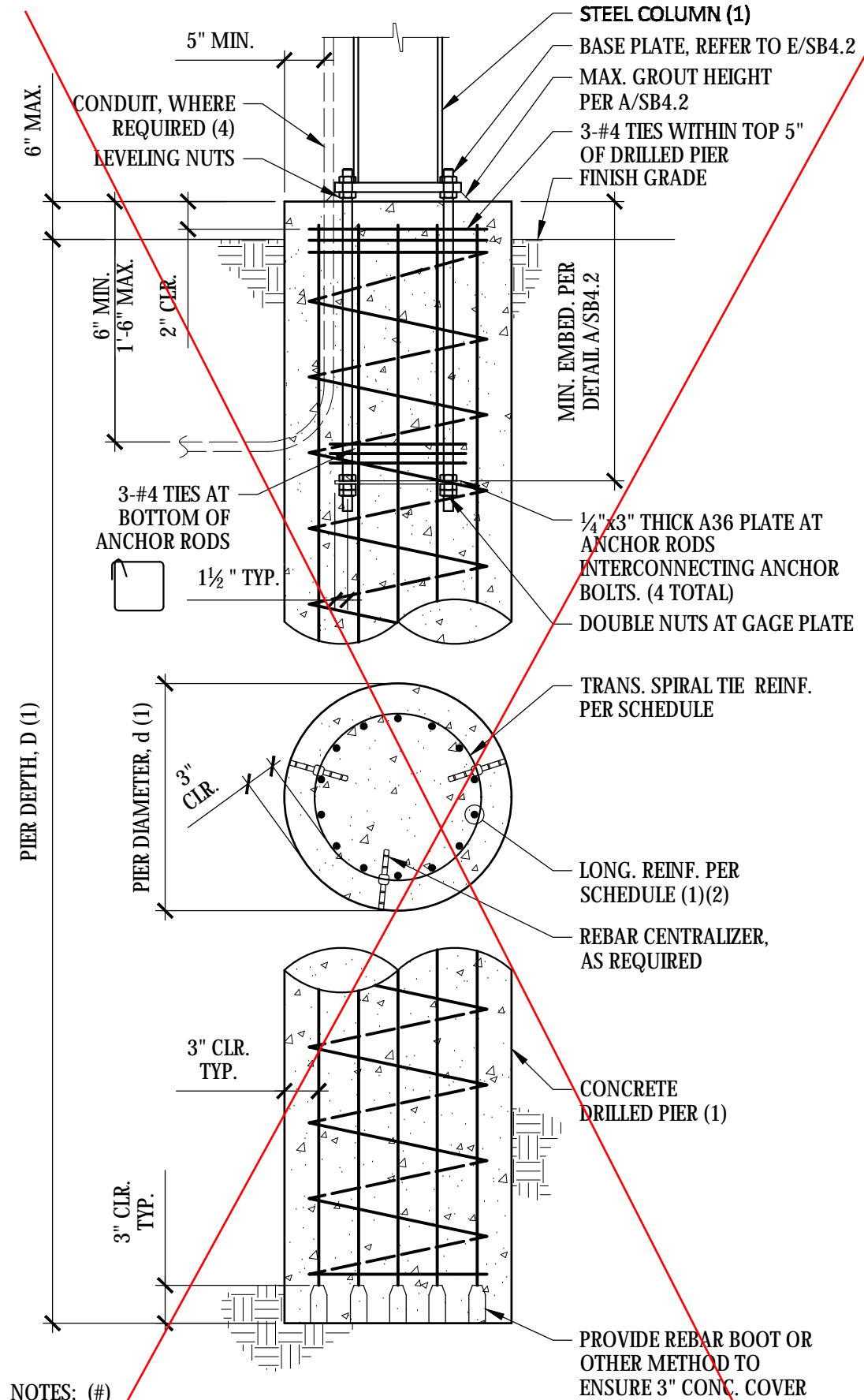
NTS



NOTES: (1)  
1. SEE ELEVATION, A/SB4.2

BASE PLATE

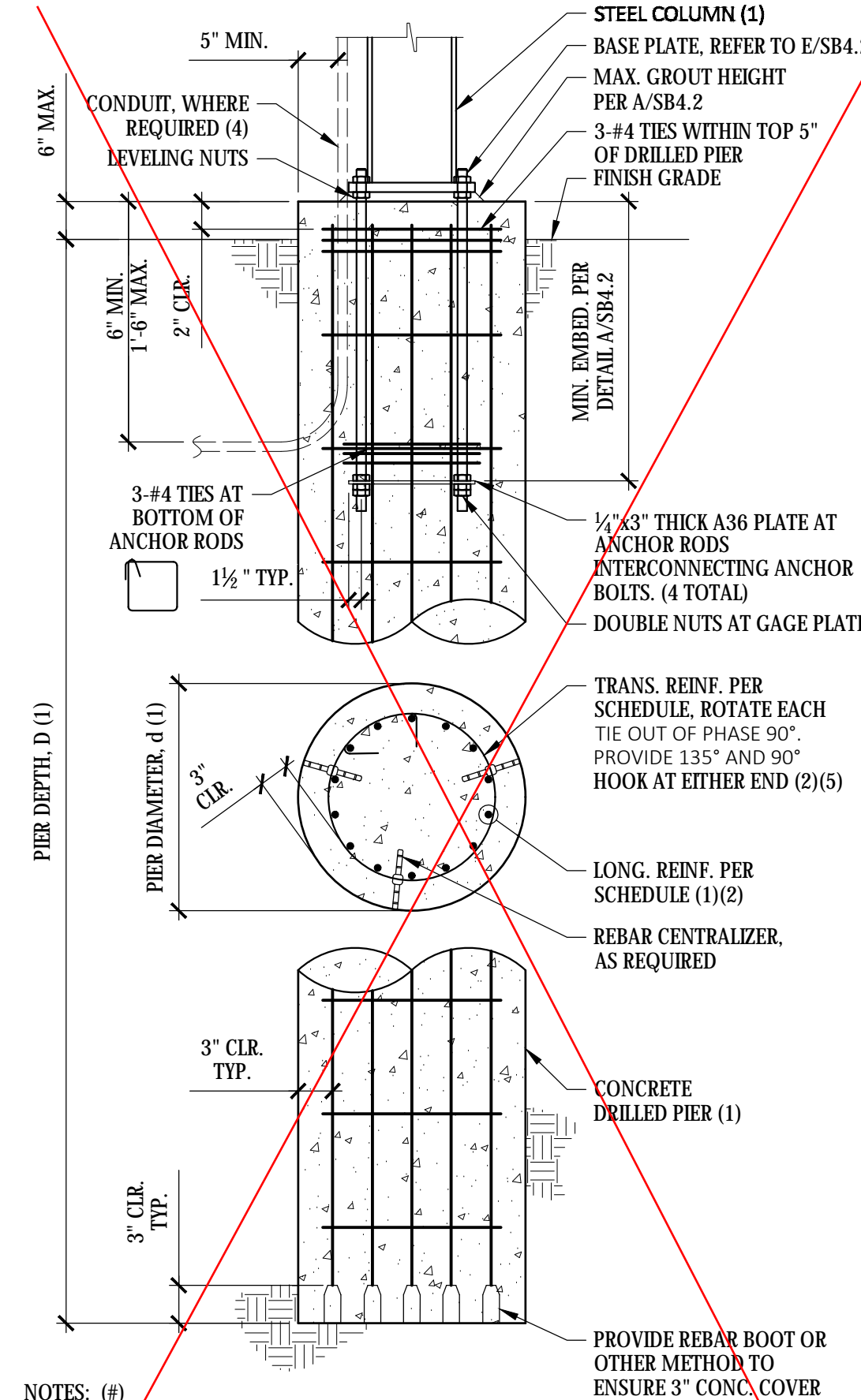
NTS



NOTES: (1)  
1. SEE ELEVATION, A/SB4.2  
2. SEE DETAILS B/SB4.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPICE REINFORCEMENT  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD; (2) 2\"/>

CONCRETE DRILLED PIER

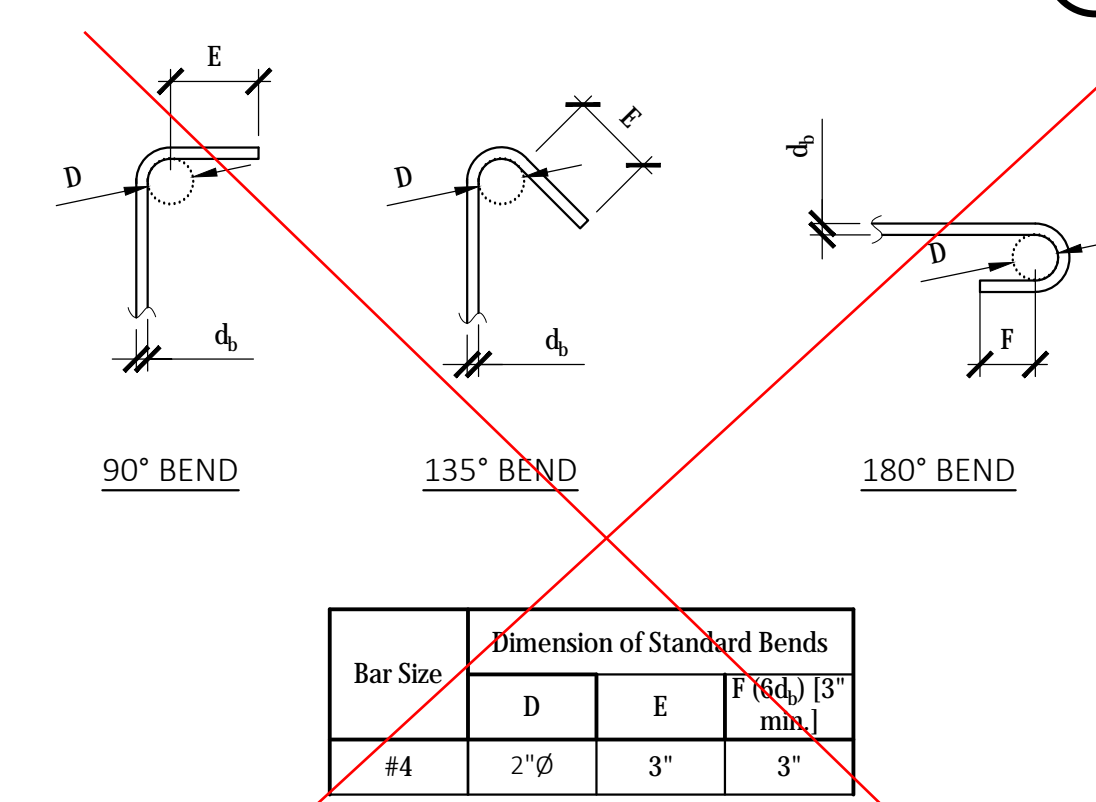
NTS



NOTES: (1)  
1. SEE ELEVATION, A/SB4.2  
2. SEE DETAILS B/SB4.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPICE REINFORCEMENT  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD; (2) 2\"/>

CONCRETE DRILLED PIER

NTS



TIE AND STIRRUP BENDS

NTS

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-121895 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

SSG  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. PARSONS  
No. 5405  
STRUCTURAL  
STATE OF CALIFORNIA  
PC SEOR REAL  
08.09.2023

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nevco  
301 East Harris Avenue, Greenville, Illinois 62246  
Phone: (618) 664-0960  
www.nevco.com

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122347 PC  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application  
for construction is required.

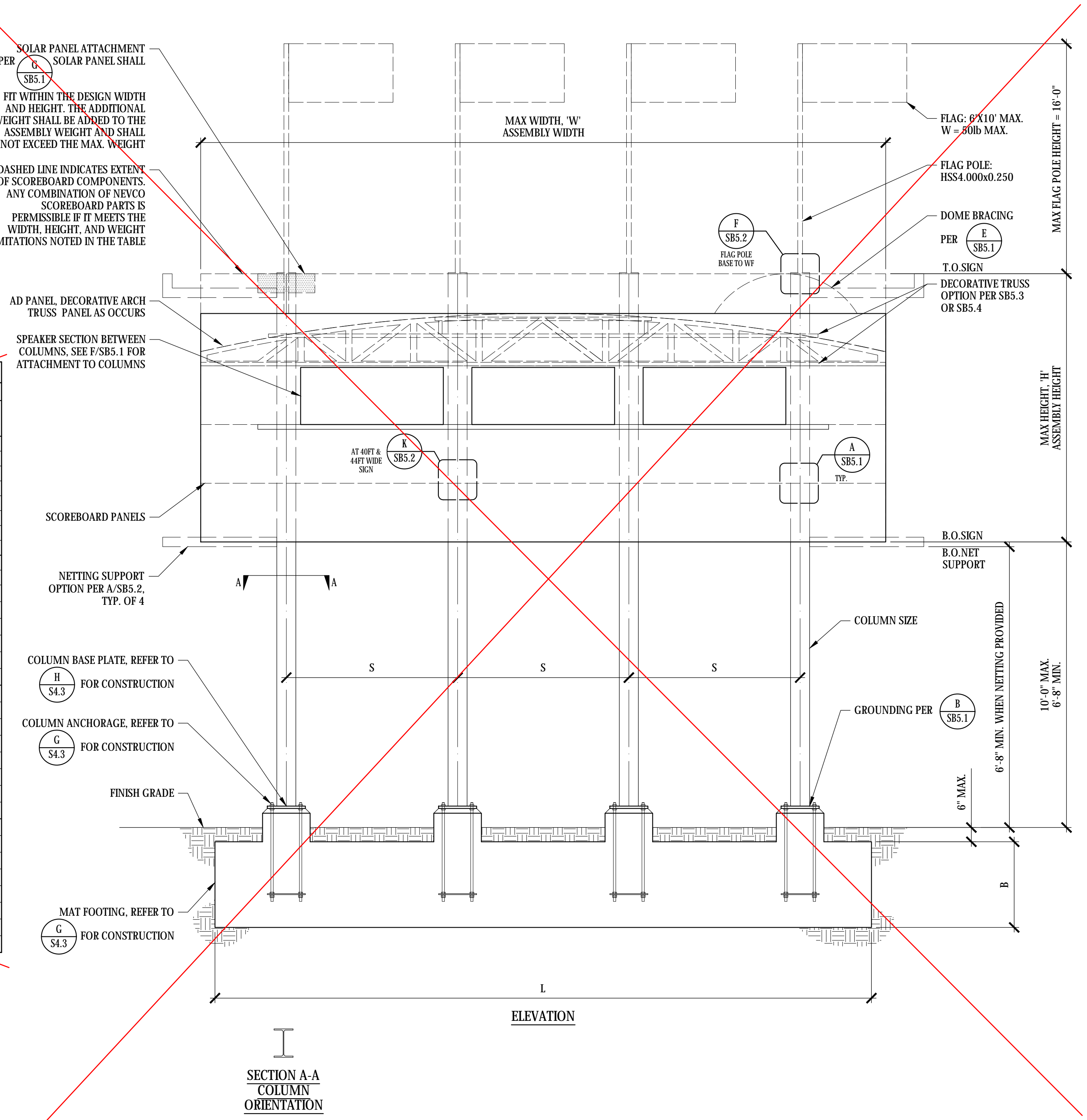
REGISTERED PROFESSIONAL ENGINEER  
GAILLARD R. RAJIC  
No. 52386  
EXPIRY: 06/30/2025  
STRUCTURAL  
STATE OF CALIFORNIA

FOUR COLUMN  
CAISSON -  
BOLTED

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB4.2

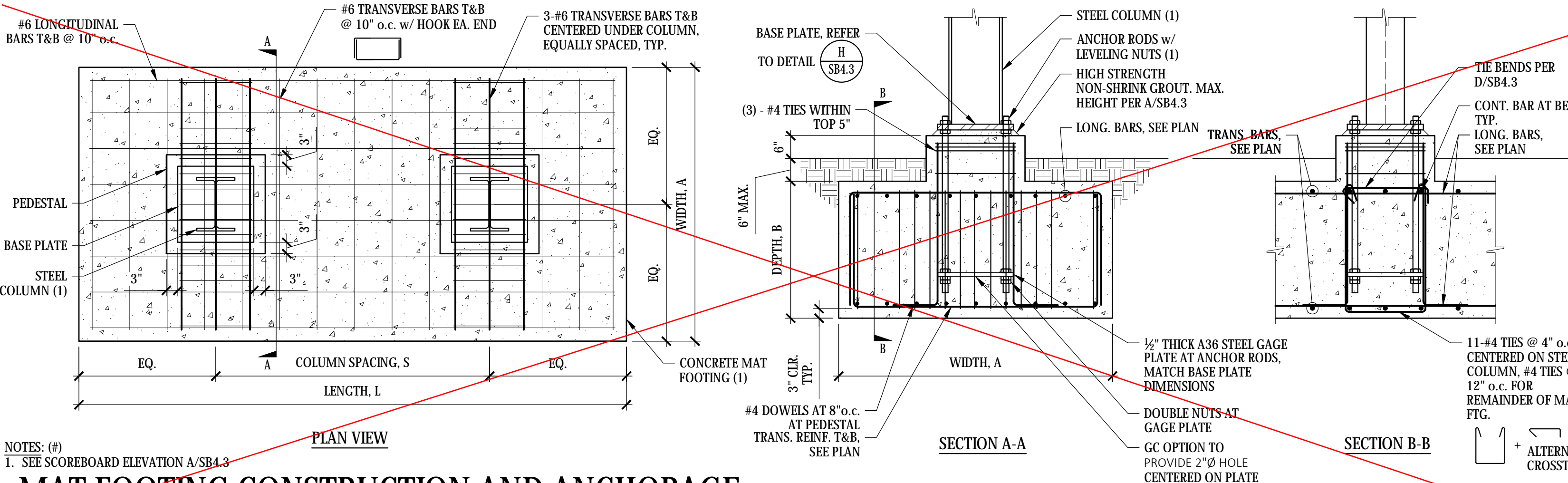


FOUR COLUMN ASSEMBLY															
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	MAT FOOTING CRITERIA			BASE PLATE			ANCHOR RODS		
		MAX. WEIGHT	ASSEMBLY HEIGHT, H				WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE
32'-0"		3,980 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1 1/4"	20"	20"	3/16	(4) - 1 1/2" Ø	F1554 - GR.55
		4,610 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	9'-0"	3'-0"	30'-0"	1 1/4"	24"	24"	3/16	(4) - 1 1/2" Ø	F1554 - GR.55
		6,150 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	35'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
		7,690 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	12'-6"	3'-0"	36'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55
		9,220 lbs.	≤ 24'-0"	8'-0"	W18x106	W18x86	14'-0"	3'-0"	39'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55
		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x97	15'-0"	3'-6"	39'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.105
36'-0"		12,290 lbs.	≤ 32'-0"	8'-0"	W21x166	W21x132	17'-0"	4'-0"	42'-0"	2"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	18'-0"	4'-0"	46'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	7'-6"	3'-0"	36'-0"	1 1/4"	20"	20"	3/16	(4) - 1 1/2" Ø	F1554 - GR.55
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x48	9'-0"	3'-0"	36'-0"	1 1/4"	24"	24"	3/16	(4) - 1 1/2" Ø	F1554 - GR.55
		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	36'-0"	1 1/2"	24"	30"	3/8	(4) - 1 1/2" Ø	F1554 - GR.55
		8,640 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-6"	3'-0"	40'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55
40'-0"		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	14'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	16'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55
		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	18'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	36'-0"	1 1/2"	20"	20"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
44'-0"		7,680 lbs.	≤ 16'-0"	10'-0"	W16x87	W16x67	11'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
		9,600 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	13'-0"	3'-0"	40'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55
		11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.105
		13,440 lbs.	≤ 28'-0"	10'-0"	W18x143	W18x119	14'-0"	4'-0"	42'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105
		15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	16'-0"	4'-0"	44'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105
		17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	18'-0"	4'-0"	46'-0"	3"	30"	42"	CIP	(6) - 2" Ø	F1554 - GR.105
		4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	7'-0"	3'-0"	42'-0"	1 1/2"	20"	20"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
		6,340 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-0"	3'-0"	43'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
		8,450 lbs.	≤ 16'-0"	12'-0"	W16x87	W16x67	10'-0"	3'-0"	45'-0"	1 1/2"	24"	30"	3/8	(6) - 1 1/2" Ø	F1554 - GR.55
		10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	12'-0"	3'-6"	46'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55
		12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	14'-0"	3'-6"	47'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.105
		14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	15'-0"	4'-0"	48'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105
		16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	16'-0"	4'-0"	49'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105
		19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	18'-0"	4'-0"	52'-0"	3"	30"	42"	CIP	(10) - 2" Ø	F1554 - GR.105



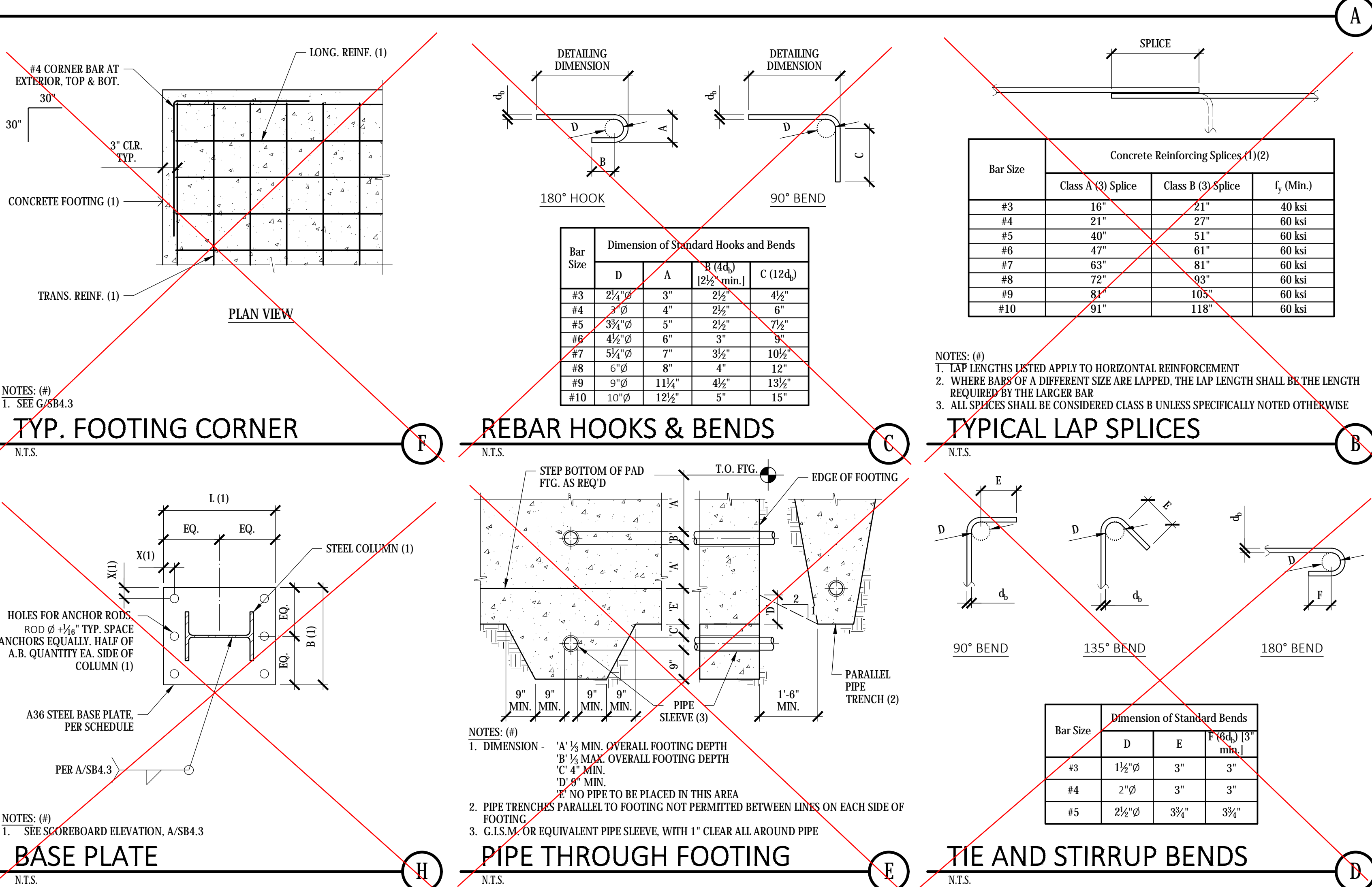
## FOUR COLUMN SCOREBOARD INSTALLATION

NTS.



## MAT FOOTING CONSTRUCTION AND ANCHORAGE

NTS.



## BASE PLATE

NTS.

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-121895 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

SSG

structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. PARVIZ  
No. 5405  
STATE OF CALIFORNIA  
PC SEOR REAL  
08.09.2023

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SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 09/20/2023

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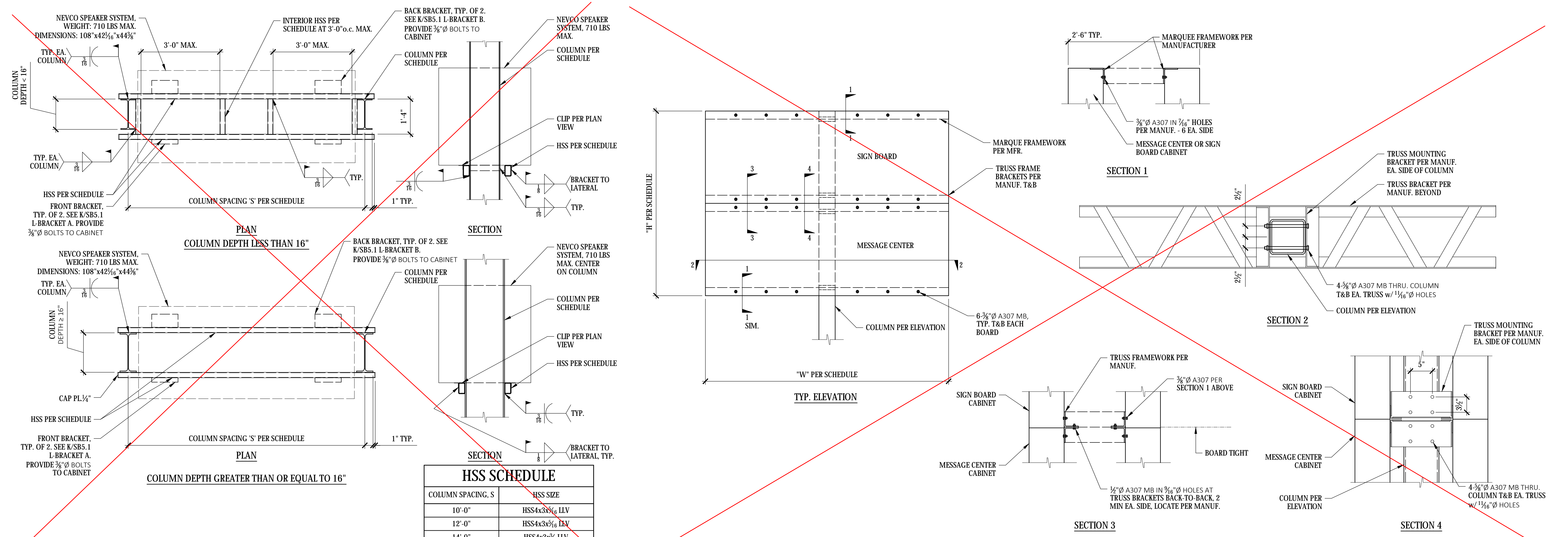
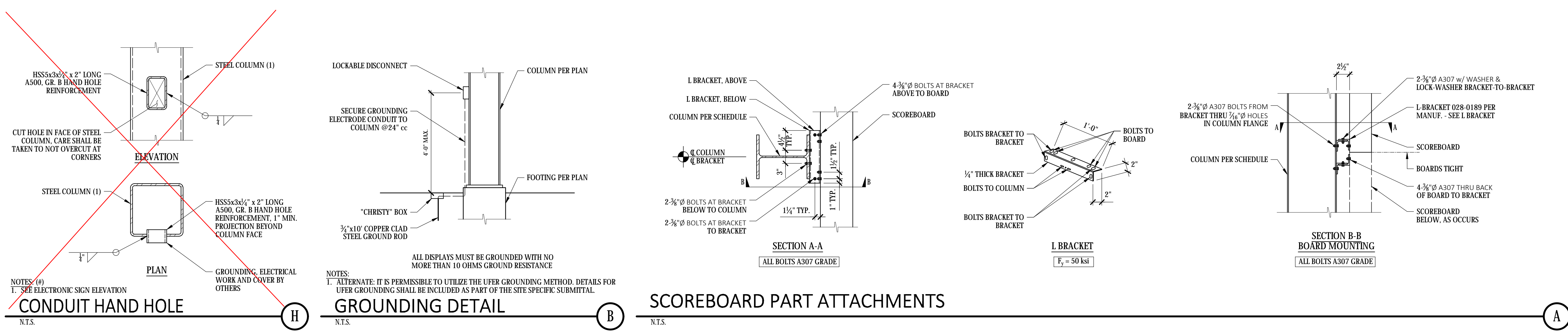
PRE-CHECK (PC) DOCUMENT  
CODE: 2022  
  
A separate project application  
for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
GAILLARD R. RAJIC  
No. 52386  
EXPIRES 12/31/2024  
STATE OF CALIFORNIA

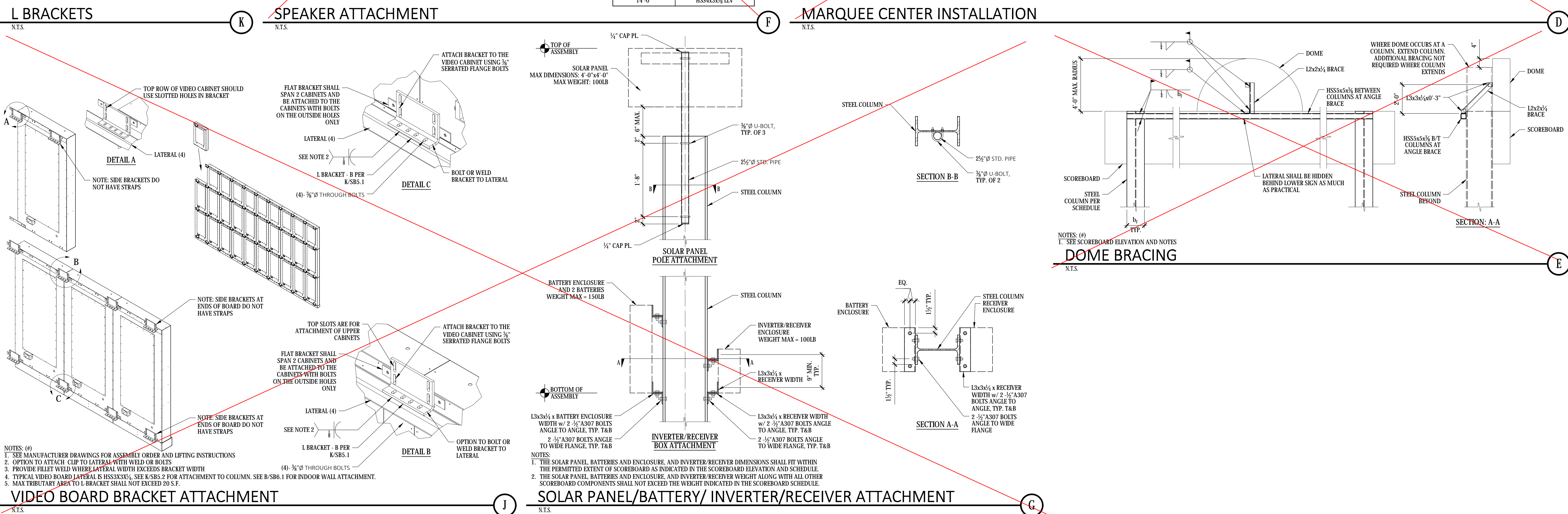
FOUR COLUMN  
MAT FOOTING

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB4.3





HSS SCHEDULE	
COLUMN SPACING, S	HSS SIZE
10'-0"	HSS4x3x1/4 LLV
12'-0"	HSS4x3x1/4 LLV
14'-0"	HSS4x3x1/4 LLV



APPLICATION #  
02-121895

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DATE: 01/24/2024

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structural engineers

REGISTERED PROFESSIONAL ENGINEER  
No. 5405  
STATE OF CALIFORNIA  
DATE SIGNED: 08.09.2023  
PC SEOR SEAL

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DATE: 09/20/2023

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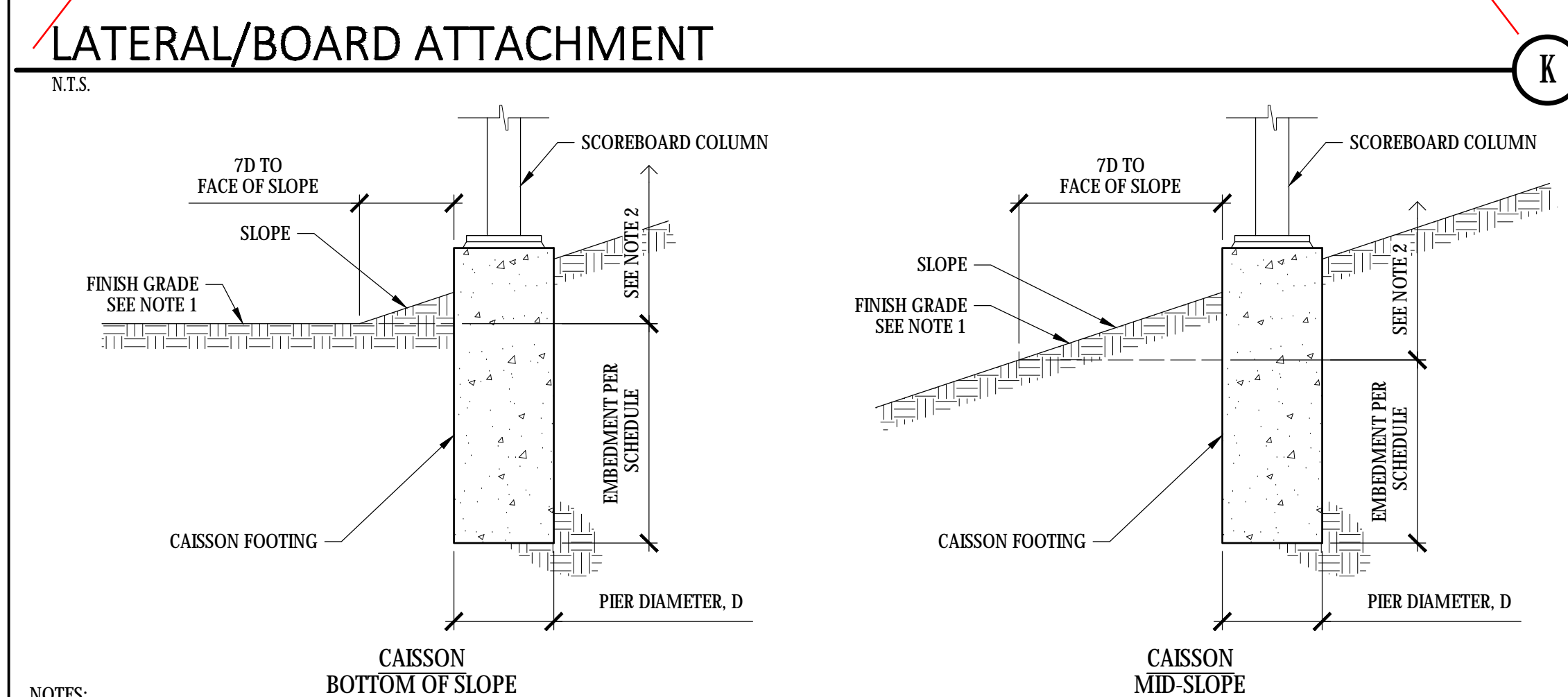
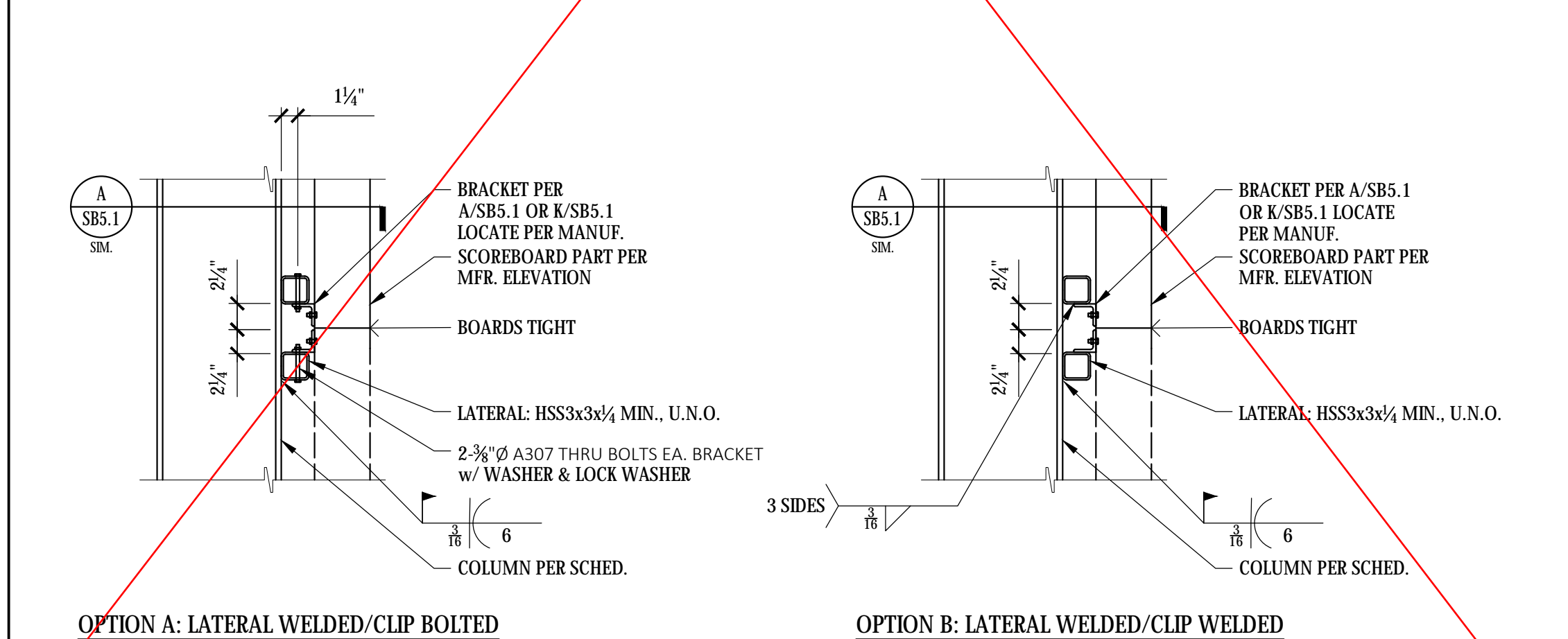
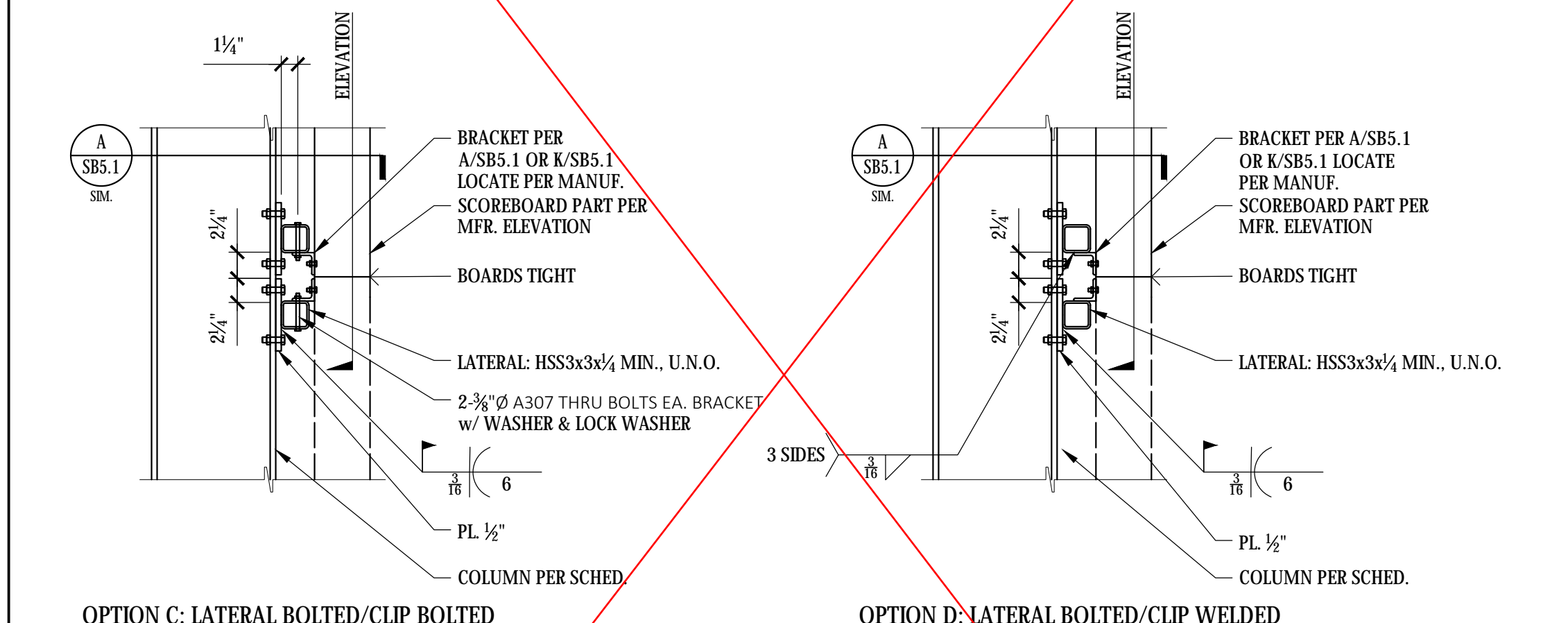
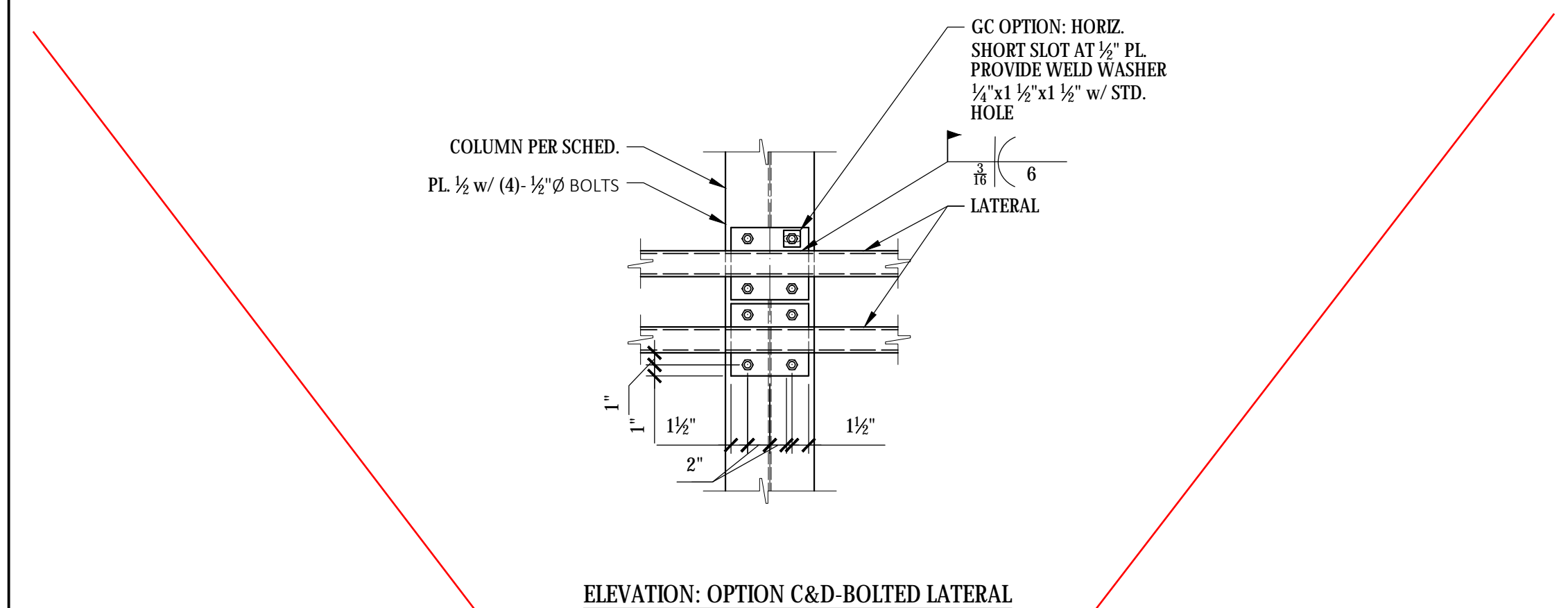
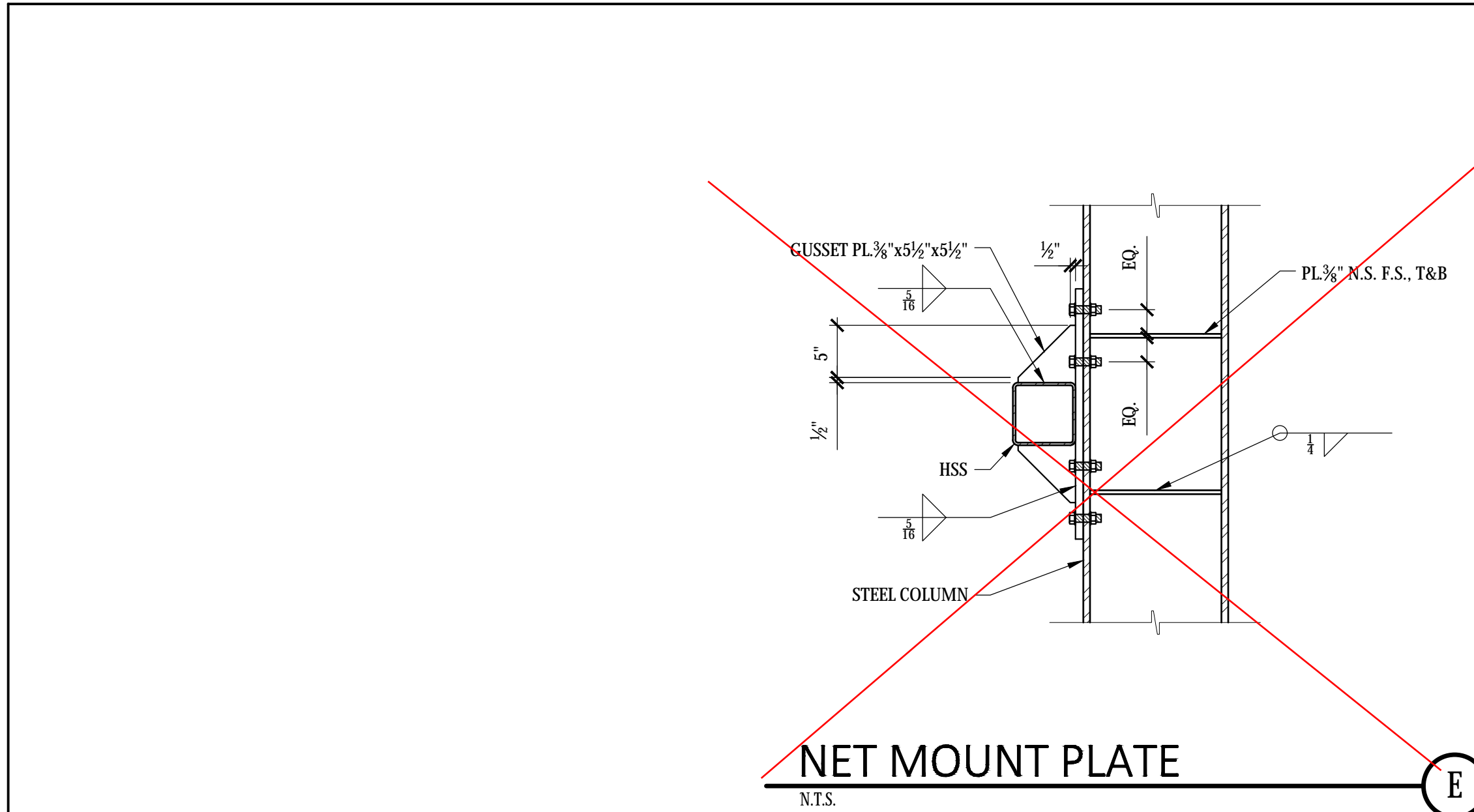
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REGISTERED PROFESSIONAL ENGINEER  
No. 52386  
EXPIRES 09/30/2025  
STATE OF CALIFORNIA

ATTACHMENT  
DETAILS

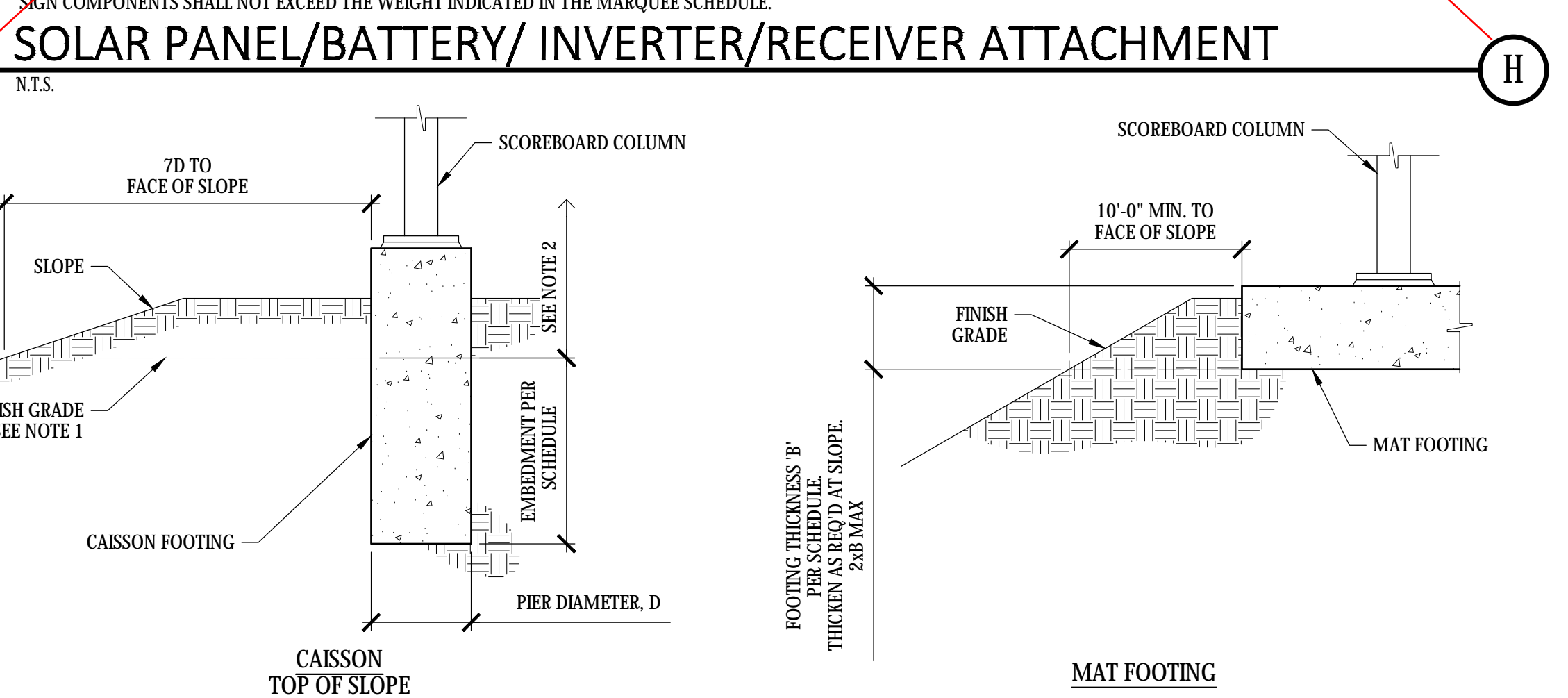
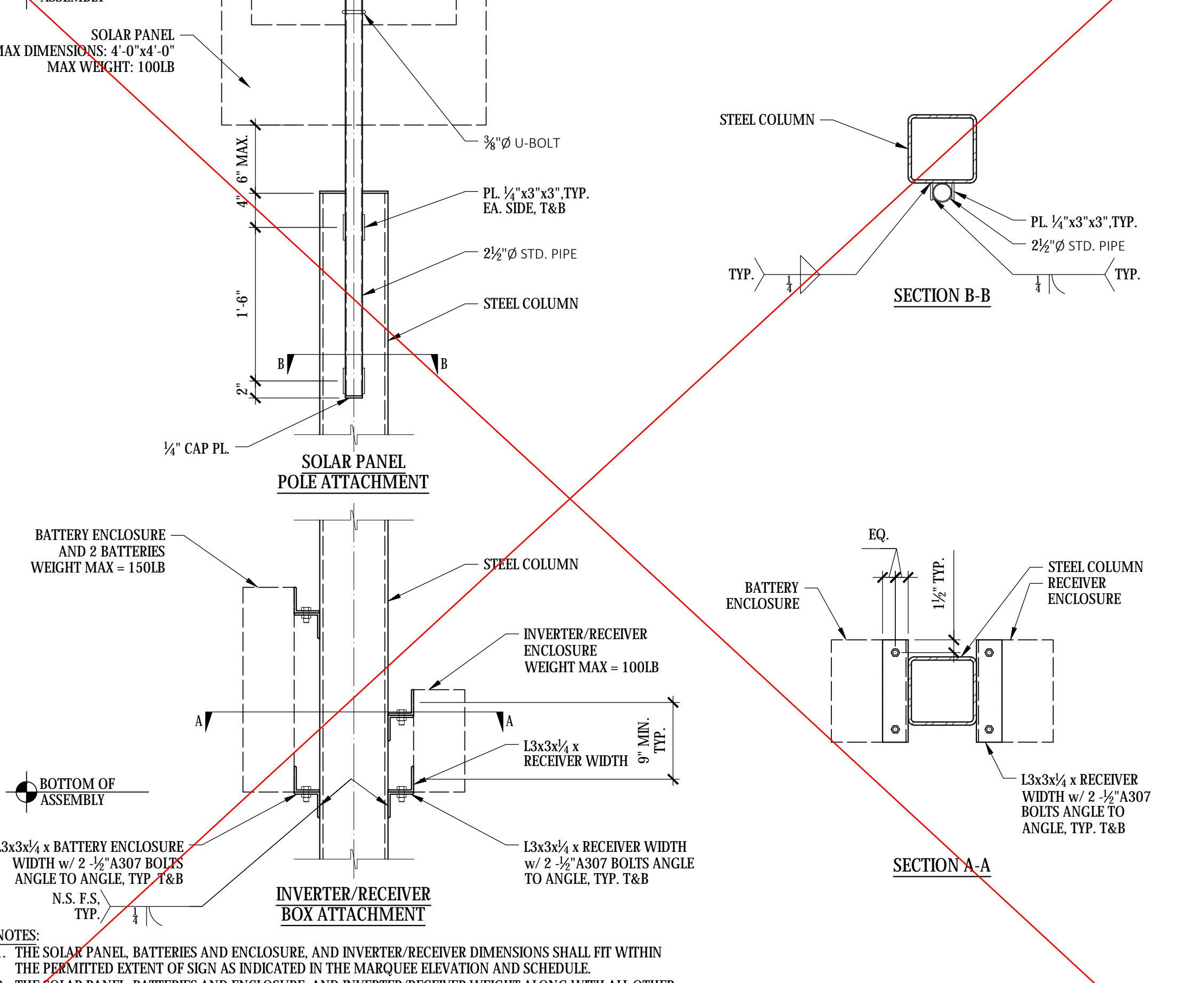
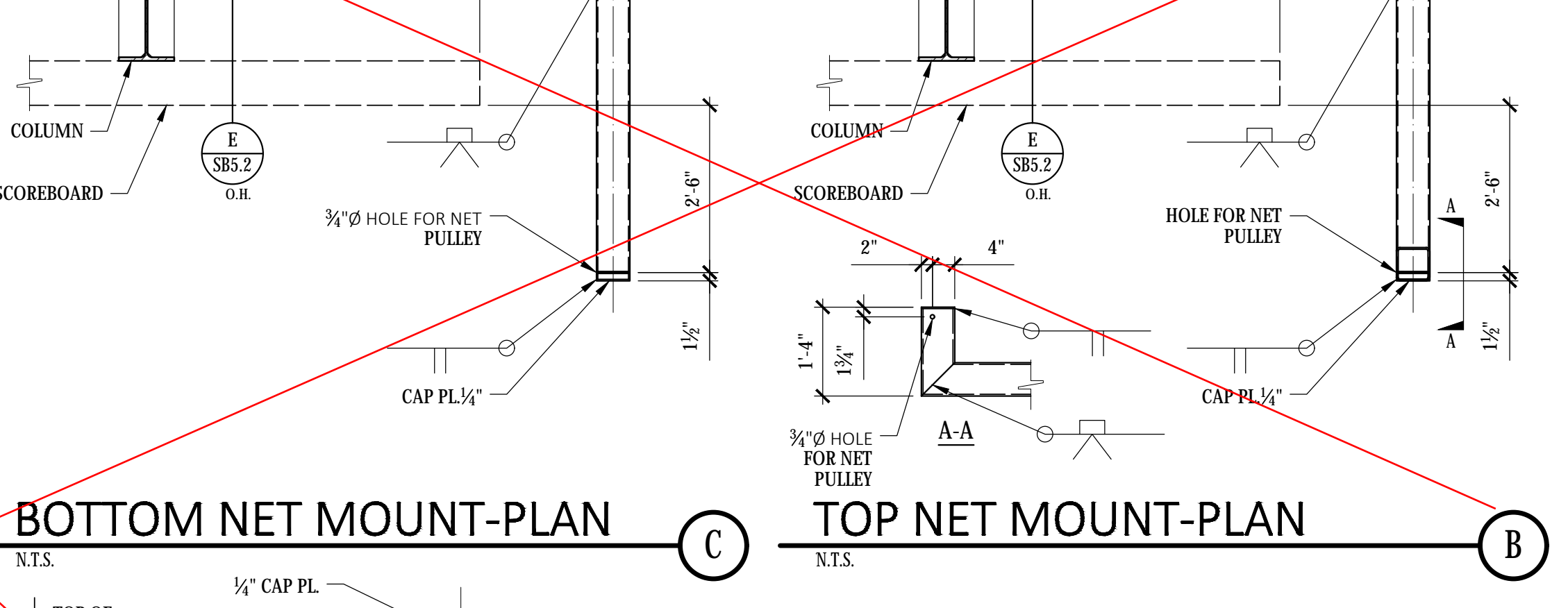
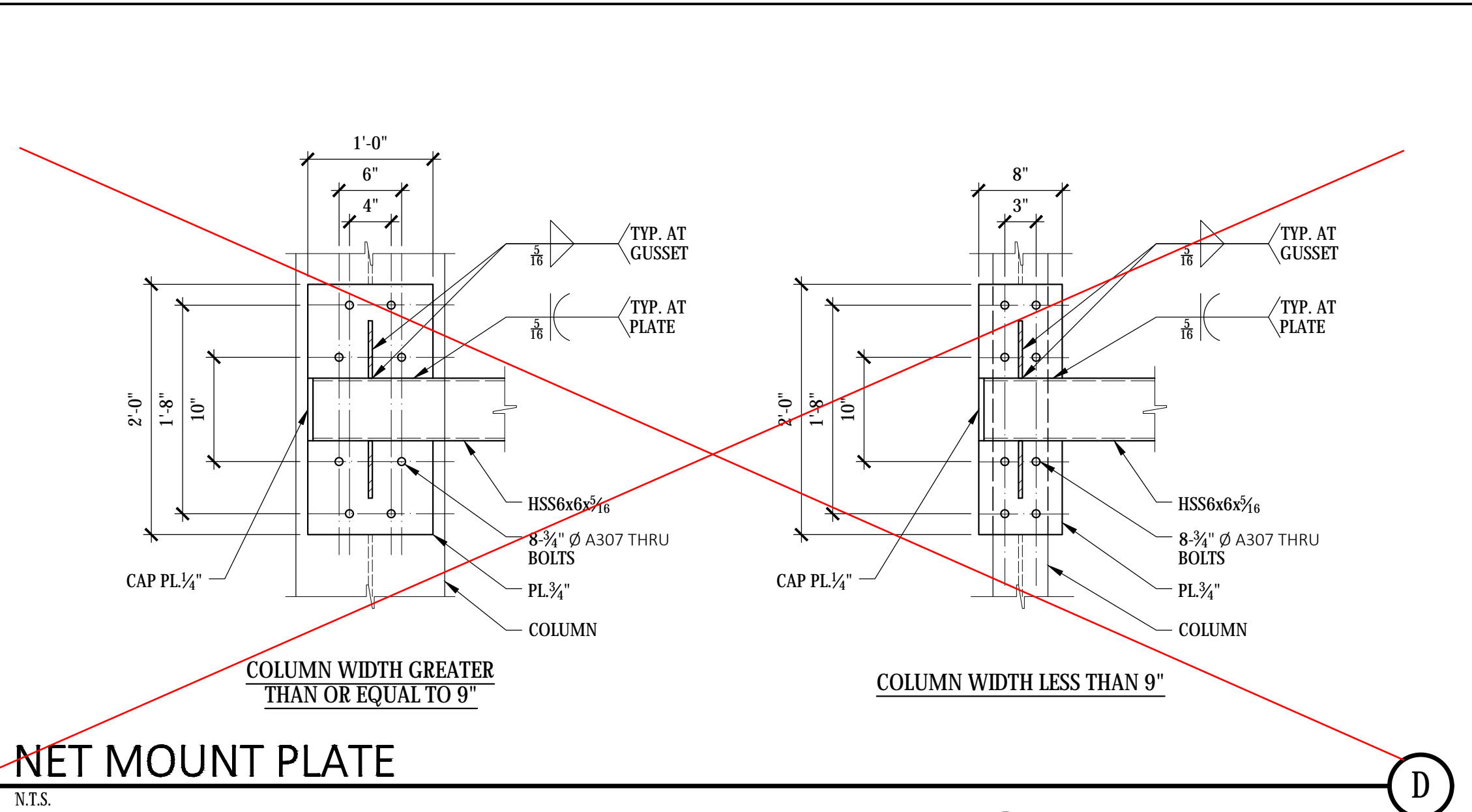
SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB5.1



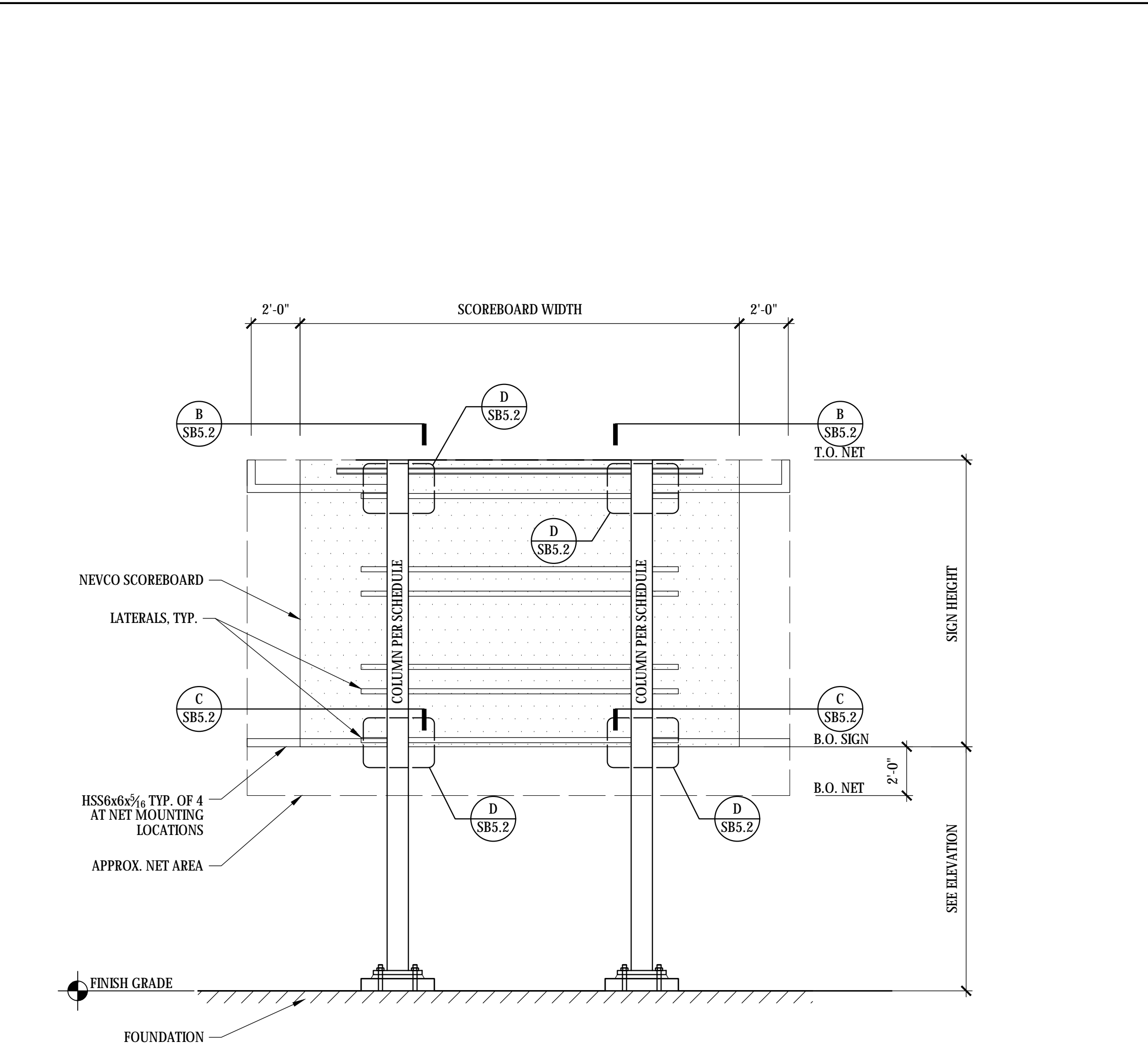


NOTES:  
1. REFERENCE FINISH GRADE FOR DETERMINATION OF MAXIMUM SCOREBOARD HEIGHT.  
2. SEE TYPICAL SCOREBOARD INSTALLATION DETAIL FOR MAXIMUM ASSEMBLY HEIGHT AND CLEARANCE TO BOTTOM OF SCOREBOARD.

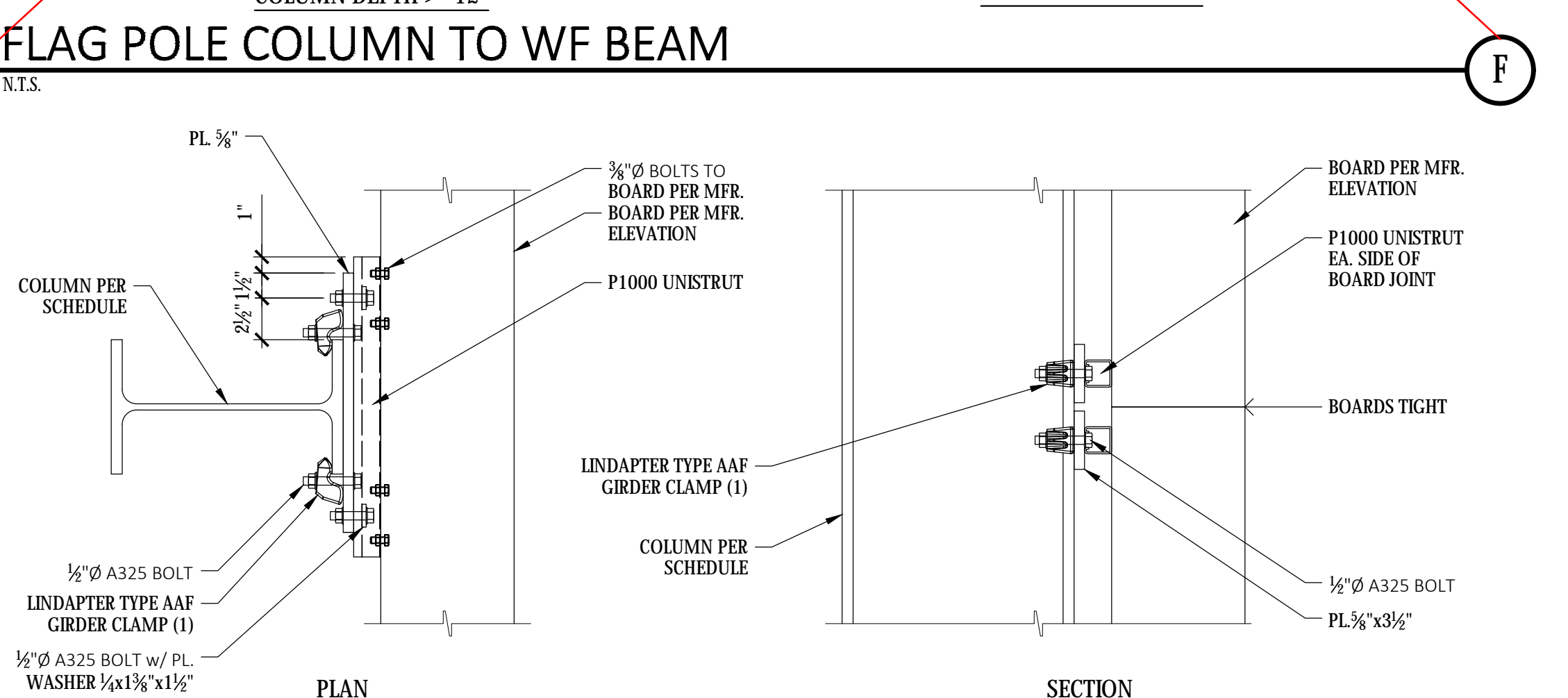
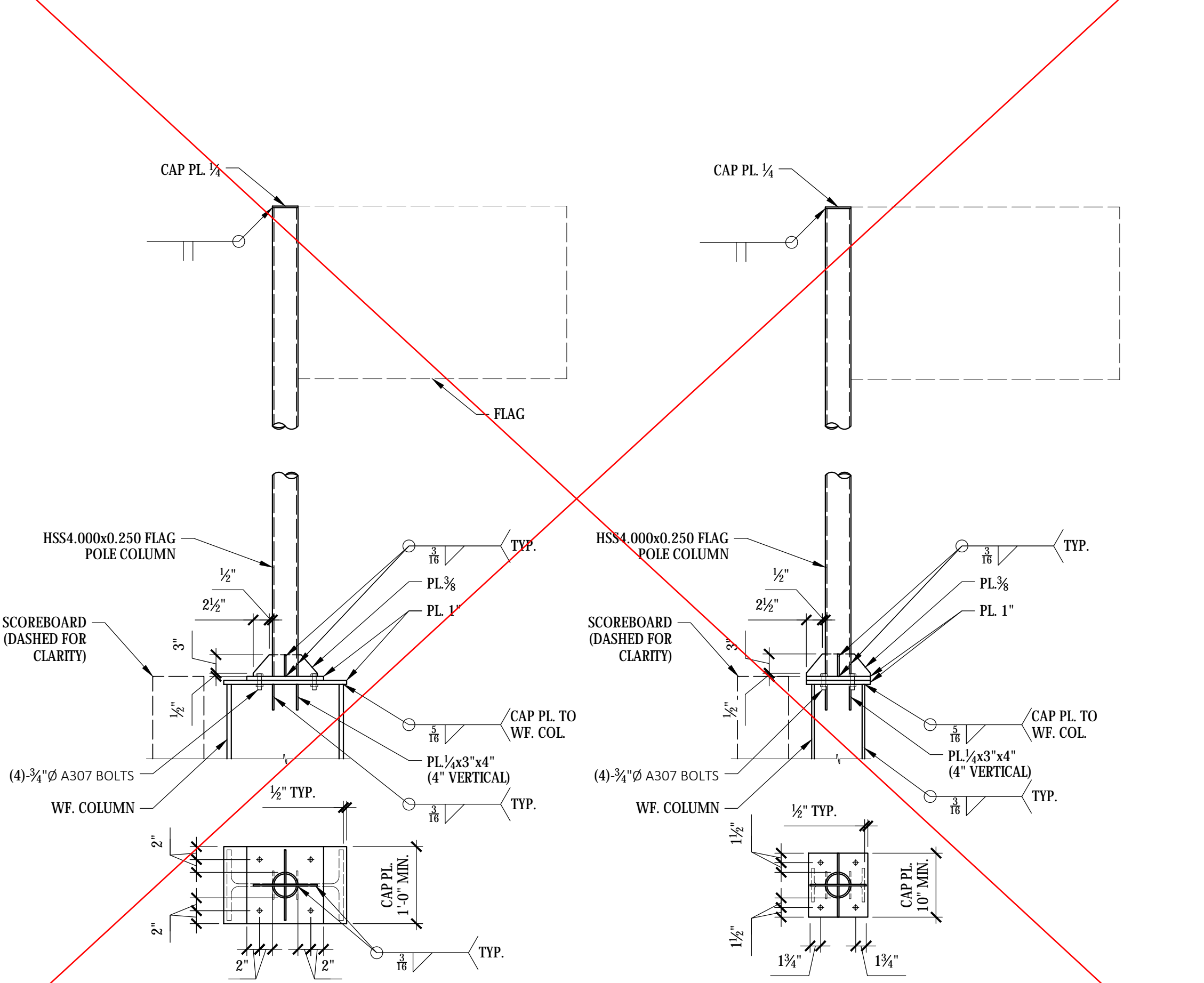
DISTANCE TO SLOPE  
N.T.S.



NOTES:  
1. THE SOLAR PANEL BATTERIES AND ENCLOSURE, AND INVERTER/RECEIVER DIMENSIONS SHALL FIT WITHIN THE PERMITTED EXTENT OF SIGN AS INDICATED IN THE MARQUEE ELEVATION AND SCHEDULE.  
2. THE SOLAR PANEL BATTERIES AND ENCLOSURE, AND INVERTER/RECEIVER WEIGHT ALONG WITH ALL OTHER SIGN COMPONENTS SHALL NOT EXCEED THE WEIGHT INDICATED IN THE MARQUEE SCHEDULE.



NOTES:  
1. ELEVATION SHOWN WITH TWO COLUMNS FOR GRAPHICAL PURPOSES. NET SUPPORT DETAILS APPLICABLE TO TWO, THREE, AND FOUR COLUMN ASSEMBLIES.



NOTES: (H)  
1. LAA050 CLAMP WITH ROCKING WASHER - INSTALL PER ICC ESR-3976

APPLICATION #  
02-121895

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-121895 INC.  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

SSG  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
No. 5405  
STATE OF CALIFORNIA  
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08.09.2023

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CODE: 2022

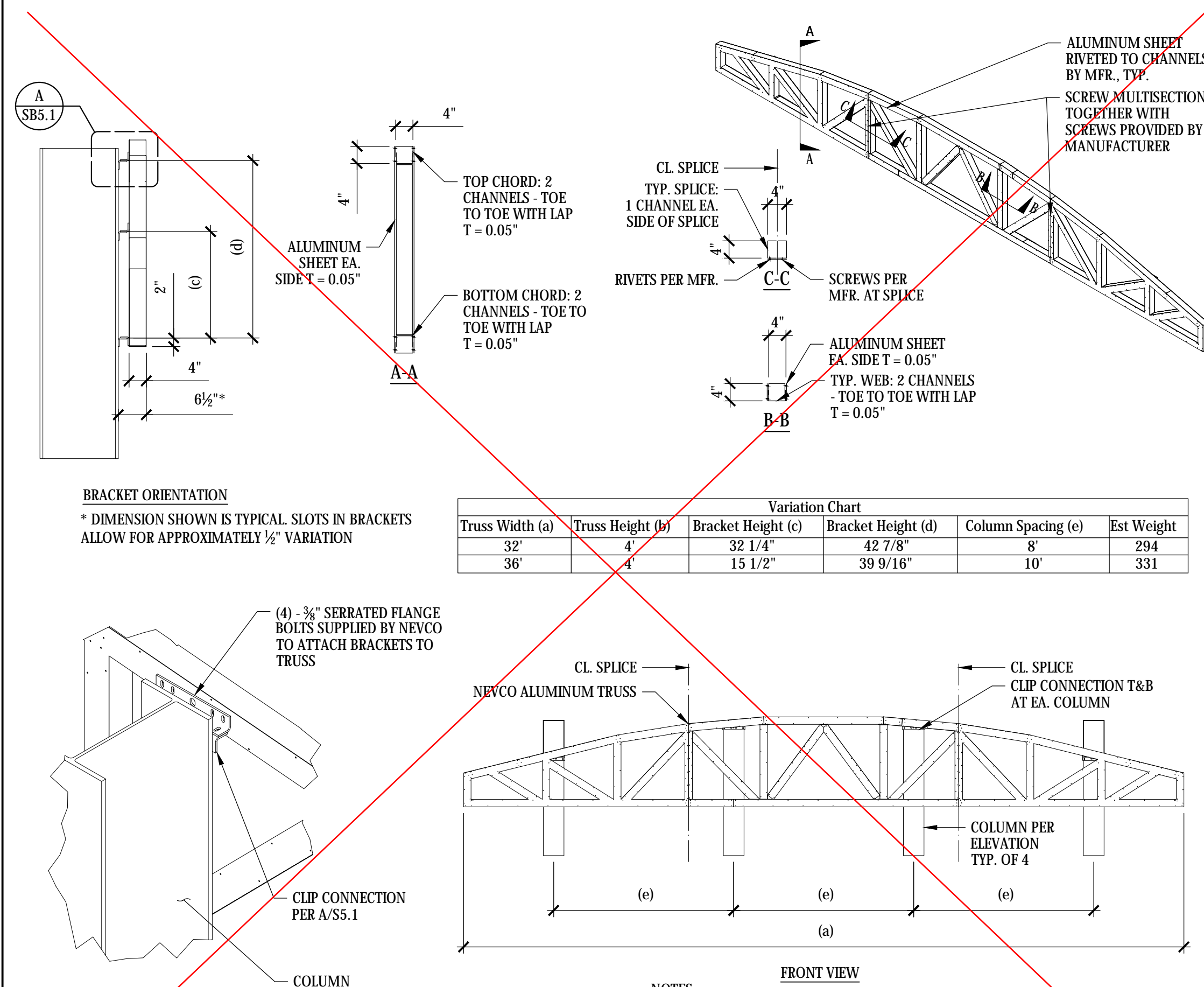
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No. 52386  
EXPIRES 09/20/2024  
STATE OF CALIFORNIA

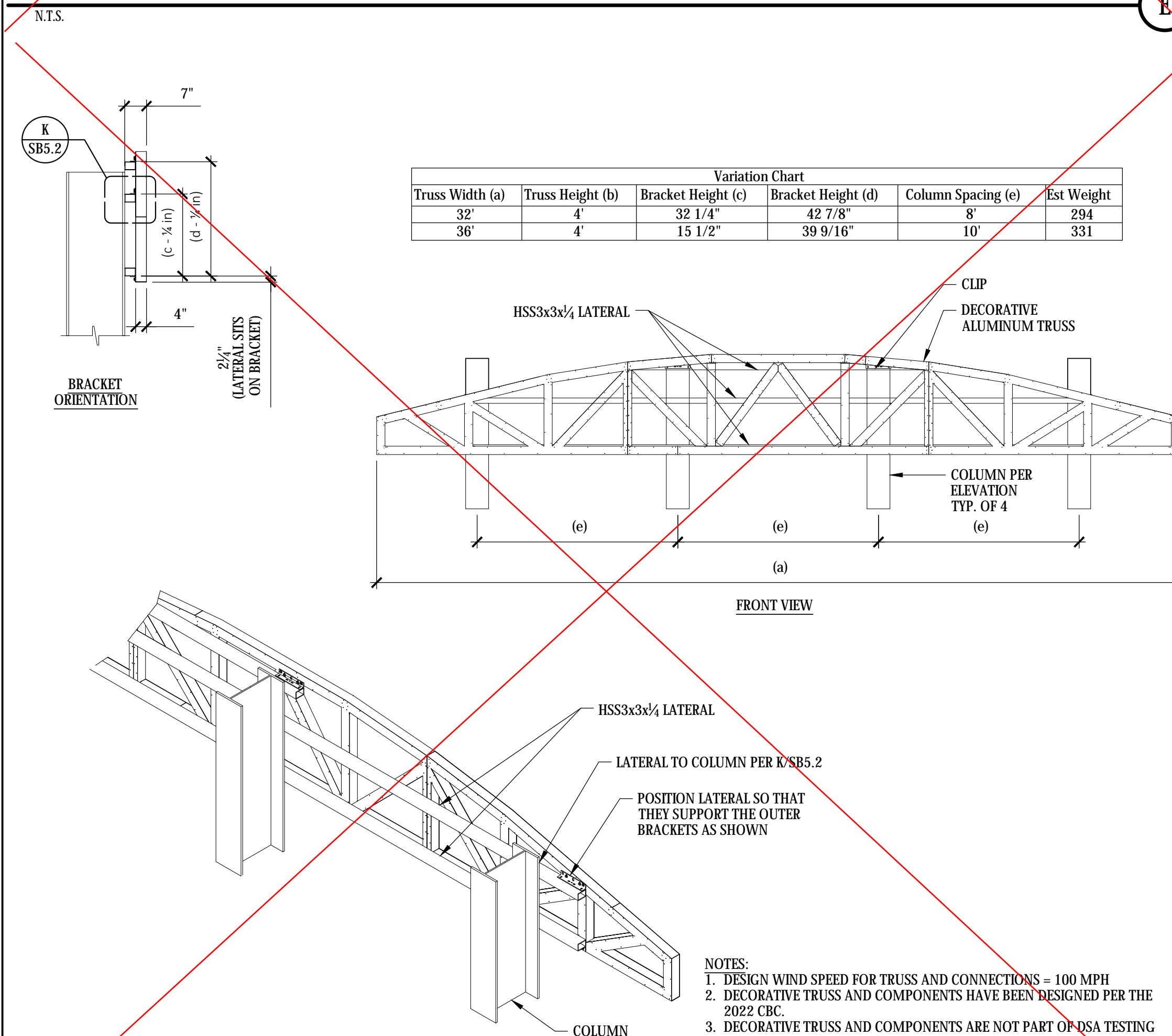
OPTIONAL SCOREBOARD  
FEATURE ATTACHMENT  
DETAILS

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB5.2

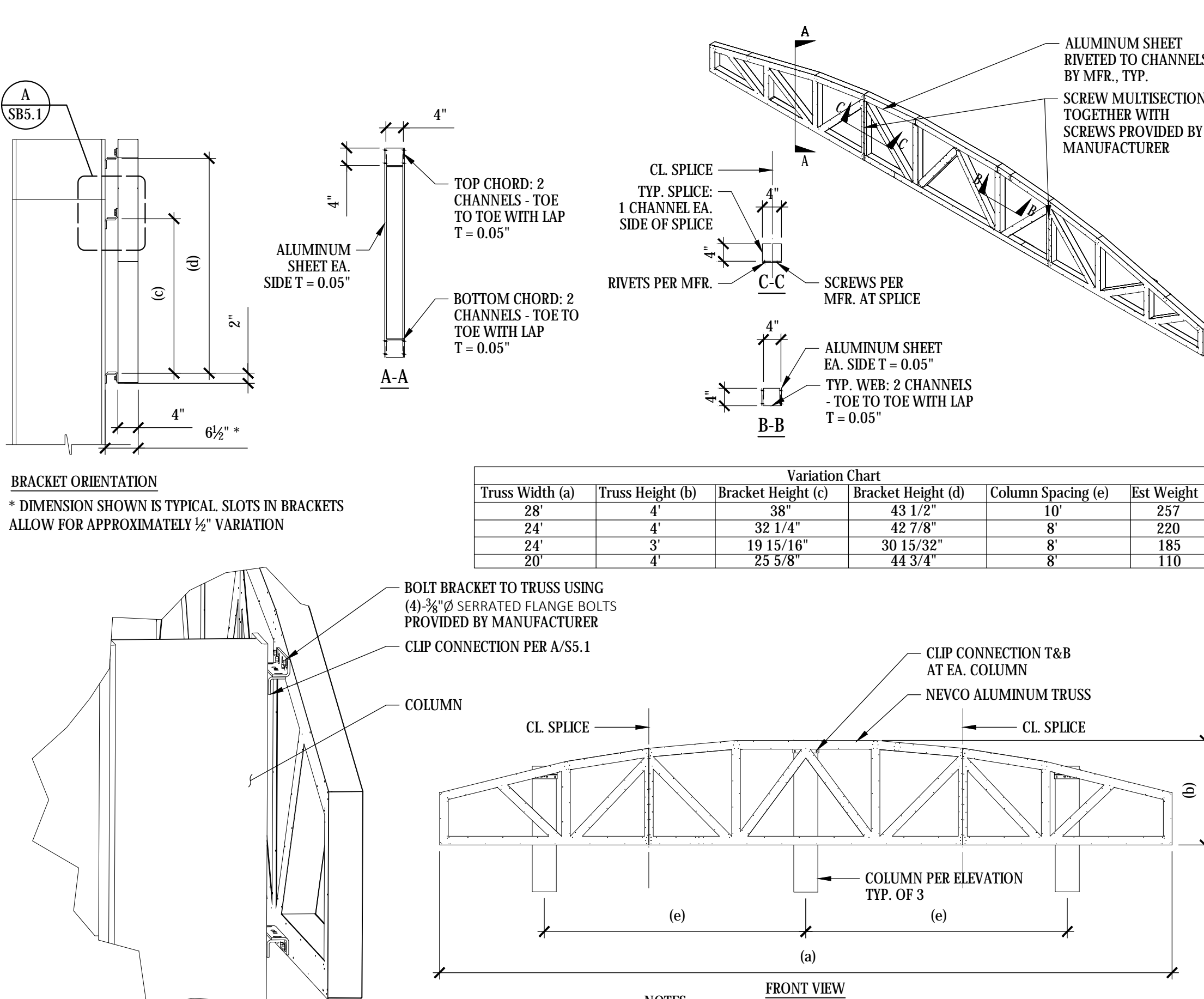




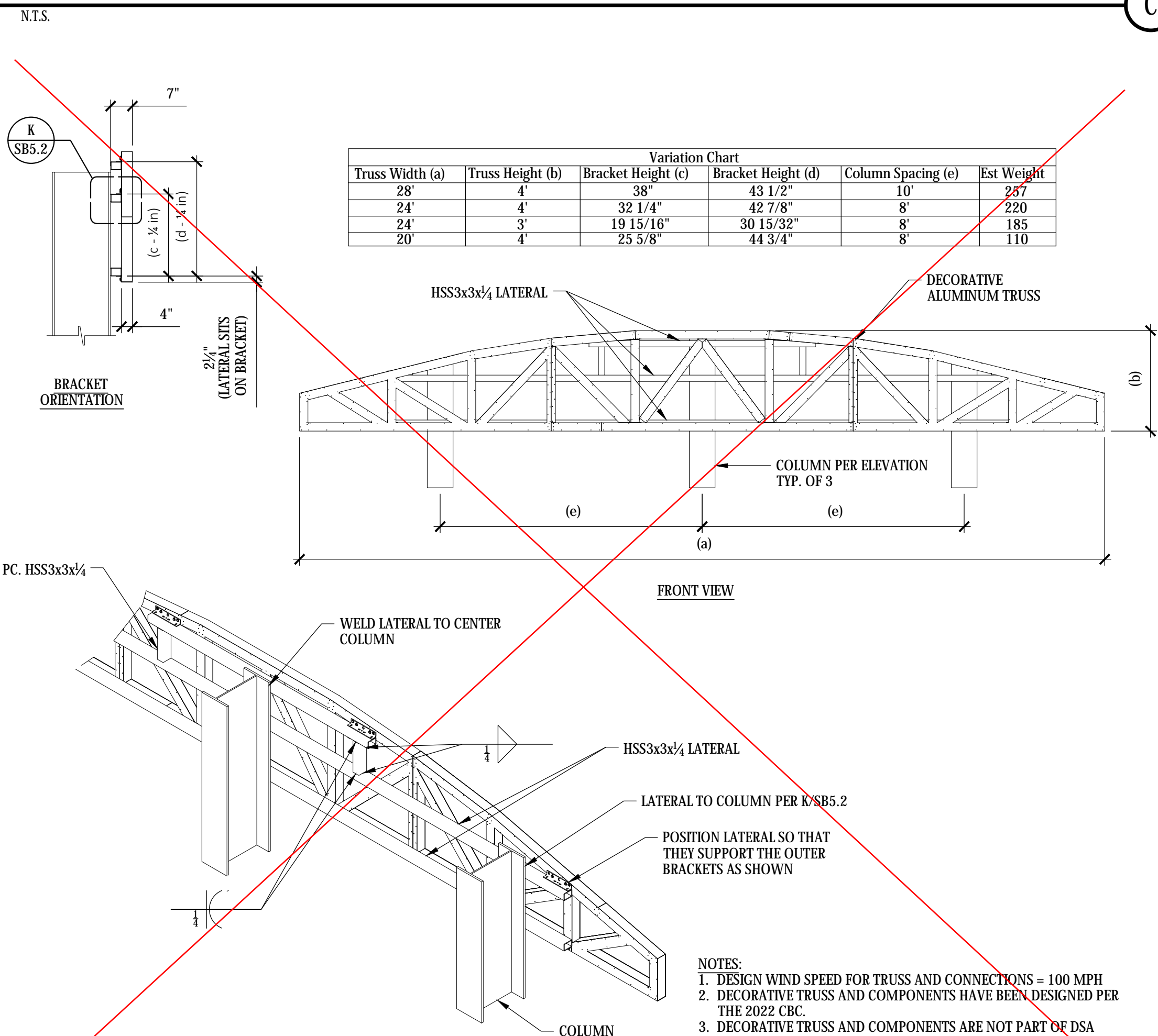
## INSTALLATION OF ALUMINUM TRUSS ON FOUR COLUMNS



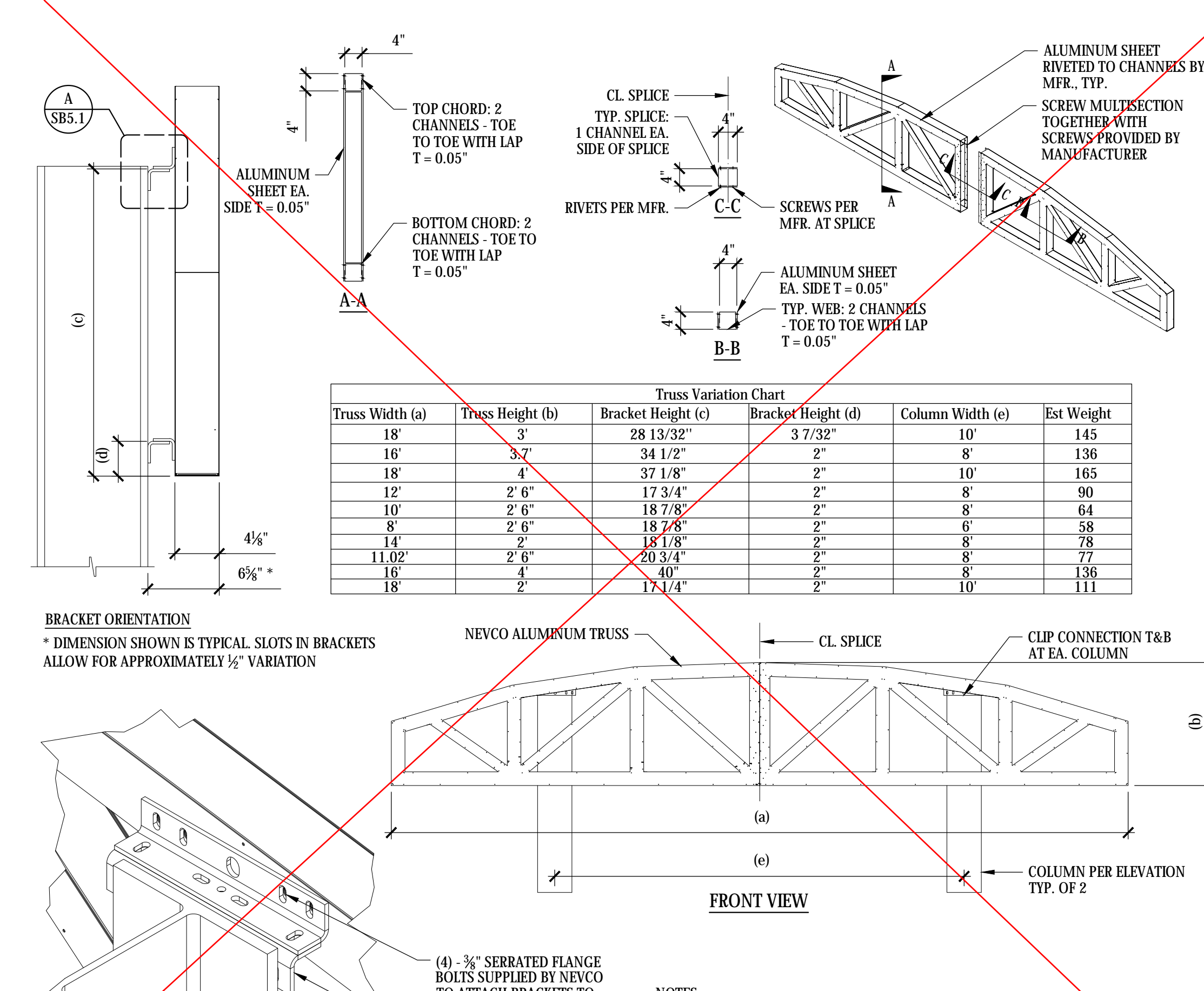
## INSTALLATION OF ALUMINUM TRUSS ON FOUR COL. w/ LATERAL



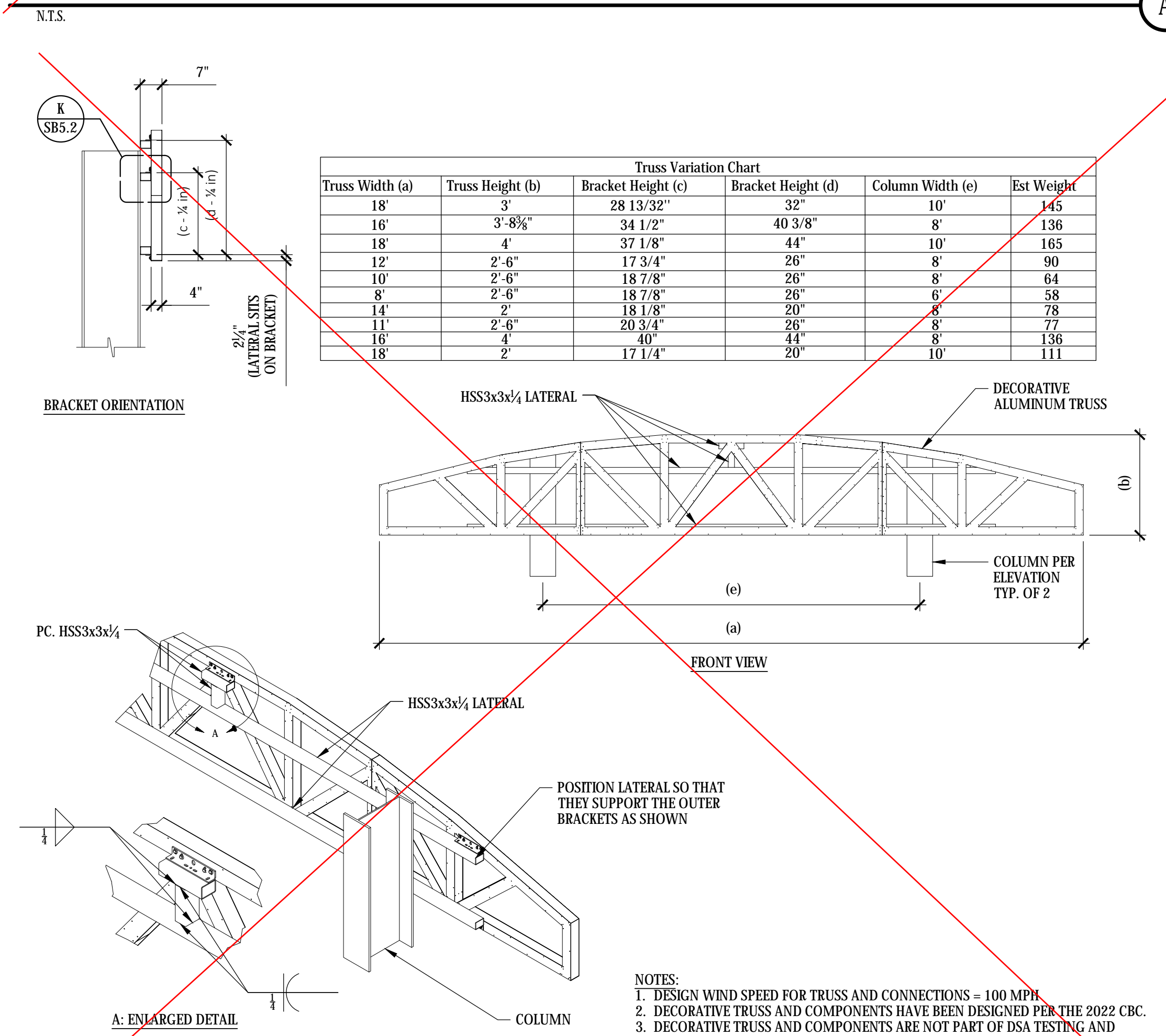
## INSTALLATION OF ALUMINUM TRUSS ON THREE COLUMNS



## INSTALLATION OF ALUMINUM TRUSS ON THREE COL. w/ LATERAL



## INSTALLATION OF ALUMINUM TRUSS ON TWO COLUMNS



## INSTALLATION OF ALUMINUM TRUSS ON TWO COL. w/ LATERAL

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DATE: 01/24/2024

**SSG**  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
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STATE OF CALIFORNIA  
08.09.2023

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EXPIRES 09/20/2024  
STATE OF CALIFORNIA

DECORATIVE ALUMINUM  
TRUSS ATTACHMENT  
DETAILS

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

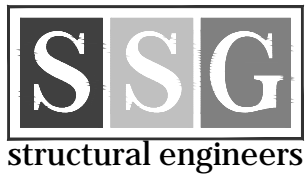
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SSG JOB # S23109

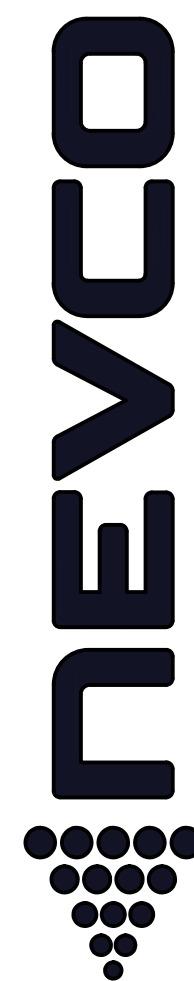
SHEET SB5.3



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APP: 02-121895 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024



PC SEOR REAL  
DATE SIGNED: 08.09.2023  
THESE DRAWINGS, NOTES AND DETAILS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF SSG STRUCTURAL ENGINEERS, LLP. ALL DRAWINGS, INFORMATION, SPECIFICATIONS, REAS, JOCKING AND ARRANGEMENTS REPRESENTED WITHIN THESE DOCUMENTS SHALL REMAIN THE PROPERTY OF THE ENGINEER. NO PART THEREOF SHALL BE COPIED, REPRODUCED, OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED AND DELIVERED WITHOUT THE EXPRESS WRITTEN CONSENT OF THE ENGINEER, CONTRACT 2023. THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS.



301 East Harris Avenue, Greenville, Illinois 62246  
Phone: (618) 664-0960  
www.nevco.com

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122347 PC  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☒ CG ☐  
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application  
for construction is required.



## ALUMINUM TRUSS ATTACHMENT DETAILS & 10mm VIDEO DISPLAY SUPPORT

SHEET INFORMATION

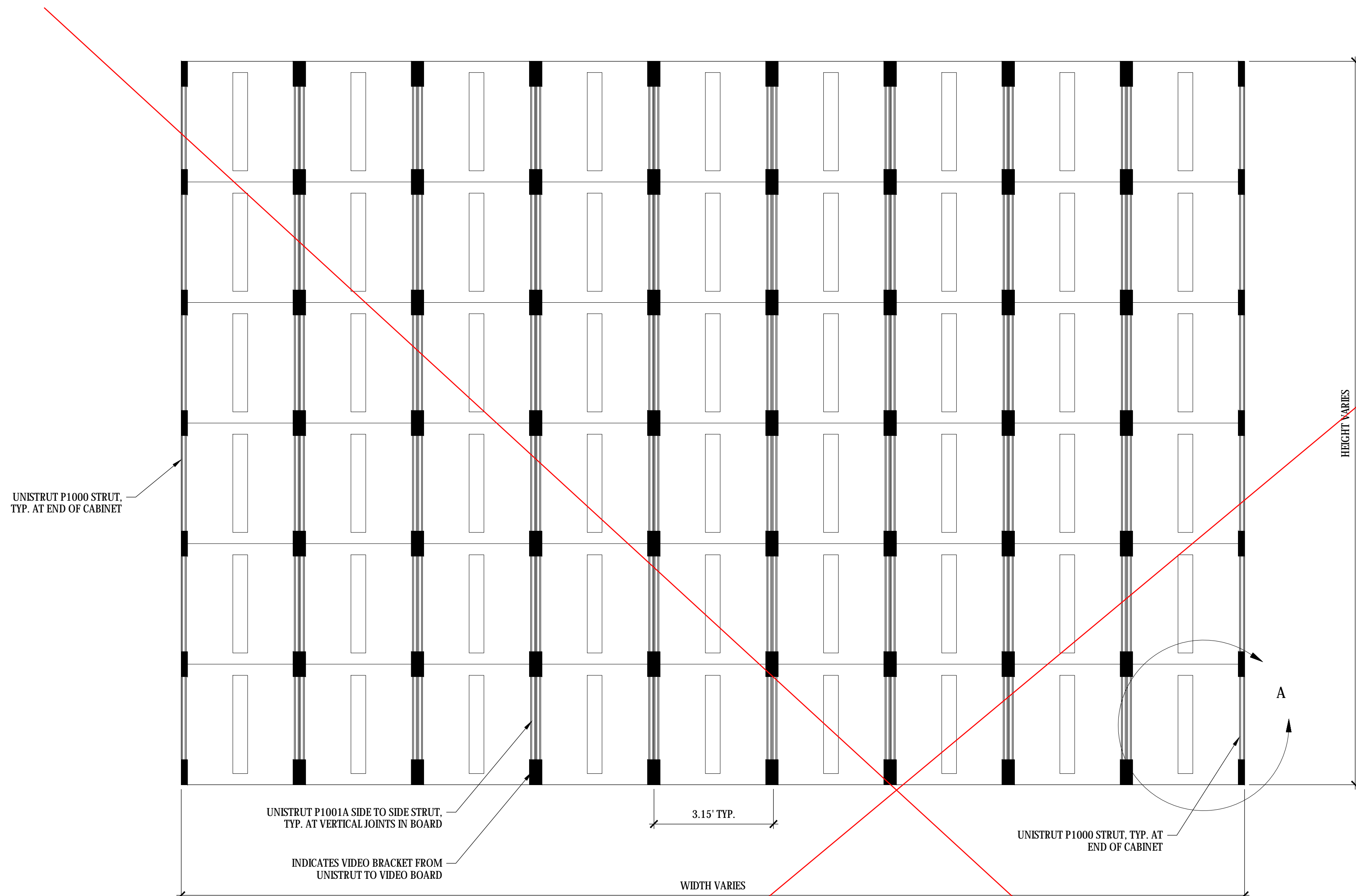
DATE 08.09.2023

DRAWN JMK

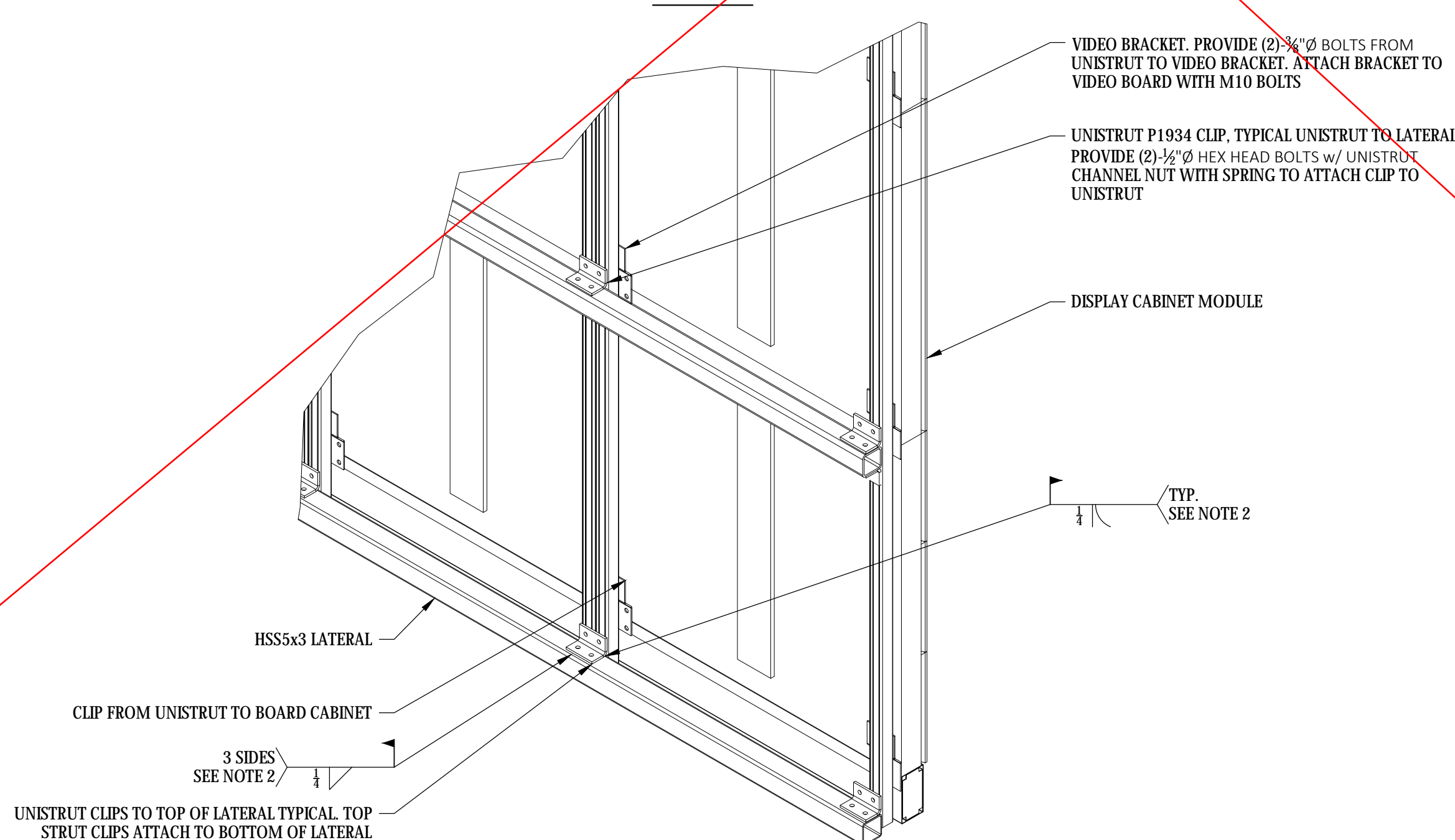
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SSG JOB # S23109

SHEET SB5.4



ELEVATION

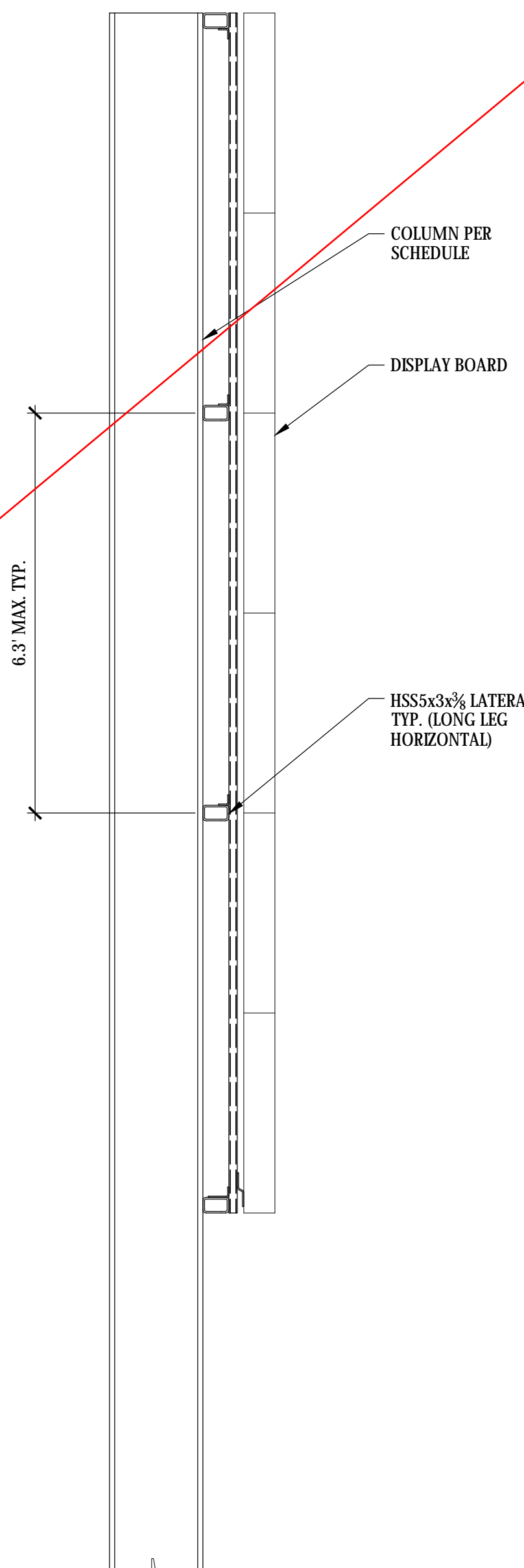


DETAIL A

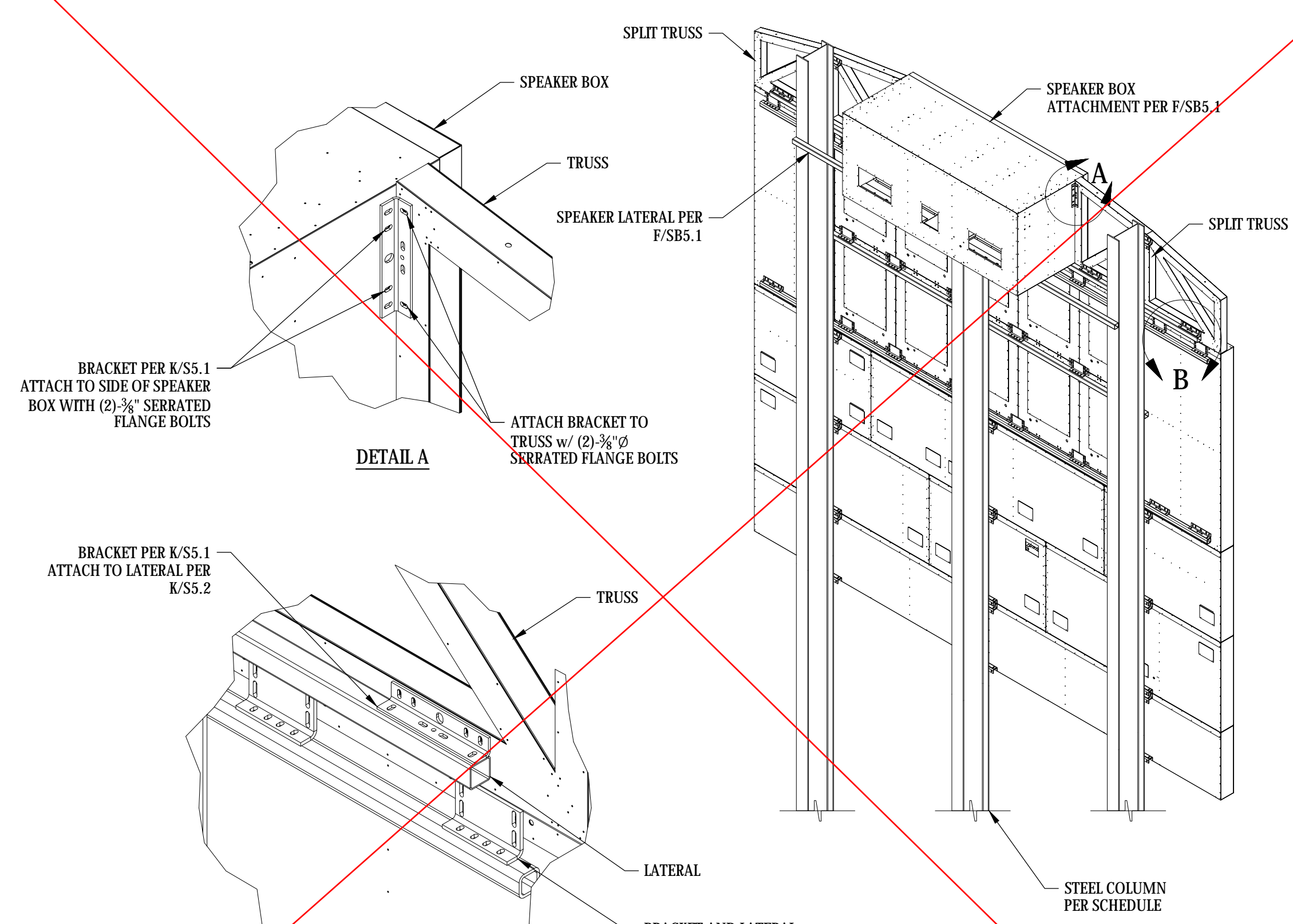
NOTES: (P)  
1. DESIGN OF SUPPORTS AND CONNECTIONS IS BASED ON A VIDEO DISPLAY WITH A WEIGHT OF 8 LB/SQ. FT. TO DETERMINE TOTAL VIDEO DISPLAY WEIGHT CONTACT NEVCO SCOREBOARDS  
2. ALTERNATE OPTION TO BOLT CLIP TO LATERAL WITH 1/2" DIAMETER THROUGH BOLTS IN LIEU OF WELD

## 10mm VIDEO DISPLAY SUPPORT

N.T.S.



SECTION



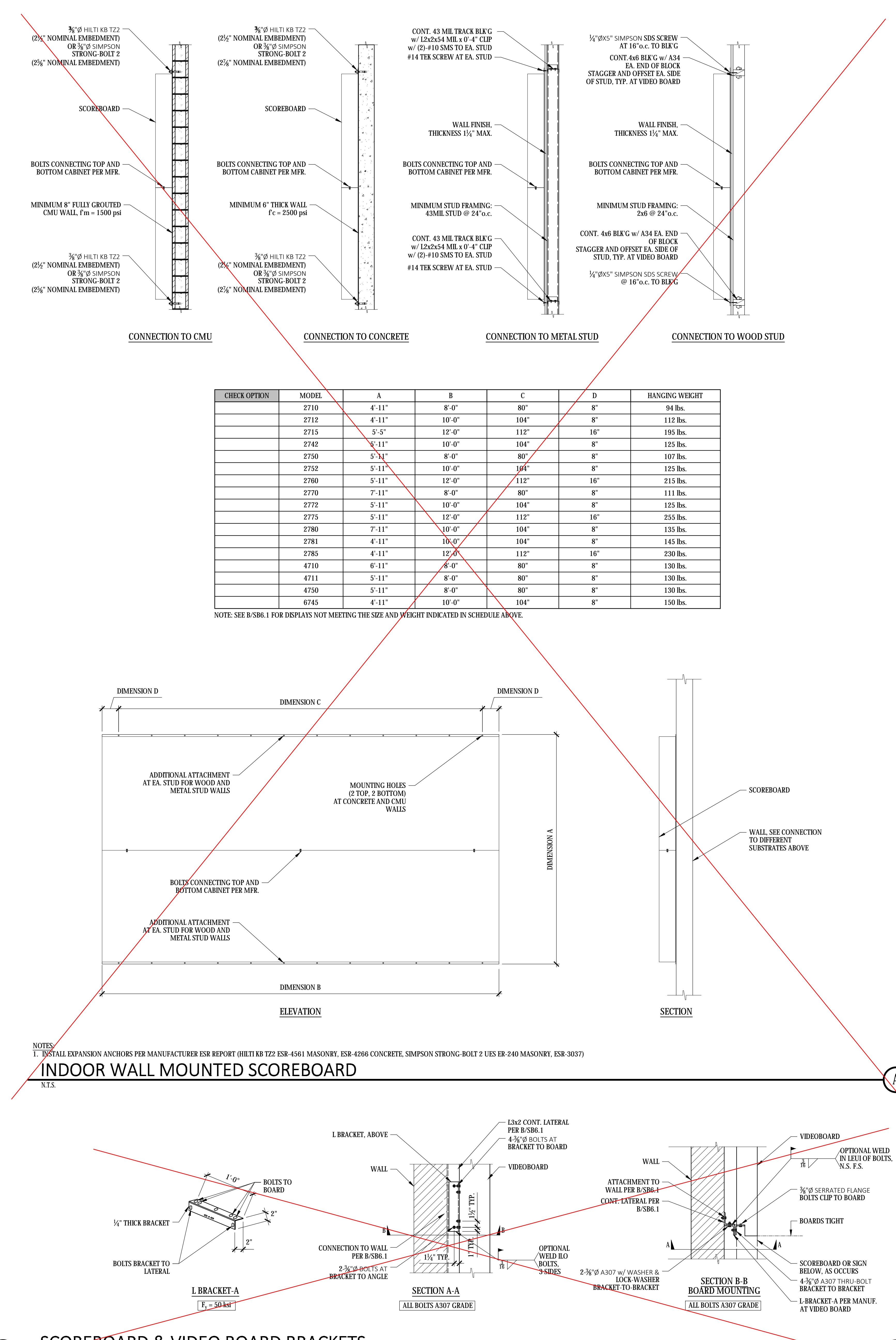
DETAIL B

NOTES:  
1. DETAIL IS APPLICABLE TO 3 COLUMN AND 4 COLUMN ASSEMBLIES

## SPLIT DECORATIVE TRUSS FLANKING SPEAKER BOX

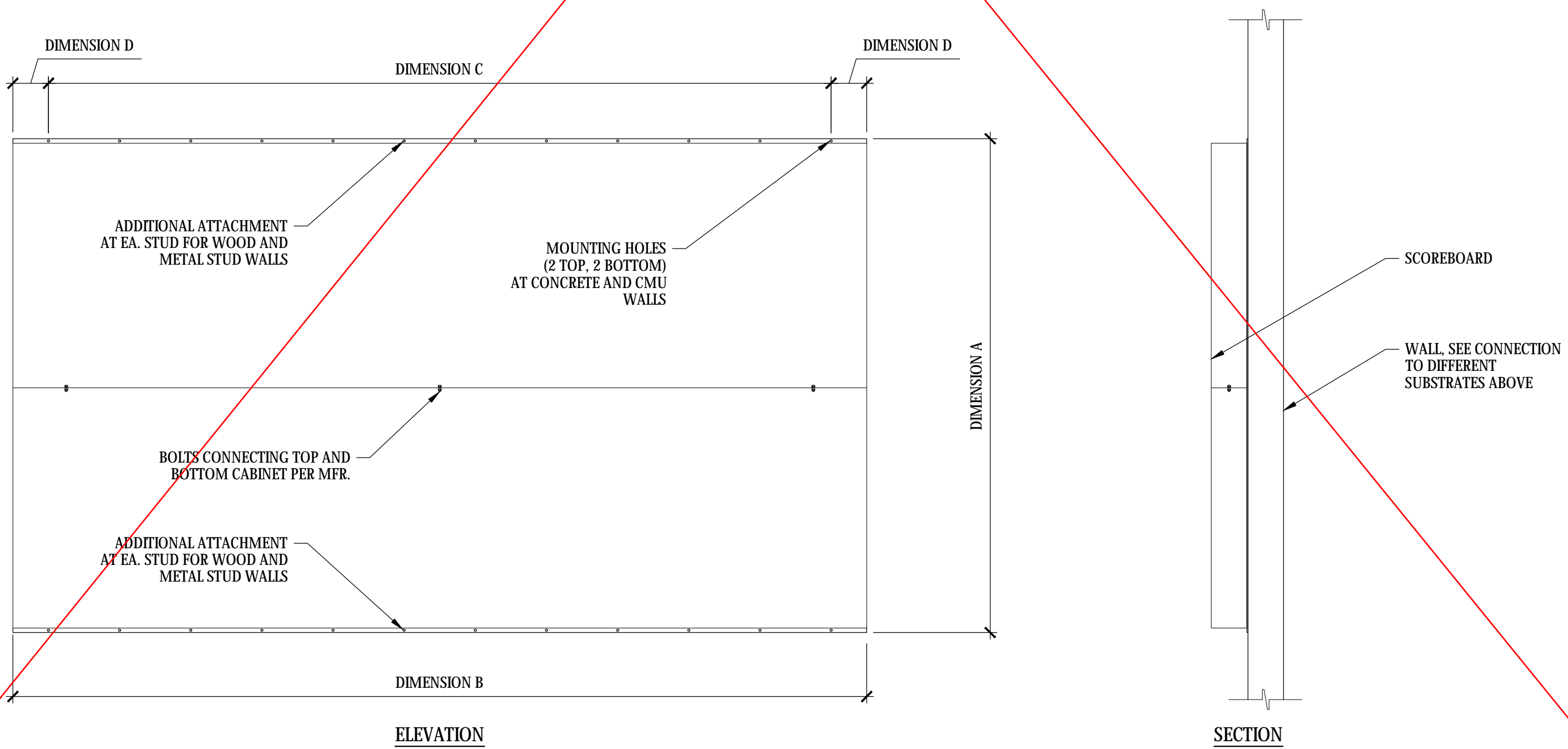
N.T.S.





CHECK OPTION	MODEL	A	B	C	D	HANGING WEIGHT
	2710	4'-11"	8'-0"	80"	8"	94 lbs.
	2712	4'-11"	10'-0"	104"	8"	112 lbs.
	2715	5'-5"	12'-0"	112"	16"	195 lbs.
	2742	5'-11"	10'-0"	104"	8"	125 lbs.
	2750	5'-11"	8'-0"	80"	8"	107 lbs.
	2752	5'-11"	10'-0"	104"	8"	125 lbs.
	2760	5'-11"	12'-0"	112"	16"	215 lbs.
	2770	7'-11"	8'-0"	80"	8"	111 lbs.
	2772	5'-11"	10'-0"	104"	8"	125 lbs.
	2775	5'-11"	12'-0"	112"	16"	255 lbs.
	2780	7'-11"	10'-0"	104"	8"	135 lbs.
	2781	4'-11"	10'-0"	104"	8"	145 lbs.
	2785	4'-11"	12'-0"	112"	16"	230 lbs.
	4710	6'-11"	8'-0"	80"	8"	130 lbs.
	4711	5'-11"	8'-0"	80"	8"	130 lbs.
	4750	5'-11"	8'-0"	80"	8"	130 lbs.
	6745	4'-11"	10'-0"	104"	8"	150 lbs.

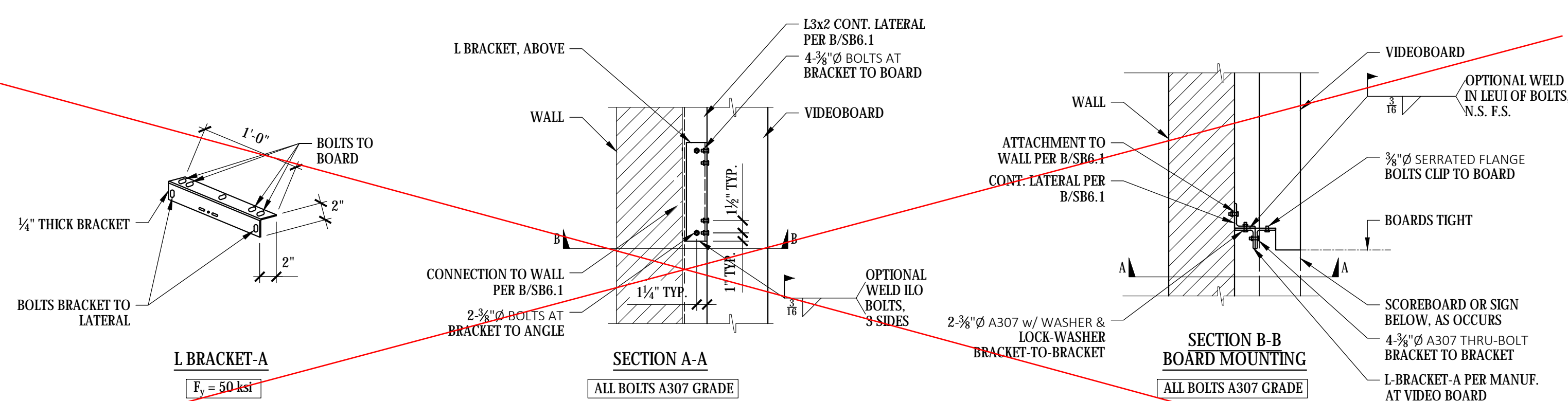
NOTE: SEE B/SB6.1 FOR DISPLAYS NOT MEETING THE SIZE AND WEIGHT INDICATED IN SCHEDULE ABOVE



NOTES:  
1. INSTALL EXPANSION ANCHORS PER MANUFACTURER ESR REPORT (HILTI KB T22 ESR-4561 MASONRY, ESR-4266 CONCRETE, SIMPSON STRONG-BOLT 2 UES ER-240 MASONRY, ESR-3037)




## INDOOR WALL MOUNTED SCOREBOARD

N.T.S



## SCOREBOARD & VIDEO BOARD BRACKETS

NTS

 <b>SSG</b> structural engineers	
 MICHAEL F. PAPPAS No. 5405 STRUCTURAL STATE OF CALIFORNIA EXPIRATION DATE: 08.09.2023	
<small>PC SEOR SEAL</small>	
<p>THESE DRAWINGS, NOTES AND DETAILS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF SSG STRUCTURAL ENGINEERS. IF ALL DRAWINGS, INFORMATION, SPECIFICATIONS, REAS, ISSUES AND ARRANGEMENTS REPRESENTED WITHIN THESE DOCUMENTS SHALL REMAIN THE PROPERTY OF THE ENGINEER. NO PART THEREOF SHALL BE COPIED, ENCHANGED TO OTHERS OR USED IN CONNECTION WITH ANY WORK, OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED AND REVISED WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE ENGINEER. COPYRIGHT 2022.</p> <p style="text-align: center;">THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS</p>	
 <div style="display: flex; justify-content: space-between; align-items: center;"> <div style="text-align: left;"> <p>301 East Harris Avenue, Greenville, Illinois 62246</p> <p>Phone: (618) 664-0360</p> </div> <div style="text-align: right;"> <p><a href="http://www.nevco.com">www.nevco.com</a></p> </div> </div>	
<div style="border: 1px solid black; border-radius: 15px; padding: 10px; width: fit-content; margin: 0 auto;"> <p style="text-align: center;">APPROVED DIV. OF THE STATE ARCHITECT</p> <p style="text-align: center;">APP: 04-122317 PC</p> <p style="text-align: center;">REVIEWED FOR</p> <p style="text-align: center;"> <input checked="" type="checkbox"/> SS             <input checked="" type="checkbox"/> FLS             <input checked="" type="checkbox"/> ACS             <input type="checkbox"/> CS         </p> <p style="text-align: center;">DATE: 09/20/2023</p> </div>	
<small>DSA STAMP</small>	
<p>PRE-CHECK (PC) DOCUMENT CODE: 2022</p> <p>A separate project application for construction is required.</p>	
 R. PAPPAS NO. 52386 EXPIRES 08-24 STATE OF CALIFORNIA	
<h2 style="margin: 0;">INDOOR WALL MOUNTED SCOREBOARD AND VIDEO BOARD ATTACHMENTS</h2>	
<small>SHEET INFORMATION</small>	
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	SB6.1



## ELECTRICAL EQUIPMENT ANCHORAGE NOTES

ALL ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. WHERE NO DETAIL IS INDICATED, THE FOLLOWING COMPONENTS SHALL BE ANCHORED OR BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2022 CBC, SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16, CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING ELECTRICAL UTILITY SERVICE. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 120/220 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.

THE FOLLOWING ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:

- COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVE A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT.
- COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

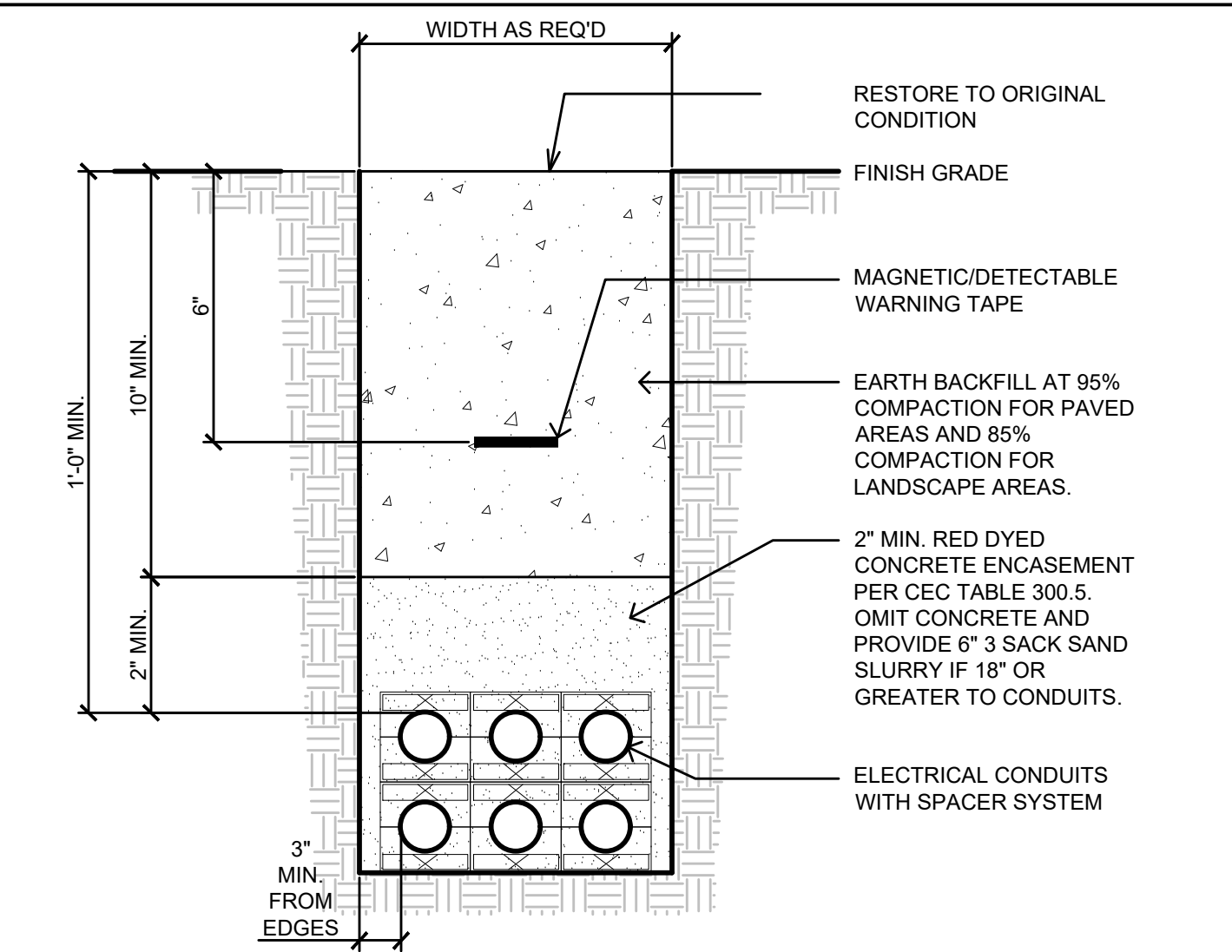
THE ANCHORAGE OF ALL ELECTRICAL COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

## ELECTRICAL DISTRIBUTION BRACING NOTES

THE ELECTRICAL DISTRIBUTION SYSTEM SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16, SECTION 13.3 AS DEFINED IN ASCE 7-16, SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8, AND 2022 CBC, SECTIONS 1617A.1.24, 1617A.1.25, AND 1617A.1.26.

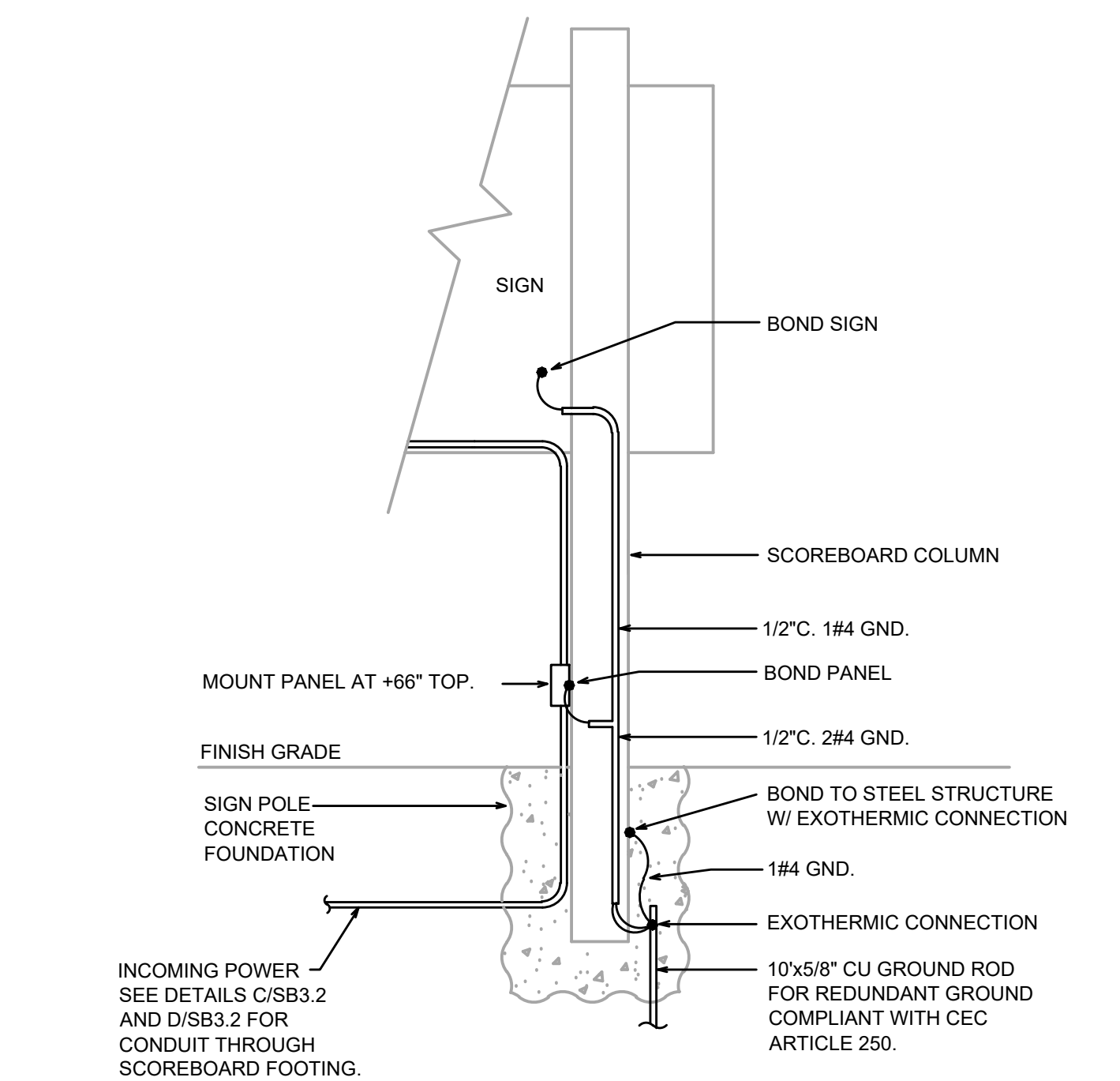
THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

THE ELECTRICAL DISTRIBUTION SYSTEM IS DETAILED ON THE APPROVED DRAWINGS WITH SPECIFIC NOTES AND DETAILS.



5 Trenching Detail

No Scale



4 Scoreboard Grounding Detail

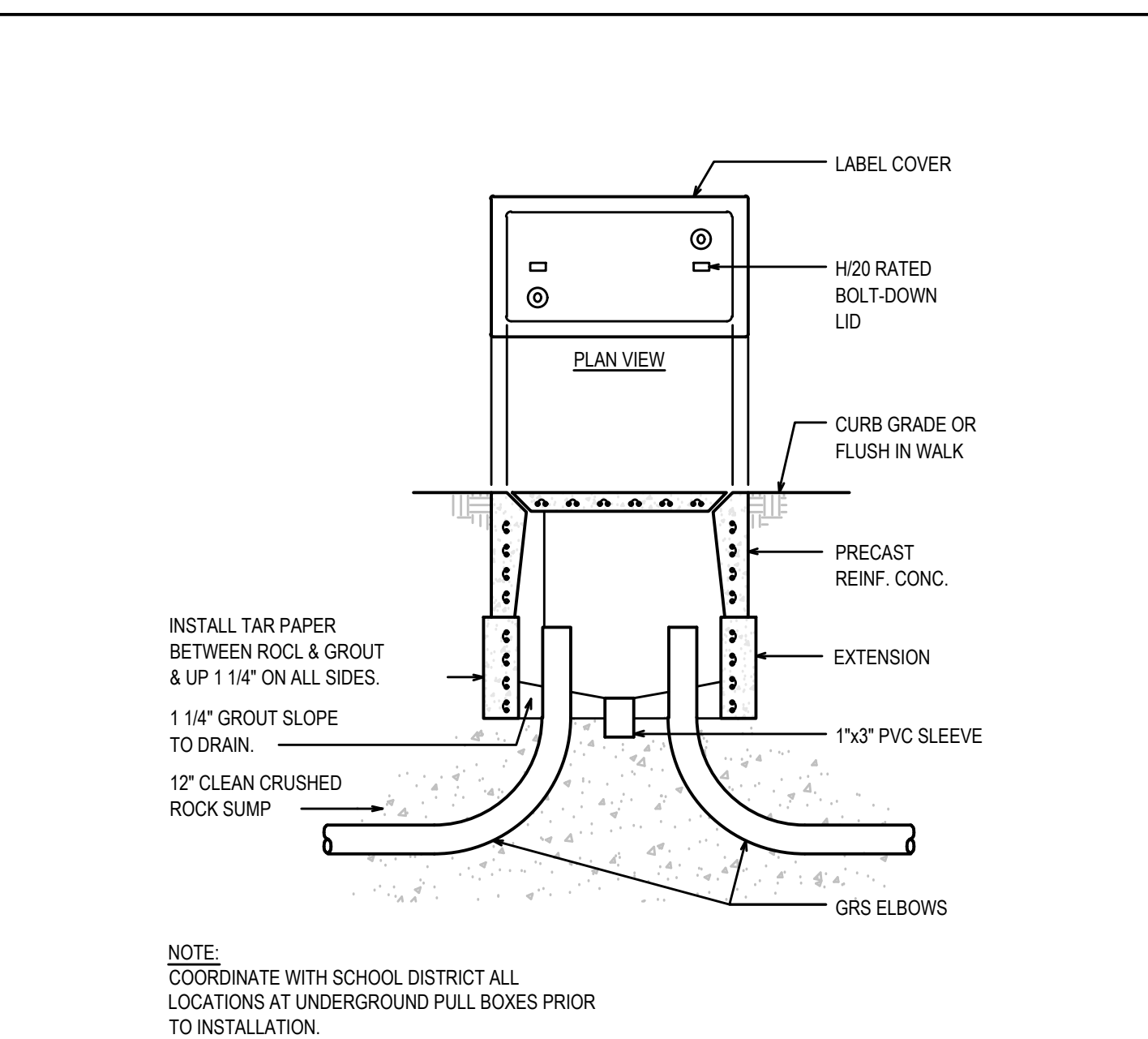
No Scale

## ELECTRICAL GENERAL NOTES

- ALL WORK SHALL MEET THE LATEST ADOPTED ADDITIONS OF THE CALIFORNIA CODE OF REGULATIONS, TITLE 24 AND ALL OTHER APPLICABLE REGULATIONS, WHICH INCLUDE:

CALIFORNIA BUILDING CODE 2022  
CALIFORNIA ELECTRICAL CODE 2022  
NON RESIDENTIAL CEC ENERGY STANDARDS 2022

- NOTHING IN THE PLANS OR SPECIFICATIONS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES.
- IT IS THE INTENTION OF THESE PLANS AND SPECIFICATIONS TO COVER EVERYTHING REQUIRED TO PROVIDE FOR COMPLETE AND OPERATIVE SYSTEMS. THE CONTRACTOR IS TO FURNISH LABOR, MATERIAL, TRANSPORTATION, EQUIPMENT, MISCELLANEOUS SERVICES, ETC. REQUIRED TO ACCOMPLISH THIS RESULT. ANYTHING WHICH MAY BE REASONABLY CONSTRUED AS A NECESSARY PART OF THE INSTALLATION IS TO BE INCLUDED, WHETHER OR NOT SPECIFICALLY SHOWN OR MENTIONED.
- THE CONTRACTOR SHALL EXAMINE THE SITE AND EXISTING CONDITIONS AND MAKE ALLOWANCES IN THE BID FOR ANY CONDITIONS NOT SHOWN ON THE ELECTRICAL DOCUMENTS.
- THE PLANS AND SPECIFICATIONS ARE INTENDED TO BE USED AS CONSTRUCTION GUIDELINES AND ARE NOT THE TOTAL INSTRUMENT OF CONTRACT DOCUMENTS. IT IS NOT THE INTENTION OF ANY CONSTRUCTION PLANS TO DIVIDE WORK AMONG DIFFERENT TRADES. VERIFY THE SCOPE OF WORK WITH THE ARCHITECT AND THE GENERAL CONTRACTOR.
- ELECTRICAL ROUTING IS DIAGRAMMATIC ONLY. ACTUAL ROUTING & PHYSICAL CONDITIONS MAY VARY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE ACTUAL ROUTING, CONNECTIONS, & PROVISION OF ALL APPURTENANCES NECESSARY FOR A COMPLETE & OPERATING SYSTEM.
- ELECTRICAL EQUIPMENT SHALL HAVE AN APPROVED TESTING LABORATORY LABEL ATTACHED (UL, CSA ETC.) PER CEC 110.2.
- PROVIDE LABELING AND DIRECTORIES FOR ALL SWITCHBOARDS AND PANELBOARDS PER CEC 408.4.
- ELECTRICAL EQUIPMENT SHALL HAVE A SHORT CIRCUIT CURRENT RATING CAPABLE OF WITHSTANDING THE AVAILABLE SHORT CIRCUIT CURRENT PER CEC 110.9.
- PROVIDE MINIMUM 30" WIDE x 78" HIGH x 36" DEEP (42" DEEP) WORK CLEARANCES IN FRONT OF PANELS, SERVICE OR EQUIPMENT RATED AT 120/208V 30 4W [277/480V 30 4W] PER CEC 110.26.
- ELECTRICAL RECEPTACLE OUTLETS ON BRANCH CIRCUIT OF 30 AMPERES OR LESS AND COMMUNICATION SYSTEM RECEPTACLES SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), RECEPTACLES SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.
- CONTROLS OR SWITCHES INTENDED TO BE USED BY THE OCCUPANT OF A ROOM OR AREA TO CONTROL LIGHTING AND RECEPTACLE OUTLETS, APPLIANCES, OR COOLING, HEATING AND VENTILATING EQUIPMENT SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), SWITCHES AND CONTROLS SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.
- ALL WALL AND SURFACE MOUNTED FIXTURES PROTRUDING IN THE PATH OF TRAVEL (POT) OR COMMON PEDESTRIAN WAYS SHALL COMPLY WITH CBC 11B-307.2. OR SHALL BE MOUNTED LESS THAN 27" AFF OR GREATER THAN 80" AFF, OR SHALL BE PROVIDED WITH A BARRIER CONFORMING TO CBC 11B-307.4.
- WIRING FOR 120/208V AND 277/480V SYSTEMS SHALL BE MIN. #12 AWG THHN/THWN-2 COPPER.
- FEEDERS SIZE #4 AND LARGER SHALL BE MEGGER TESTED. TEST RESULTS SHALL BE SUBMITTED TO THE ENGINEER.
- COLORS/FINISHES/MATERIALS FOR ALL ELECTRICAL DEVICES, PLATES, LIGHT FIXTURES, ETC. SHALL BE CHOSEN BY THE ARCHITECT.
- EXISTING EQUIPMENT TO BE REMOVED AND/OR REPLACED SHALL BE DELIVERED TO THE DISTRICT MAINTENANCE DEPARTMENT OR DISPOSED OF, AT THE DISCRETION OF THE DISTRICT.
- CALL USA UNDERGROUND ALERT AND VERIFY WITH DISTRICT THE DESIRED ROUTING AND LOCATIONS OF UNDERGROUND CONDUITS AND STRUCTURES PRIOR TO TRENCHING.
- ALL UNDERGROUND CONDUITS SHALL HAVE MINIMUM 24" COVER. TRENCH PER DETAIL 5/E-1. INSTALL GALVANIZED RIGID STEEL RISERS & ELBOWS WHERE RISERS OCCUR. WRAP GRS BELOW GRADE OR PROVIDE PVC COATED GRS. EXPOSED CONDUIT SHALL BE GRS TO 8'-0", THEN EMT ABOVE AS APPROPRIATE. UNDER NO CIRCUMSTANCE SHALL PVC CONDUIT BE INSTALLED ABOVE GRADE.
- CONDUIT INSTALLED ABOVE GRADE SHALL BE MIN. 3/4" TRADE SIZE. CONDUIT BELOW GRADE SHALL BE MIN. 1" TRADE SIZE.
- INCLUDE ALL WORK REQUIRED TO INVESTIGATE, DEMOLISH, & RECONNECT EXISTING ITEMS.
- ALL LOW VOLTAGE EQUIPMENT SHALL BE DEENERGIZED PRIOR TO DEMO WORK. CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO LIVE EQUIPMENT.

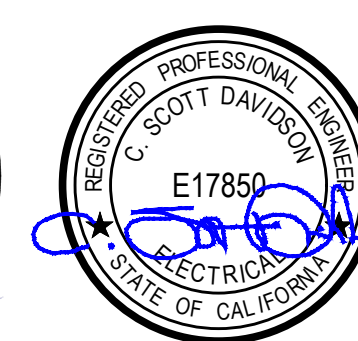


6 Pull Box Detail

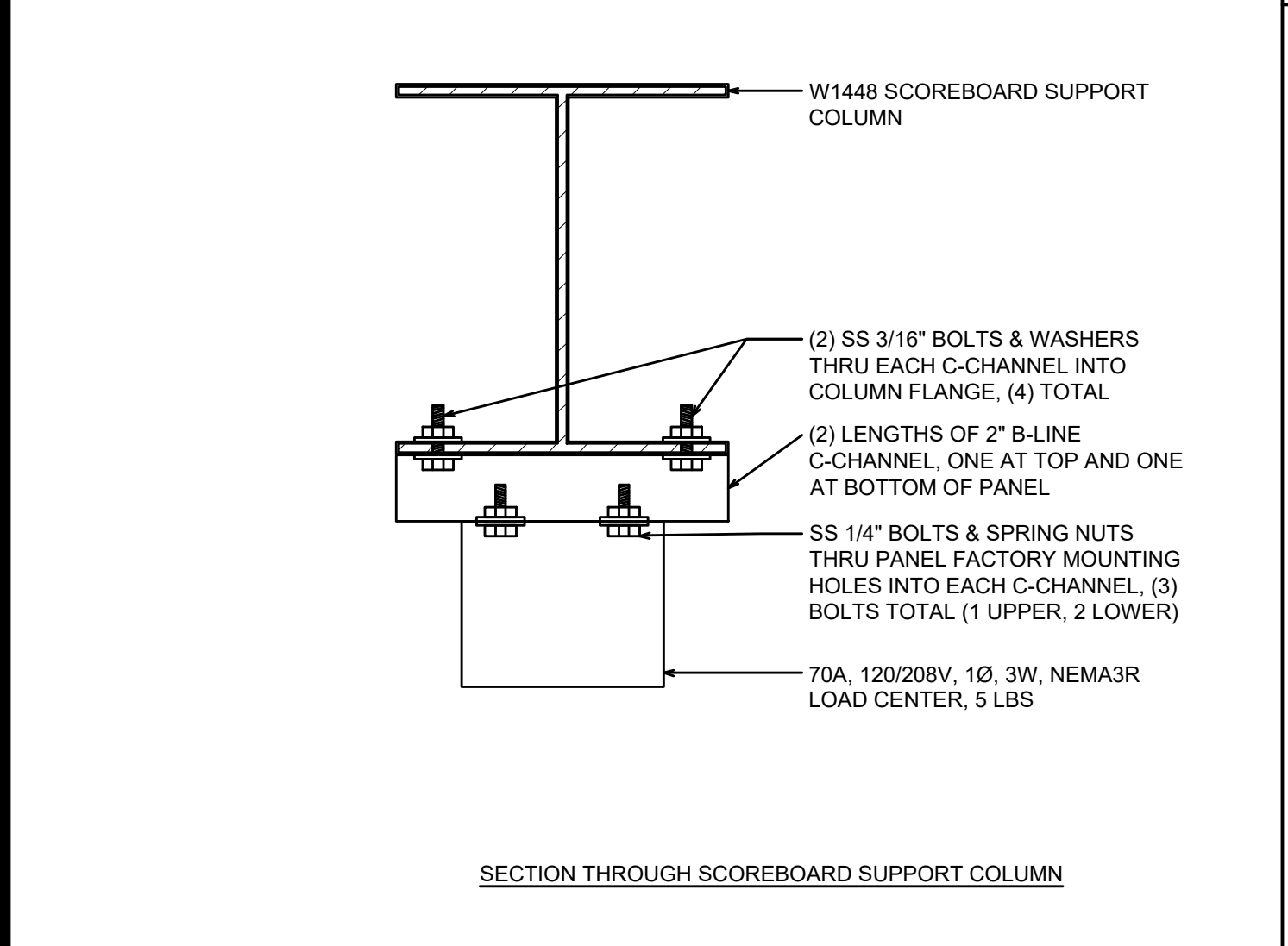
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## ELECTRICAL SYMBOLS

SYMBOL	DESCRIPTION	NOTES
	POLE WITH POST TOP AREA LUMINAIRE	
	POLE WITH AREA LUMINAIRE	
	RECESSED TROFFER LIGHT FIXTURE	
	SURFACE CEILING LIGHT FIXTURE	
	RECESSED DOWN LIGHT	
	WALL LIGHT	
	EXIT SIGN, CEILING	
	EXIT SIGN, WALL	AT +80" AFF
	EMERGENCY LIGHT FIXTURE	PROVIDE UNSWITCHED HOT CONDUCTOR TO BATTERIES
	SWITCHBOARD	REFER TO POWER SINGLE LINE DIAG.
	POWER PANEL	REFER TO PANEL SCHEDULE
	TERMINAL CABINET	
	DISCONNECT SWITCH, FUSIBLE, WP	REFER TO MECH. PLANS & SPECS.
	COMBO STARTER/DISCONNECT SWITCH, WP	REFER TO MECH. PLANS & SPECS.
	JUNCTION BOX	4-11/16" SQUARE BOX & COVER PLATE MIN.
	MOTOR	REFER TO MECH. PLANS AND SPECS.
	DUPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	QUADPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	WP, GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	DATA OUTLET (RJ-45 CAT6) WITH 2 JACKS	HOMERUN CABLES TO IDF.
	(2) WAP DATA JACKS (RJ-45 CAT6A) MOUNTED IN ATTIC SPACE	HOMERUN CABLES TO IDF.
	WALL MOUNT VoIP OUTLET (RJ-45 CAT6)	HOMERUN CABLES TO IDF.
	WALL MOUNT DATA/COMM OUTLET	HOMERUN CABLES TO IDF.
	WIREMOLD 5400 SURFACE WIREWAY	
	CEILING MOUNT PA SPEAKER	MATCH EXISTING SYSTEM COMPONENTS
	WALL MOUNT PA SPEAKER IN SURFACE ENCLOSURE	MATCH EXISTING SYSTEM COMPONENTS
	AUDIO/VISUAL INPUT WITH HDMI/VGA/3.5MM AUDIO/USB JACKS AND WALL PLATE	INSTALL CABLES BETWEEN TEACHER STATION AND PROJECTOR. SEE DETAIL 6/E-2.
	ANALOG CLOCK, BATTERY POWERED	VERIFY COMPATIBILITY WITH EXISTING SYSTEM
	MAIN DISTRIBUTION FRAME (MDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	INTERMEDIATE DISTRIBUTION FRAME (IDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM TERMINAL BLOCK	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM HEAD END	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	TEL. SYSTEM TERMINAL BLOCK	WHERE EXISTING
	TEL. SYSTEM HEAD END	WHERE EXISTING
	FIBER OPTIC SPLICE LOCATION	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	CAT6 PATCH PANEL	WHERE EXISTING
	FIRE ALARM SLC & NAC TERMINAL BLOCKS	LOCATION FOR REFERENCE. SEE F.A. PLANS.
	FIRE ALARM EXPANDER PANEL	SEE F.A. PLANS
	FIRE ALARM CONTROL PANEL	SEE F.A. PLANS
	EMERGENCY VOICE/ALARM COMMUNICATION PANEL	SEE F.A. PLANS
	EXISTING WIRING TO REMAIN	
	WIRING BELOW GRADE	3/4" CONDUIT MIN.
	WIRING IN WALL OR CEILING	3/4" CONDUIT MIN.
	LOW VOLTAGE WIRING	
	CONDUIT RISER	3/4" CONDUIT MIN.
	FLEXIBLE CONDUIT	3/4" CONDUIT MIN.
	CONDUIT STUB AND CAP	3/4" CONDUIT MIN.
	HASH MARKS DENOTES QTY. OF CONDUCTORS	
	WIRE SIZE INDICATED, IF OTHER THAN #12 AWG	
	HOME RUN (TO PANEL 'A', CIRCUIT '15')	3/4" CONDUIT MIN.
	"EXISTING"	
	"UNLESS OTHERWISE NOTED"	
	"WEATHERPROOF" / NEMA 3R	
	"GROUND FAULT INTERRUPTER"	

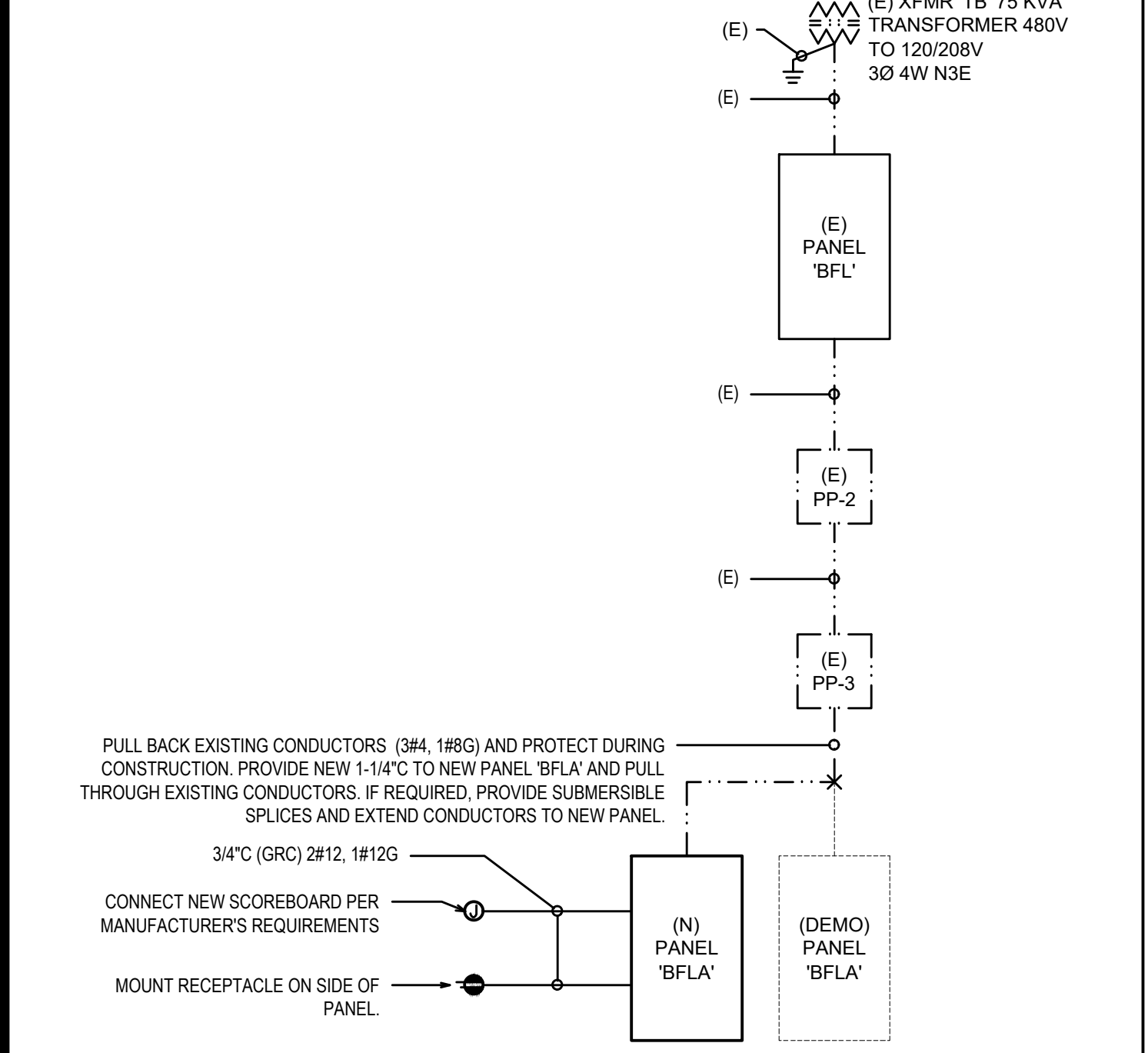


3	Panel Grounding Detail
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2 Panel Mounting Detail

No Scale



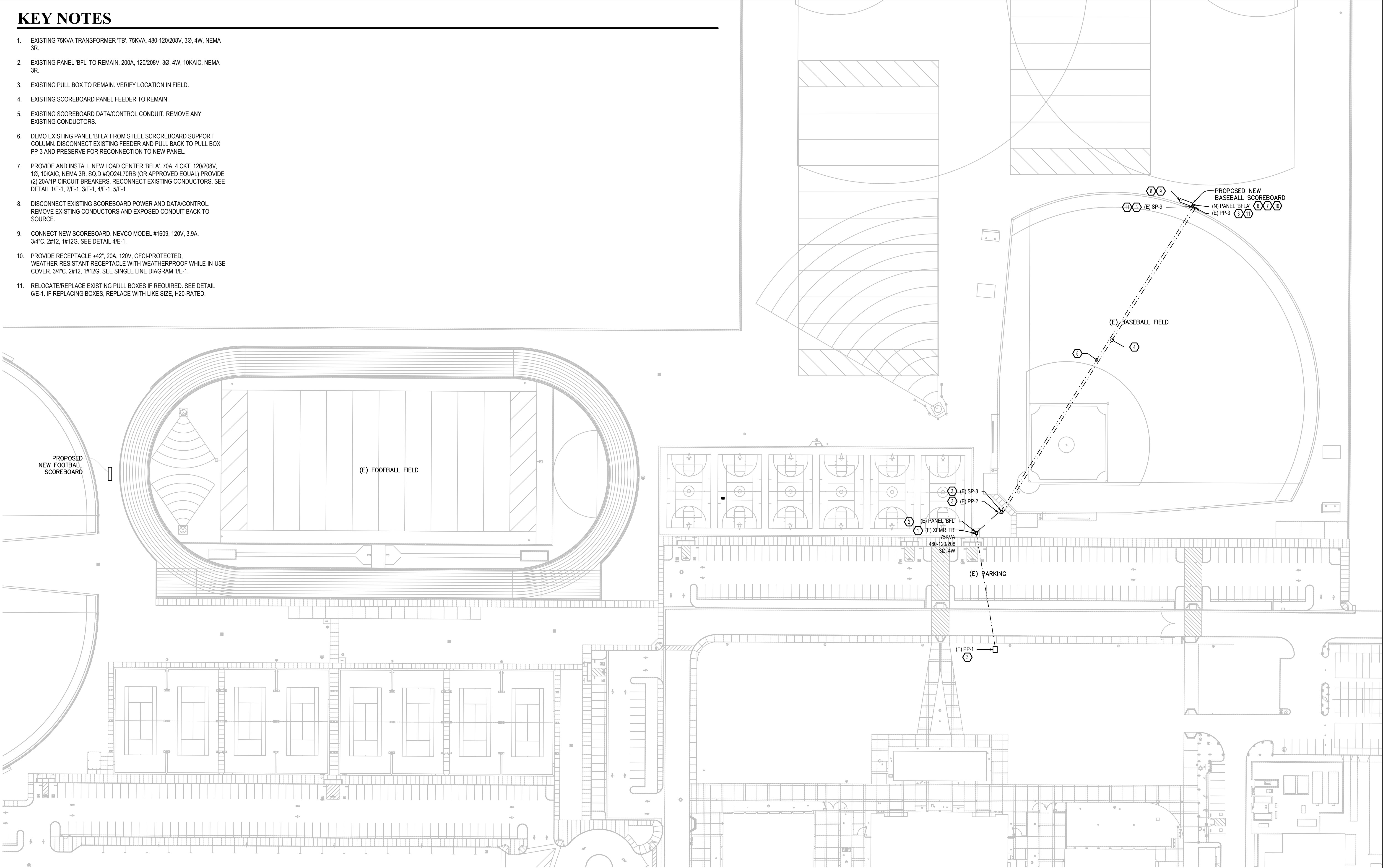
1 Single Line Diagram

No Scale



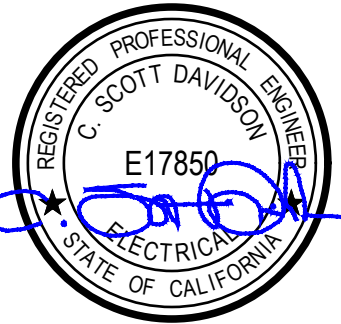
KEY NOTES

- EXISTING 75KVA TRANSFORMER 'TB'. 75KVA, 480-120/208V, 3Ø, 4W, NEMA 3R.
- EXISTING PANEL 'BFL' TO REMAIN. 200A, 120/208V, 3Ø, 4W, 10KAIC, NEMA 3R.
- EXISTING PULL BOX TO REMAIN. VERIFY LOCATION IN FIELD.
- EXISTING SCOREBOARD PANEL FEEDER TO REMAIN.
- EXISTING SCOREBOARD DATA/CONTROL CONDUIT. REMOVE ANY EXISTING CONDUCTORS.
- DEMO EXISTING PANEL 'BFLA' FROM STEEL SCOREBOARD SUPPORT COLUMN. DISCONNECT EXISTING FEEDER AND PULL BACK TO PULL BOX PP-3 AND PRESERVE FOR RECONNECTION TO NEW PANEL.
- PROVIDE AND INSTALL NEW LOAD CENTER 'BFLA'. 70A, 4 CKT, 120/208V, 1Ø, 10KAIC, NEMA 3R. SQ.D #QO24L70RB (OR APPROVED EQUAL) PROVIDE (2) 20A/1P CIRCUIT BREAKERS. RECONNECT EXISTING CONDUCTORS. SEE DETAIL 1/E-1, 2/E-1, 3/E-1, 4/E-1, 5/E-1.
- DISCONNECT EXISTING SCOREBOARD POWER AND DATA/CONTROL. REMOVE EXISTING CONDUCTORS AND EXPOSED CONDUIT BACK TO SOURCE.
- CONNECT NEW SCOREBOARD. NEVCO MODEL #1609, 120V, 3.9A. 3/4"C. 2#12, 1#12G. SEE DETAIL 4/E-1.
- PROVIDE RECEPTACLE +42", 20A, 120V, GFCI-PROTECTED, WEATHER-RESISTANT RECEPTACLE WITH WEATHERPROOF WHILE-IN-USE COVER. 3/4"C. 2#12, 1#12G. SEE SINGLE LINE DIAGRAM 1/E-1.
- RELOCATE/REPLACE EXISTING PULL BOXES IF REQUIRED. SEE DETAIL 6/E-1. IF REPLACING BOXES, REPLACE WITH LIKE SIZE, H20-RATED.



1 PARTIAL ELECTRICAL SITE PLAN

SCALE: 1"= 50'-0"



**Hardin-Davidson**  
Engineering  
356 Pollasky Ave.  
Suite 200  
Clovis, CA 93612  
559.323.4995 tel  
559.323.4928 fax

APPROVALS  
APPLICATION #  
02-121895

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-121895 INC:  
REVIEWED FOR  
SS ☒ FLS ☒ ACS ☐  
DATE: 01/24/2024

DATE: 10/17/2023

MADERA SOUTH HIGH SCHOOL  
BASEBALL SCOREBOARD  
MADERA, CA 93637

REVISIONS  
1  
2  
3  
4  
5

**BrooksRansom**  
ASSOCIATES  
7415 N. PALM AVE. STE 100 | FRESNO, CA 93711  
(559) 449-9444 OFFICE | (559) 449-8404 FAX

SHEET:  
E-2  
PROJECT 23314