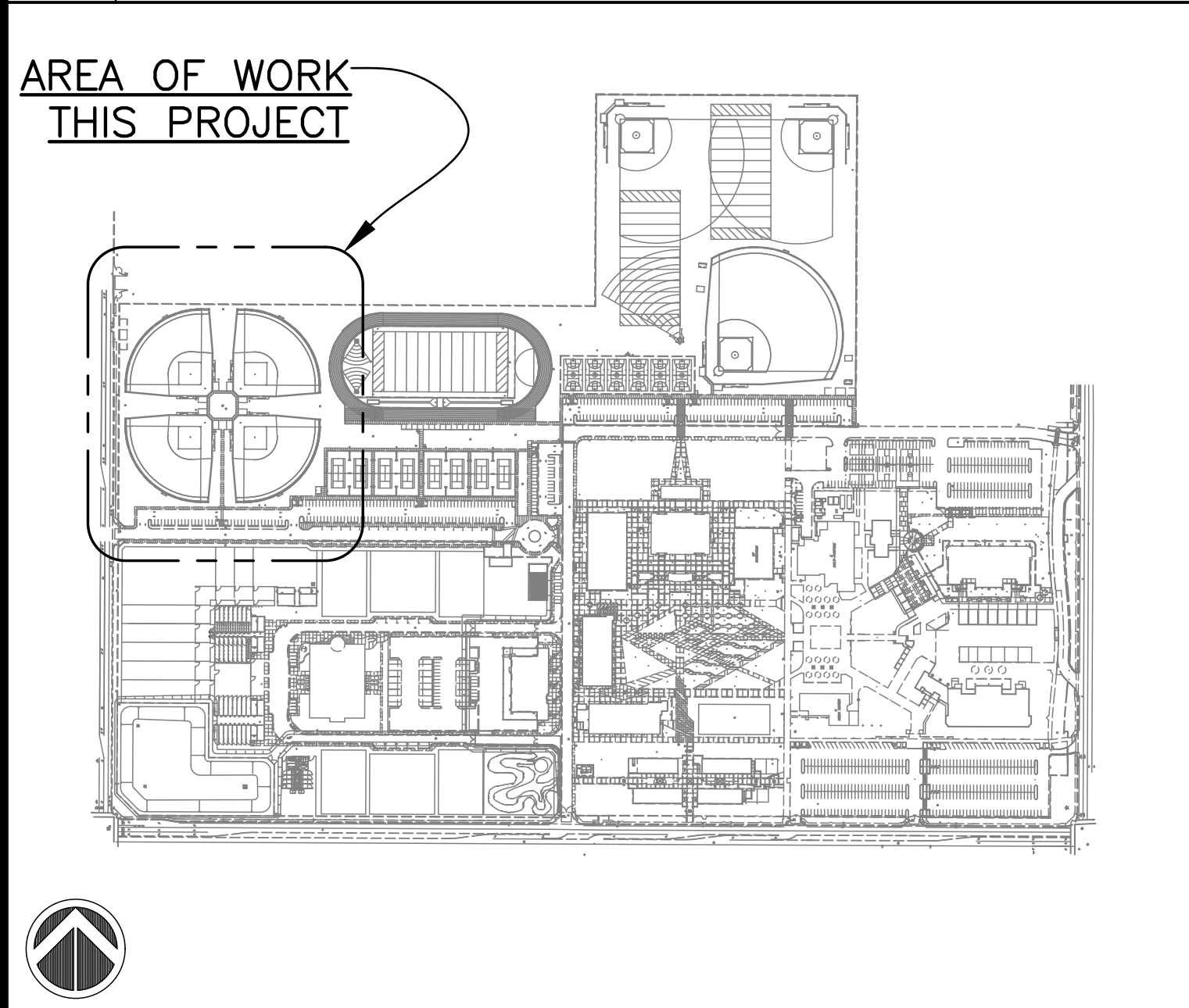
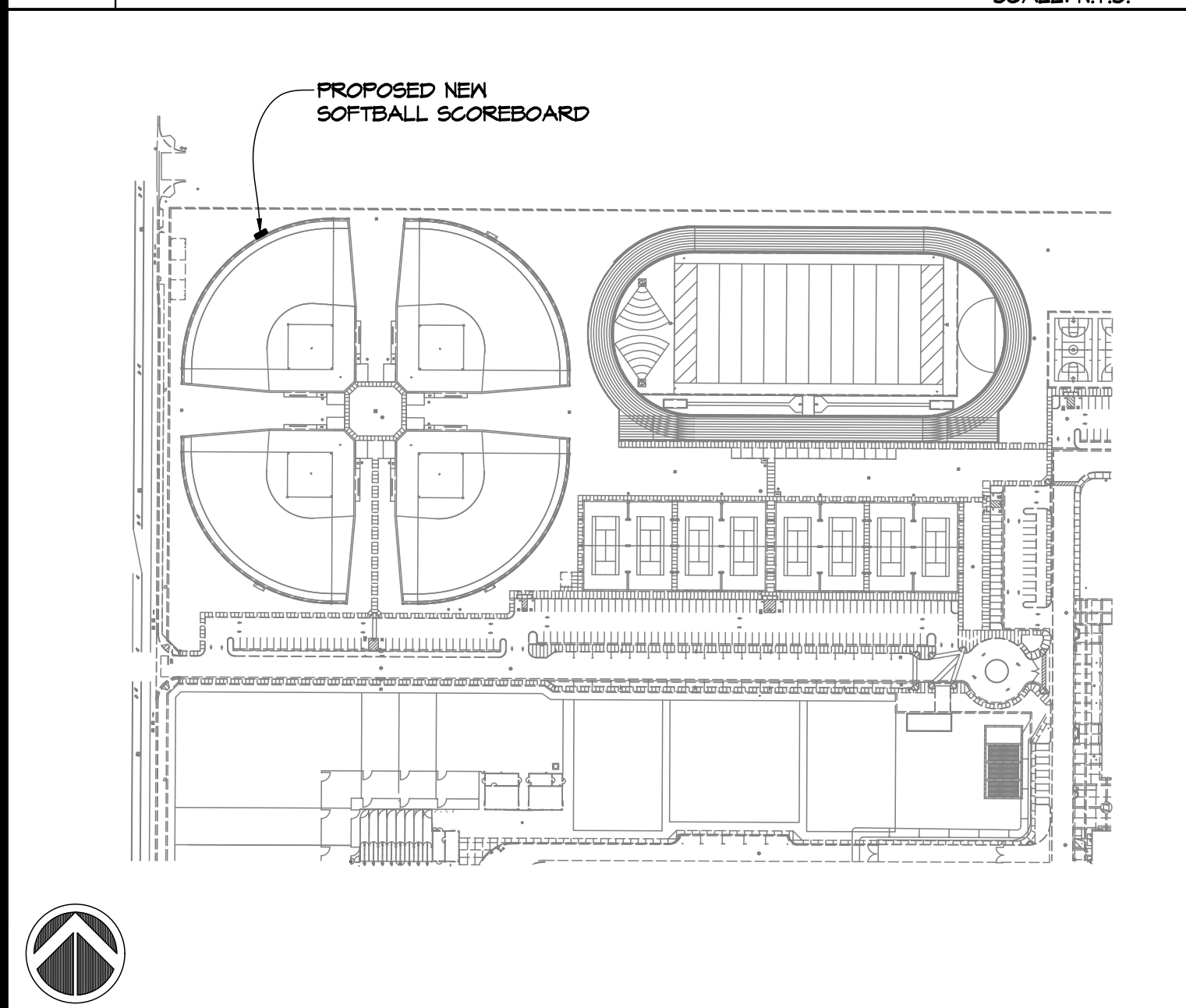


10 VICINITY MAP



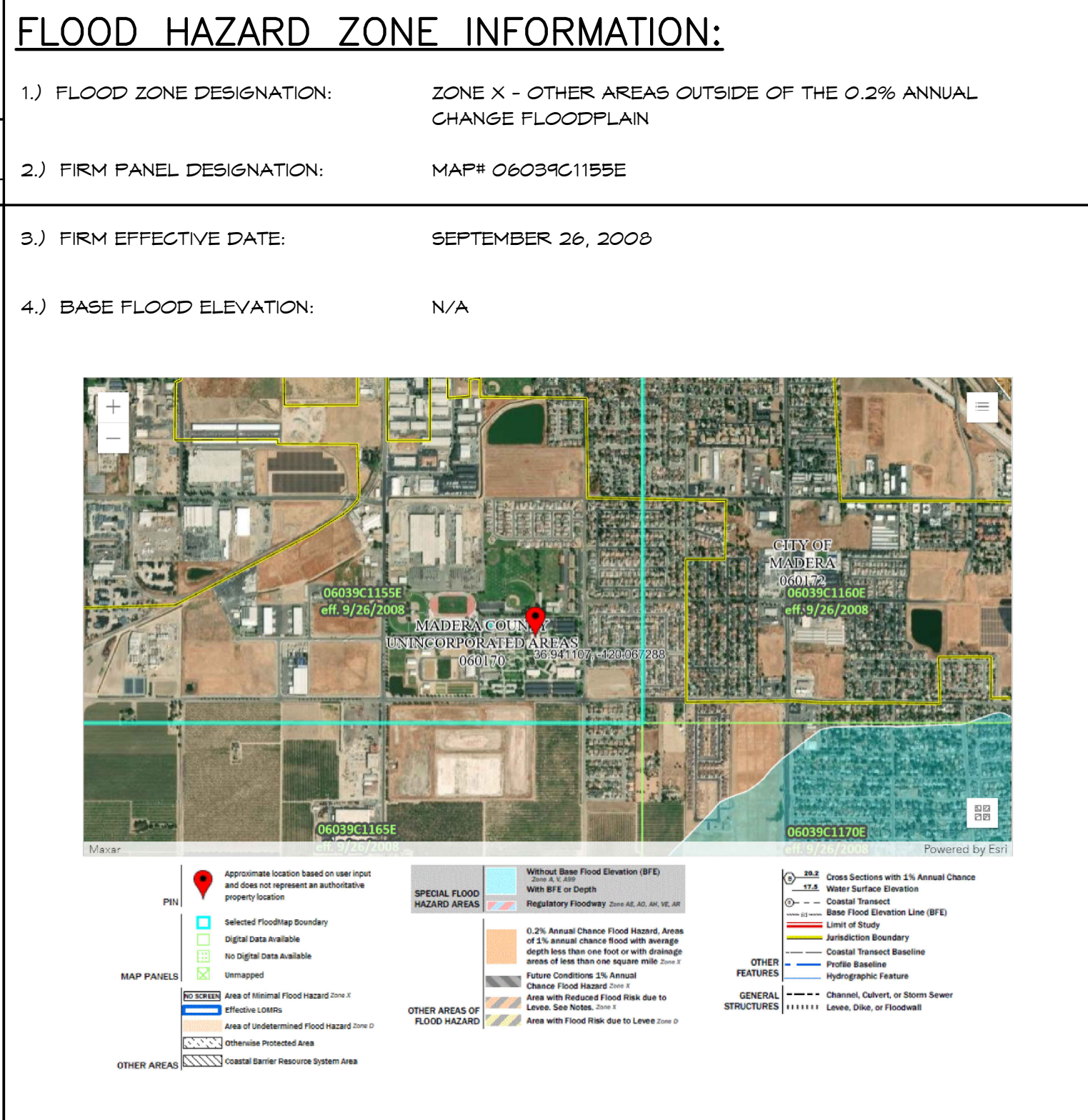
11 SITE PLAN



12 AREA OF WORK

OWNER:	MADERA UNIFIED SCHOOL DISTRICT 1205 S. MADERA AVE. MADERA, CA 93637 T: (559) 675-4540	
STRUCTURAL:	BROOKS RANSOM ASSOCIATES 7415 N. PALM AVE. STE. 100 FRESNO, CA 93711 T: (559) 449-8444 F: (559) 449-8404 CONTACT: ARTURO LOPEZ	
ELECTRICAL:	HARDIN-DAVIDSON ENGINEERING 356 POLLASKY AVE. STE. 200 CLOVIS, CA 93612 T: (559) 523-4995	

9 CONSULTANTS



10 FLOOD ZONE

PROJECT INFORMATION:

PROJECT NAME: MADERA SOUTH HIGH SCHOOL

LOCATION: 105 N. PECAN AVE. MADERA, CALIFORNIA 93637

PROJECT DESCRIPTION: PROVIDE NEW MODEL NEVCO 1609-PC OUTDOOR SOFTBALL SCOREBOARD

SCOPE OF WORK:

- 1.) CONSTRUCT SCOREBOARDS STRUCTURAL SUPPORTS AND FOOTINGS.
- 2.) MAKE ELECTRICAL POWER CONNECTION FROM EXISTING ELECTRICAL PULL BOX, INCLUDING DISCONNECT.
- 3.) PAINT EXPOSED STEEL.

GENERAL NOTES:

- 1.) ALL WORK SHALL BE DONE IN ACCORDANCE WITH THESE CONSTRUCTIONS DRAWINGS, THE CONTRACT SPECIFICATIONS AND, WHERE APPLICABLE, THE CITY OF MADERA AND THE STATE OF CALIFORNIA STANDARD SPECIFICATIONS.
- 2.) THE CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION ACTIVITIES WITH THE SCHOOL DISTRICT'S USE OF THE FACILITIES AND OTHER CONTRACTORS WHO MAY BE DOING CONSTRUCTION WITHIN THE PROJECT SITE.
- 3.) THE CONTRACTORS SHALL CONTACT DISTRICT OFFICIALS FOR DETERMINATION OF DEPTH AND LOCATION OF UNDERGROUND UTILITIES PRIOR TO EXCAVATION IN THE PROJECT SITE.
- 4.) BEFORE COMMENCING WORK, THE CONTRACTOR SHALL NOTIFY ALL UTILITY AUTHORITIES OR UTILITY COMPANIES HAVING POSSIBLE INTEREST IN THE WORK OF THE CONTRACTOR'S INTENTION TO EXCAVATE PROXIMATE TO EXISTING FACILITIES AND THE CONTRACTOR SHALL VERIFY THE LOCATION OF ANY UTILITIES IN THE WORK AREA, NOTIFY U.S.A. AT (800) 642-2444, TWO (2) DAYS PRIOR TO EXCAVATION.
- 5.) CONTRACTOR SHALL PROVIDE 6' HIGH TEMPORARY CHAIN LINK FENCE AROUND THE PERIMETER OF THE WORK AREAS EXCEPT WHERE ENCLOSED BY EXISTING FENCING.
- 6.) ALL CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 CALIFORNIA BUILDING CODE (CBC).
- 7.) CHANGE TO THE APPROVED DRAWINGS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-338, PART 1, TITLE 24, CCR.
- 8.) A "DSA CERTIFIED" CLASS 3 PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- 9.) A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
- 10.) FIRE SAFETY DURING DEMOLITION AND CONSTRUCTION SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF CHAPTER 33 OF THE 2022 CALIFORNIA BUILDING CODE AND THE APPLICABLE PROVISIONS OF CHAPTER 33 OF THE CALIFORNIA FIRE CODE.
- 11.) THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-317(C), PART 1, TITLE 24, CCR)
- 12.) GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.
- 13.) SUBSTITUTIONS AFFECTING DSA REGULATED ITEMS SHALL BE CONSIDERED AS A CONSTRUCTION CHANGE DOCUMENT OR ADDENDUM, AND SHALL BE APPROVED BY DSA PRIOR TO FABRICATION AND INSTALLATION PER DSA IR A-6 AND SECTION 338(C) PART 1, TITLE 24 CCR.

6 GENERAL DESIGN NOTES

SHEET INDEX:	
SHT. NO.	TITLE
GENERAL	
C-1	COVER SHEET
SP-1	STRUCTURAL PARTIAL SITE PLAN
NEVCO DSA P.C. 04-122317	
SB0.1	COVER SHEET
SB0.2	STRUCTURAL NOTES
SB0.3	EXAMPLE DSA 108-TESTING AND INSPECTIONS
SB1.1	MARGUEE CAISSON - EMBEDDED
SB1.2	MARGUEE CAISSON - BOLTED
SB1.3	MARGUEE MAT FOOTING
SB2.1	TWO COLUMN CAISSON - EMBEDDED
SB2.2	TWO COLUMN CAISSON - BOLTED
SB2.3	TWO COLUMN MAT FOOTING
SB3.1	THREE COLUMN CAISSON - EMBEDDED
SB3.2	THREE COLUMN CAISSON - BOLTED
SB3.3	THREE COLUMN MAT FOOTING
SB4.1	FOUR COLUMN CAISSON - EMBEDDED
SB4.2	FOUR COLUMN CAISSON - BOLTED
SB4.3	FOUR COLUMN MAT FOOTING
SB5.1	ATTACHMENT DETAILS
SB5.2	OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAILS
SB5.3	DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS
SB5.4	DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS 4-10mm VIDEO BOARD
SB6.1	INDOOR WALL MOUNTED SCOREBOARD
TOTAL SHEET COUNT: 24	

3 SHEET INDEX

NOT APPLICABLE

4 DEFERRED SUBMITTAL

APPROVALS:
APPLICATION #
02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122089 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024

DATE: 12/12/2023

COVER SHEET
MADERA SOUTH HIGH SCHOOL
SOFTBALL SCOREBOARD
MADERA, CA 93637

REVISIONS	

Brooks Ransom
ASSOCIATES
7415 N. PALM AVE. STE. 100 FRESNO, CA 93711
(559) 449-8444 OFFICE | (559) 449-8404 FAX



SHEET: C-1

PROJECT 23314.01

PARTIAL SITE PLAN
MADERA SOUTH HIGH SCHOOL
SOFTBALL SCOREBOARD
MADERA, CA 93637

REVISIONS

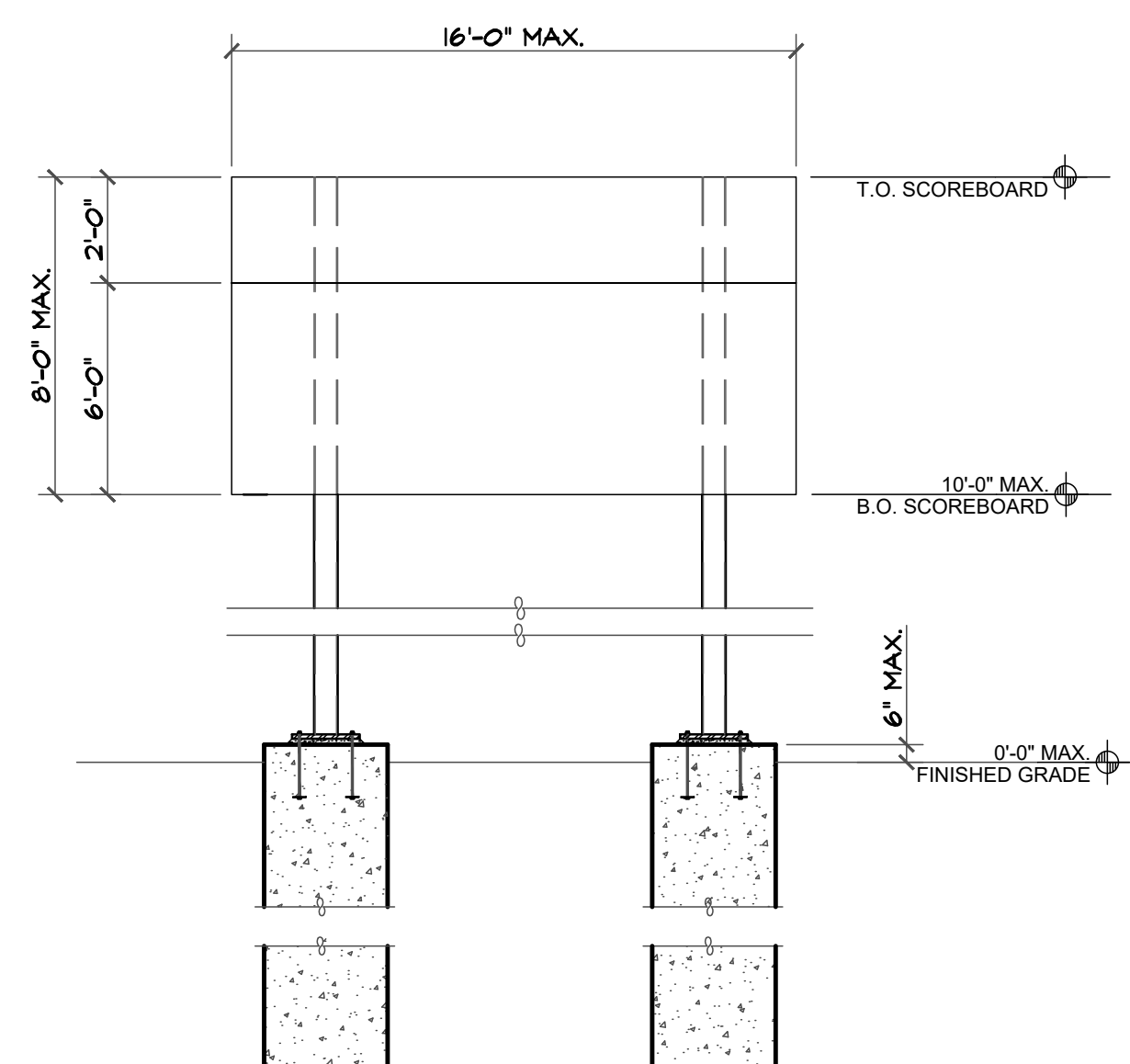
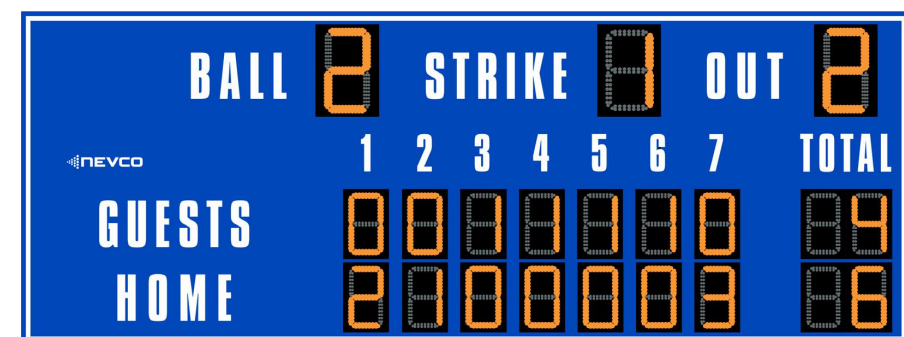


SHEET:

SP-1

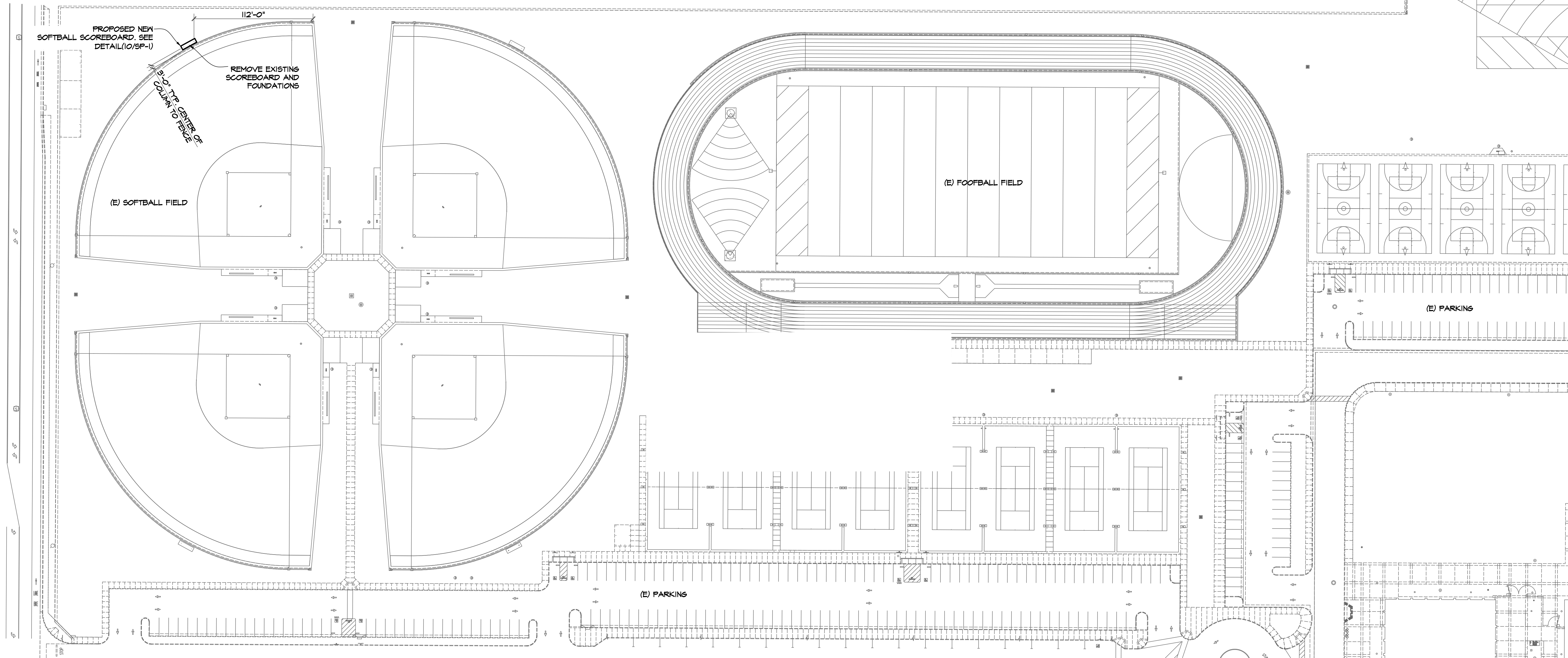
PROJECT 23314.01

SEE DSA P.C. 04-122317 FOR NEVCO 1609-PC SCOREBOARD

ELEVATION

SCALE: N.T.S

10	DETAIL
----	--------




12	PARTIAL SITE PLAN
----	-------------------

SCALE: 1" = 50'-0"



DSA P.C. 04-122317



SSG
structural engineers



NEVCO

301 East Harris Avenue, Greenville, Illinois 62246
Phone: (618) 664-0380

www.nevco.com

DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application for construction is required.



08.09.2023

JMK

SSG JOB #

SHEET **CD0 1**

SB0.1

TABLE C - SITE SPECIFIC SEISMIC AND WIND VALUES				TABLE B - STRUCTURAL DESIGN VALUES			
EARTHQUAKE DESIGN DATA		MAXIMUM		SITE SPECIFIC		All values reported are unfactored and strength level, unless noted otherwise	
		S= 3.73 g	≥	S= 0.71 g		Value	
Mapped Spectral Response Accelerations (Maximum)		S= 1.0 g	≥	S= 0.24 g		PER SCHEDULE	
Site Class		D		D		30 psf	
Spectral Response Coefficients (Maximum)		S _m = 2.49 g	≥	S _m = 0.67 g		H/240	
		S _m = 1.0 g	≥	S _m = 0.42 g		Value	
Wind Design Data		Value		Value		100 mph	
Design Wind Speed (3-sec gust), V _{EIT}		100 mph		≥ 94 mph		77 mph	
Exposure Category		C		C		II	
TABLE D - SITE FLOOD ZONE THIS SECTION NOT REQUIRED IF SITE IS IN FLOOD ZONE X				Exposure Category		C	
				Geotechnical Engineer:		± 0.18	
Letter Dated:				Design Wind Pressure(s) for Components & Cladding (Not specifically designed by the Registered Design Professional, and to be modified by applicable factors per ASCE 7)		q=21.8k, psf k/VARES	
				Earthquake Design Data		Value	
				Risk Category		II	
				Importance Factor, I		I 0	
				Mapped Spectral Response Accelerations (Maximum)		S= 3.73 g S= 1.0 g	
				Site Class		A through E	
				Spectral Response Coefficients (Maximum)		S _m = 2.49 g S _m = 1.0 g	
				Seismic Design Category		E	
				Analysis Procedure Used		Equivalent Lateral Force Procedure (ASCE 7, 12.8)	
				Basic Seismic Force Resisting System		Non-Building Structure, ASCE 7-16 Chapter 15	
				Response Modification Factor, Signs and Billboards Table 15.4-2		R= 3.0	
				Seismic Response Coefficient		C= 0.83	
				Design Base Shear		V= C _s w _p	
				Flood Design			
				When the scoreboard is located in a flood zone other than Zone X, a letter stamped and signed from a Geotechnical Engineer is needed to validate allowable soil values specified in the PC are still applicable.			
				Geotechnical Design Data		Value	
				Geotechnical Design Based on: 2022 California Building Code, Chapter 18A, Table 1806.A.2 (Class 5 Material)			
				Allowable Soil Bearing Pressure (DL + LL)		1,500 psf	
				Design Passive Pressure, P _p (Tabular value has been increased per CBC Section 1806A.3.4 for pier design)		100 pcf	
				Design Skin Friction, f _s		100 psf	

10. STEEL COATING SPECIFICATIONS FOR WEATHER PROTECTION IF DIFFERENT THAN NOTED ON SB0.3

TABLE A - SCOREBOARD ASSEMBLY WORKSHEET ⁽¹⁾

SCOREBOARD ASSEMBLY FOOTNOTES

2. See Step 3 of Scoreboard Assembly Worksheet Instructions

☐ SB6.1 INDOOR WALL MOUNTED SCOREBOARD

2022 CALIFORNIA BUILDING STANDARDS CODE (TITLE 24, CCR):

2022 ADMINISTRATIVE CODE, PART 1, TITLE 24 CODE OF REGULATIONS (CCR)
2022 CALIFORNIA BUILDING CODE VOLUMES 1 & 2, PART 2, TITLE 24 CCR
2022 CALIFORNIA ELECTRICAL CODE, PART 3, TITLE 24 CCR
2022 CALIFORNIA MECHANICAL CODE, PART 4, TITLE 24 CCR
2022 CALIFORNIA PLUMBING CODE, PART 5, TITLE 24 CCR
2022 CALIFORNIA ENERGY CODE, PART 6, TITLE 24 CCR
2022 CALIFORNIA FIRE CODE, PART 9, TITLE 24 CCR
2022 CALIFORNIA GREEN BUILDING STANDARDS CODE, PART 11, TITLE 24 CCR
2022 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24 CCR

REFERENCED CODE SECTIONS FOR APPLICABLE STANDARDS:
2022 CALIFORNIA BUILDING CODE, CHAPTER 35
2022 CALIFORNIA FIRE CODE, CHAPTER 80

GENERAL REQUIREMENTS

1. THE ARCHITECT OR PROFESSIONAL ENGINEER IN GENERAL RESPONSIBLE CHARGE SHALL SIGN AND SEAL ALL DRAWINGS AND SPECIFICATIONS PER TITLE 24, PART 1, SECTIONS 4-316(E) AND 4-317 (H).
2. CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA, OR CONSTRUCTION CHANGE DOCUMENTS APPROVED BY THE DIVISION OF THE STATE ARCHITECT (DSA), AS REQUIRED BY TITLE 24, PART 1, SECTION 4-338.
3. THE DISTRICT SHALL EMPLOY A CLASS 2 PROJECT INSPECTOR WHEN OVERALL STRUCTURE HEIGHT IS 35 FEET OR GREATER, OTHERWISE A CLASS 3 PROJECT INSPECTOR MAY BE USED. THE PROJECT INSPECTOR SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK , AND SHALL SUBMIT VERIFIED REPORTS ON A DSA-6 FORM. THE DUTIES OF THE PROJECT INSPECTION ARE DEFINED IN TITLE 24, PART 1, SECTION 4-342.
4. ALL SCOREBOARD CONTROLS SHALL BE FULLY ACCESSIBLE VIA WIRELESS CONTROL OR COMPLETE DESIGN SHALL BE DEMONSTRATED IN THE SITE-SPECIFIC APPLICATION.
5. ALL ASSEMBLIES SHALL HAVE ELECTRICAL DISCONNECT PER CEC 600.6 AND BE ELECTRICALLY GROUNDED PER CEC 600.7. SEE DETAIL B/SB5.1
6. IN FLOOD ZONES, LOCATION OF ELECTRICAL ELEMENTS SHALL CONFORM TO ASCE 24, SECTION 7.2 PER DSA PR-14-01 SECTION 1.2.1.
7. SEE PAGE, SB0.2, FOR ALL MATERIAL SPECIFICATIONS AND NOTES.
8. PROJECT DESIGN PROFESSIONAL OF RECORD IS RESPONSIBLE FOR PREPARATION OF THE PROJECT SPECIFIC DSA 103 AND IS RESPONSIBLE FOR ALL SHOP DRAWING AND SUBMITTAL REVIEWS. SEE SB0.3 FOR EXAMPLE DSA 103

STRUCTURAL NOTES

GENERAL NOTES

1. The following notes, typical details and schedules shall apply to all phases of this project unless otherwise shown or noted.
2. Specific notes and details shall take precedence over general notes and typical details.
3. All materials and workmanship shall conform to the minimum standards of the 2022 edition Title 24 of the California Building Code (CBC) and such other regulating agencies exercising authority over any portion of the work. The Contractor shall have a current copy of the CBC on the job site.
4. The "Contract or Construction Documents" shall consist of these notes, details, schedules, plans, and drawings.
5. All specifications, including but not limited to materials and products, shall be those put forth in the "Contract or Construction Documents". No substitutions shall be permitted to be used or assumed to be used in the bidding or construction process without written approval by the Structural Engineer of Record.
6. The Contractor shall examine the "Contract or Construction Documents" and shall notify the Architect or Structural Engineer of Record of any discrepancies he may find before proceeding with the work.
7. All information on existing conditions shown on drawings are based on best present knowledge available, but without guarantee of accuracy. The Contractor shall verify and be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.
8. The Contractor shall immediately notify the Architect or Structural Engineer of Record of any condition which in his opinion might endanger the stability of the structure or cause distress of the structure.
9. All work shall conform to the best practice prevailing in the various trades comprising work. The Contractor shall be responsible for coordinating the work of all trades.
10. These "Contract or Construction Documents" represent the finished structure, and do not indicate the method of construction. The Contractor shall supervise and direct the work and shall be solely responsible for construction means, methods, techniques, sequences and procedures.
11. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
- A. Labeling (as required or specified) shall be provided in accordance with CBC Section 1703A.5.
- B. Evaluation and follow-up inspection services (as required or specified), shall conform to CBC Section 1703A.6.
12. The Contractor shall provide temporary bracing and shoring for all structural members as required for structural stability of the structure during all phases of construction.
13. The Contractor shall take all steps necessary to ensure proper alignment of the structure after the installation of all structural and finish materials. This shall include any necessary preloading of the structure to determine final position of the completed work.
14. Observation visits to the project site by field representatives of Architect and/or Structural Engineer of Record (support services) shall not include inspections of safety or protective measures, nor construction procedures, techniques or methods. Any support services performed by Architect or Structural Engineer of Record during any phase of construction, shall be distinguished from continuous and detailed inspection services (as required by any regulating governmental agency, e.g. the Authority Having Jurisdiction) provided by others. These support services, whether of material or work, are performed solely for the purpose of assisting in quality control and in achieving conformance with contract documents, but do not guarantee Contractor's performance and shall not be construed as supervision of construction.
15. These notes, details, drawings and specifications (Contract or Construction Documents) do not carry necessary provisions for construction safety. These documents and all phases of construction hereby contemplated are to be governed, at all times, by applicable provisions of the current California Occupational Safety and Health Act.
16. Where any conflict occurs between the requirements of federal, state and local laws, codes, ordinances, rules and regulations, the most stringent shall govern.
17. Written dimensions shall have precedence over scaled dimensions.
18. Drawings (notes, schedules, details and plans) shall have precedence over Structural Calculations.
19. In the event that certain features of the construction are not fully shown on the drawings or called for in the General Notes or Specifications, then their construction shall be of the same character as for similar conditions that are shown or called for.
20. ASTM designation and all standards refer to the latest amendments.
21. These structural "Contract or Construction Documents" shall not be modified without prior written approval of the Structural Engineer of Record.
22. Only structural working drawings approved by the Division of the State Architect are permitted to be used for construction on this project. All other drawings or documents are obsolete and are not permitted on the job site, nor shall they be used for any construction purposes. Contractors using unapproved drawings or documents are solely responsible for all work not performed in accordance with the "approved" drawings.
23. A Division of the State Architect certified project inspector employed by the District (Owner) and approved by the Division of the State Architect shall provide continuous inspection of the work. The duties of the inspector are defined in Section 4-342, Part 1, Title 24 California Code of Regulations.

FOUNDATION NOTES

1. Basis: See Structural Design Values Chart, Sheet SB0.1 Table B
2. Unexpected soil conditions: Allowable values and foundation design are based upon the minimum values provided in Table 1806A.2 of the 2022 California Building Code. See SB0.1 for values
3. Excavate to required depths and dimensions (as indicated in drawings), cut square and smooth with firm level bottoms. Care shall be taken not to over-excavate foundation at lower elevation and prevent disturbing of soils around higher elevation.
4. Footings shall be poured in neat excavations, without side forms whenever possible.
5. Carry all foundations to required depths into compacted fill or natural soil (as per Structural Plans and Details).
6. All foundation excavations shall be inspected and approved by the Inspector of Record or Geotechnical Engineer prior to forming and placement of reinforcing or concrete.
7. Foundations shall not be poured until all required reinforcing steel, sleeves, inserts, conduits, pipes, etc. and formwork is properly placed and inspected by the Authority having Jurisdiction.
8. The sides and bottoms of excavations which are to have concrete contact must be moistened several times just prior to pouring upon them.
9. De-water footings, as required, to maintain dry working conditions.

REINFORCING STEEL

1. All reinforcing steel shall be deformed intermediate grade bars conforming to ASTM A615, Grade 60 (f_y = 60 ksi) unless noted otherwise.
2. Reinforcing steel shall not be welded, unless specifically noted otherwise.
3. To hold reinforcing bars in their true position and prevent displacement, standard tie and anchorage devices must be provided. Placing of reinforcement shall conform to ACI 318-19 Section 26.6.2.
4. Shop drawings for fabrication of any reinforcing steel shall be approved by Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record, for their review, prior to fabrication.
5. Refer to typical details for minimum splice length and minimum radius of bend of reinforcing steel.
6. All reinforcing steel splices shall be staggered 24", unless specifically noted or detailed otherwise.
7. All reinforcing bar bends shall be made cold.
8. Fabrication, erection and placement of reinforcing steel shall conform to Concrete Reinforcing Steel Institute of Standard Practice.
9. Reinforcing steel shall be clean of rust, grease or other material likely to impair bond.

CONCRETE

1. All concrete shall have a minimum ultimate compressive strength (f_c) as outlined below at 28 days. All concrete shall be regular weight (unless specifically noted otherwise).
- A. Concrete for footings: 4,500 psi w/c = 0.45 max.
2. Maximum Fly Ash content shall be 15% by weight, of total cementitious materials and shall conform to ASTM C618.
3. All concrete work shall comply with CBC Chapter 19A and ACI 318-19 and latest edition of ACI Manual of Concrete Practice.
4. Special Inspection (as required or specified) shall conform to CBC Chapter 17A.
5. Cement shall be portland cement Type V and shall conform to ASTM C150.

6. Aggregates shall conform to ASTM C33, provide aggregates from a single source.
7. Water shall conform to ASTM C94 and be potable.
8. Where not specifically detailed, the minimum concrete cover on reinforcing steel shall be:
- A. Concrete cast against and permanently exposed to earth or weather: 3"
10. All reinforcing steel, anchor bolts, dowels, inserts and any other hardware to be set in concrete shall be well secured in position prior to pouring of concrete.
11. Vibrate all concrete as it is placed, with a mechanical vibrator operated by experienced personnel. The vibrator shall be used to consolidate the concrete, not transport it. Reinforcing and forms shall not be vibrated.
12. Formwork design and removal shall conform to ACI 318-19 Section 26.11. Remove forms in accordance with the following minimum schedule:
- A. Side forms of footings: Minimum 48 hours
- B. Column and pier forms: 72 hours & 70% of design strength
15. Concrete shall not free fall more than six feet. Use tremie, pump or other approved methods.
16. Concrete shall be maintained in a moist condition for a minimum of 5 days after placement.
17. The Contractor may use concrete admixtures as a construction means and methods to execute "Contract or Construction Documents". Use of admixture is solely the responsibility of the Contractor.
18. Mix designs shall be prepared by an approved testing laboratory, signed by a licensed engineer and shall be submitted to the Project Specific Design Professional of Record for approval. SSG is not responsible for review or approval of site specific concrete mix design.
19. Only one grade of concrete shall be allowed on project site at any one time

20. Concrete strength shall be verified by standard cylinder tests (in accordance with CBC Section 1905A.1.16) made by an approved testing laboratory.
21. Concrete placed when the air temperature has fallen to, or is expected to fall below 40° shall conform to ACI 318-19 Section 26.5.4, and ACI 306R-16.
22. Concrete placed during hot weather shall conform to ACI 318-19 Section 26.5.5, and ACI 305R-14.
23. Conduits and sleeves placed within structural concrete shall not be tied directly to structural reinforcement.
- A. 1" concrete cover shall be maintained around all reinforcement.
24. No stakes shall be permitted within the footing section.
25. Concrete shall reach minimum 75% design strength or cure for 3 days minimum prior to installation of steel columns and scoreboard components.

DRILLED CAISSON/PIER AND GRADE BEAM NOTES

1. Excavations for drilled caissons/pier shall be performed in compliance with local grading codes and ordinances as well as CBC Chapters 18A and 33A.
2. Provide Special Inspection in accordance with CBC Section 1705A.8 and Table 1705A.8.
3. Excavations for all drilled caissons/piers shall be approved by the Project Geotechnical Engineer or Project Special Inspector prior to placing of concrete.
4. Reinforcement for drilled caissons/pier shall be approved by the Structural Engineer of Record prior to placing in caisson/pier excavation.
5. De-water caisson/pier footings and building excavation as required to maintain dry working conditions.
6. Caisson/piers are to be poured within 24 hours after completion of drilling operation. Shoring requirements shall be determined by contractor. Contractor shall provide fall protection and safety barriers at and near the drilled hole as required by OSHA and the Authority Having Jurisdiction.
7. The Contractor shall be responsible for all shoring, bracing, etc. necessary to support cut and/or fill banks, and existing structures during excavation, and the forming and placement of concrete.
8. Bottom of caissons/piers shall be thoroughly cleaned prior to placement of concrete.

STRUCTURAL STEEL AND WELDING

1. All structural steel construction shall conform to AISC 360-16 and AISC 341-16.
- A. Fabrication of all structural steel shall be done in the shop of an approved fabricator. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
2. All structural steel shall conform to the following specifications:
- A. Angles, channels, plates, bars, rounds, and other miscellaneous shapes: Shall conform to ASTM A36 and shall have a minimum yield stress (F_y) of 36 ksi.
- B. Wide-flange shapes: Shall conform to ASTM A992 and shall have a minimum yield stress (F_y) of 50 ksi.
- C. Structural tubes: Shall be ASTM A500, Grade C, and shall have a min. yield stress (F_y) of 50ksi.
3. All structural-steel fasteners shall conform to the following specifications:
- A. Bolts shall conform to ASTM A307.
- B. Anchor Bolts shall conform to ASTM F1554, Grade as noted in drawings
- C. Carbon steel nuts shall conform to ASTM A563
- D. Stainless steel nuts shall conform to ASTM F594.
- E. Washers shall conform to ASTM F436
4. Special Inspection shall be provided for all structural steel and welding, in accordance with CBC Chapter 17A.
5. All structural steel shall be fabricated, erected and welded in accordance with AISC Specifications for Structural Steel Buildings (AISC 360-16) and Code of Standard Practice for Steel Buildings and Bridges (AISC 303-16).
6. All welding shall be done by qualified and certified welders.
7. Shop drawings for the fabrication of any structural steel shall be approved by the Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record for their review, prior to fabrication.
8. No holes other than those specifically detailed shall be allowed through structural steel members. Burning of holes is not permitted.
9. All welding shall conform to 'AWS D1.1' specifications for welding. (E-70XX Electrodes).
10. Where fillet weld size is not indicated, use 'AWS' minimum size based on the thickness of the thinner part being welded, as specified in AISC Specifications for Structural Steel Buildings (AISC 360-10), Section J2.2.
11. All butt welds to be complete joint penetration, unless specifically noted otherwise.
12. Welder qualification requirements, welding procedure and welding electrodes for all structural steel (except structural sheet steel, see steel decking) shall conform to CBC Sections 1705A.2.1 and 2204A.1.
13. Provide 3" minimum concrete cover around all structural steel below grade.
14. Structural steel embedded into concrete shall be uncoated.
15. Structural steel shall be hot-dip galvanized (minimum ASTM A123 or A153 Class D) or painted with zinc-rich primer, undercoat, and finish coat; or equivalent paint system.
16. All exposed steel fasteners, including cast-in-place anchor bolts/rods, shall be stainless steel (Type 304 minimum), hot-dip galvanized (ASTM A153, Class D minimum or ASTM F2329), or protected with corrosion preventive coating that demonstrated no more than 2% of red rust in minimum 1,000 hours of exposure in salt spray test per ASTM B117. Zinc plated fasteners do not comply with this requirement.

ABBREVIATIONS

A.B.	Anchor Bolt	HORIZ.	Horizontal
ABV.	Above	HSS	Hollow Steel Section
ACI	American Concrete Institute	HT.	Height
ADI.	Adjacent	ICC	International Building Code
AJI	Division of the State Architect	ICC	International Code Council
ASC	American Institute of Steel Construction	ID	Inside Diameter
AOR	Architect of Record	IN.	Inch, Inches
APPROX.	Approximately	INT.	Interior
ASCE	American Society of Civil Engineers	ksi	Kips per Square Inch
ARCH.	Architect, Architecture	LL	Live Load
ASTM	American Society of Testing and Materials	MAX.	Maximum
ATR	All Thread Rod	MB	Machine Bolt
AWS	American Welding Society	MFR	Manufactured, Manufacturer
B.O.	Bottom of _____	MIN.	Minimum
BOT.	Bottom	MPI	Miles per Hour
b/t	Between	N/R	Not Required
CAC	California Administrative Code	N.T.S.	Not to Scale
CBC	California Building Code	o.c.	On Center
CIP	Cast-in-place	o/	Over
CJP	Complete Joint Penetration	OD	Outside Diameter
CL	Centerline	PEN.	Penetration
CLR	Clear	PL	Plate
COL	Column	PJP	Partial Joint Penetration
CONC.	Concrete	psi	Pounds per Square Inch
CONN.	Connection	PSF	Pounds per Square Foot
CONT.	Continue, Continuous	REBAR	Reinforcing Bar
Ø	Diameter	REINF.	Reinforcement
DBL	Double	REQ'D	Required
DET.	Detail	S.F.	Square Feet
DL	Dead Load	SHT	Sheet
DSA	Division of State Architect	SIM.	Similar
DWGS.	Drawings	SMS	Sheet Metal Screw
EA	Each	SQ	Square
E.F.	Each Face	STAGCD	Staggered
ELEC.	Electric, Electrical	STD.	Standard
ELEV.	Elevation	STL	Steel
EMBED.	Embedded, Embedment	SEOR	Structural Engineer of Record
EOR	Engineer of Record	T&B	Top and bottom
EQ.	Equal	THR'D	Threaded
EQUIP.	Equipment	T.O.	Top of _____
E.S.	Each Side	TYP.	Typical
E.W.	Each Way	U.N.O.	Unless Noted Otherwise
EXT.	Exterior	VERT.	Vertical
FAB.	Fabricated	VIF	Verify in Field
FDN.	Foundation	w/c	Water/Cement Ratio
F.G.	Finish Grade	WT.	Weight
F.O.	Face of _____		
FRMG.	Framing		
FT.	Foot, Feet		
FTC.	Footing		
GA.	Gauge		
GALV.	Galvanized		
GEOR	Geotechnical Engineer of Record		

POST INSTALLED ANCHOR & TESTING

1. All post-installed anchors are to be tension tested with the exception that torque testing is allowed if the anchors are specifically designed as torque controlled
2. Test quantity of post-installed anchors as noted below:

Application	Quantity
Non-structural (Equipment Anchorage, etc.)	50%
Structural	100%

3. Apply proof test loads to anchors without removing the nut if possible. If not, remove nut and install a threaded coupler to the same tightness of the original nut using a torque wrench and apply load.
4. All tests shall be performed in the presence of the inspector.
5. Reaction loads from test fixtures may be applied close to the anchor being tested, provided the anchor is not restrained from withdrawing or restricted from a concrete shear cone type failure mechanism.
6. Test equipment is to be calibrated by an approved testing laboratory in accordance with standard recognized procedures.
7. The following criteria apply for the acceptance of installed anchors:
- A. Hydraulic ram method: anchors tested with a hydraulic jack or spring loaded devices shall maintain the test load for a minimum of 15 seconds and shall exhibit no discernable movement during the tension test, e.g. as evidenced by loosening of the washer under the nut.
- B. Torque wrench method: anchors tested with a calibrated torque wrench must attain the manufacturer recommended torque within ½ turn of the nut.
- Exceptions:
- Wedge or sleeve type: one-quarter turn of the nut from 3/8" sleeve anchor only.
 - Threaded type: one-quarter turn of the screw after initial seating of the screw head.
8. If any anchor fails testing, test all anchors of the same type not previously tested until twenty consecutive anchors pass, then resume the initial test frequency. If the anchors are used for the support and bracing of non-structural components (pipe, duct or conduit), the twenty shall be only those anchors installed by the same trade.
9. Test loads per ICC ESR, IAPMO, OR UES report
10. When installing drilled-in anchors and/or powder driven pins in existing non-prestressed reinforced concrete, use care and caution to avoid cutting or damaging the existing reinforcing bars. When installing them into existing prestressed concrete (pre- or post-tensioned) locate the prestressed tendons by using a non-destructive method prior to installation. Exercise extreme care and caution to avoid cutting or damaging the tendons during installation. Maintain a minimum clearance of one inch between the reinforcement and the drilled-in anchor and/or pin.

ANCHOR TORQUE TEST VALUES

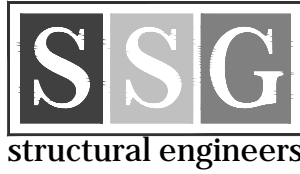
Anchor Diameter	CONCRETE		MASONRY	
	HILTI KB TZ 2	SIMPSON STRONG BOLT Z	HILTI KB TZ 2	SIMPSON STRONG BOLT Z
	ESR-4266	ESR-3037	ESR-4561	ER-240
3/8"	30 ft-lb	30 ft-lb	15 ft-lb	20 ft-lb
1/2"	50 ft-lb	60 ft-lb	25 ft-lb	35 ft-lb
5/8"	40 ft-lb	90 ft-lb	30 ft-lb	55 ft-lb
3/4"	110 ft-lb	150 ft-lb	50 ft-lb	100 ft-lb

If the manufacturer's recommended installation torque is less than the test torque noted in the table, the manufacturer's recommended installation torque should be used in lieu of the tabulated values.

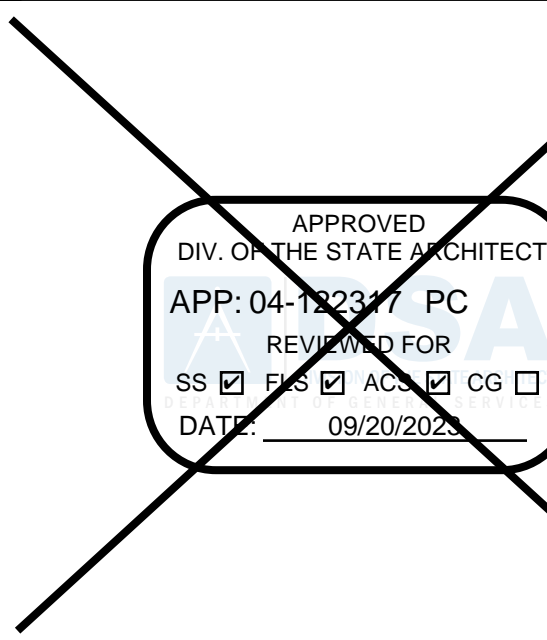
See manufacturer's ESR report for Maximum Impact Wrench Torque Rating.

APPLICATION# 02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122089 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024



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PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application for construction is required.



STRUCTURAL
NOTES &
SPECIAL
INSPECTIONS

SHEET INFORMATION
DATE 08.09.2023
DRAWN JMK
CHECKED MEP
SSG JOB # S23109
SHEET SB0.2

~~EXAMPLE DSA 103 - TESTING AND INSPECTIONS~~

ASA 103-22 LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS, 2022 CBC		
Project Number:	School Name:	School District:
Contract Number:	Nexus ScoredBy/PC:	Nexus ScoredBy/PC:
DSA Test Number:	Increment Number:	Date Created:
		Date Closed:
		2022-03-20 10:38:59
2022 CBC		
<p>*IMPORTANT: This form is only a summary listing of structural tests and some of the special inspections required for the project. Generally, the structural tests and special inspections noted on this form are those that will be performed by the Special Inspector of Design/Laboratory of Structural Steel Inspection. The actual complete test and inspection program must be determined as detailed on the DSA approved contract. It depends at the bottom of this form identifies work NOT subject to the DSA requirement for structural testing.</p> <p>The Special Inspector is responsible for providing inspection of all facets of construction, including but not limited to, special inspections such as steel erection, concrete placement, welding, cold formed steel framing, anchorage of non-structural components, etc., per Title 24 Part 2, Chapter 17A (2022 CBC).</p>		
<p>**NOTE: Underlined section and table references found in this document are from the CBC or California Building Code.</p>		
KEY TO LOCATIONS		
1. TYPE	PERFORMED BY	
Continuous – Indicates that a continuous special inspection is required	Civil/Structural Engineer – Indicates that the special inspection shall be performed by a registered professional engineer who is his authorized representative.	
Periodic – Indicates that a periodic special inspection is required	LOR – Labor Only – Indicates that the special inspection shall be performed by a LOR (Laboratory of Structural Steel Inspection) and Acceptance EIAI (Permit) – See Section C-B35.	
	Ft Project Inspection – Indicates that the special inspection may be performed by an engineer.	
	Special Inspection – Indicates where specified by approved DSA.	
Test – Indicates that a test is required	SI Special Inspector – Indicates that the special inspector shall be performed by an appropriately qualified/certified person.	
<p>DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES STATE OF CALIFORNIA</p>		

D5A 10-22-22 LISTS OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (CONCRETE), 2022 CBC						
Table 1705A-1, Table 1718-B-19 Section 26.1, 26.13						
Inspection Numbers			School Name:		School District:	
66 1228-1000			Newco School Board		District 66	
66 419-1000			Newco Schoolboard PC		District 66	
Test or Special Inspection						
1. Verify the inspection is enough prior to placing of post-tensioning tendons			Periodic	SI	Table 1705A-1, Item 3. Special Inspection to verify specified concrete strength test prior to installing.	
2. Inspection application of post-tensioning and post-tensioning forces and providing of post-tensioning tendons			Continuous	SI	1705A-3, 4, Table 1705A-1 Item 3, 1718-B-19 Section 26.13	
C1 PRECAST CONCRETE (IN ADDITION TO SECTION C1)						
Test or Special Inspection			1718-B-19	Performed by	Code Reference and Notes	
1. Inspection installation of precast concrete members				SI	1718-B-19 Section 26.13	
2. Inspection erection of precast concrete members.				SI	1718-B-19 Section 26.13	
3. For precast concrete diaphragms connected to reinforcement at joints designed as moment or high deformable elements (RCC or RSC or RSC in situations assigned to Group Design Category 2 or 3) inspect such connections and reinforcement in the field and at joints			Continuous	SI	Table 1705A-1, Item 3. May be performed by field inspection specially approved by DSA.	
4. Installation of the embedded parts, 5. Completion of the continuity of the reinforcement, 6. Completion of connections					1718-B-19, 1718-B-19 Section 26.13, 1718-B-19 Section 26.13	
5. Inspection installation of precast concrete diaphragm connections for compliance with ACI 308.5.			Periodic	SI	Table 1705A-1, 1718-B-19 Section 26.13, 26.13, 26.13	

QSA 103-22- LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC				
1. Project Information Project Name: 1745 602-16, Tables 3 and 4 School Name: Nevus Scoreboards PC Project Number: Nevus Scoreboards PC QSA 1227-23-1 QSA File No:		School District: Nevus Scoreboards PC Date Created: 02/20/2023		
Test or Special Inspection	Type	Performed By	Code References and Notes	
<input checked="" type="checkbox"/> a. Test post installed anchors <input type="checkbox"/> b. Test post installed anchors	Test	LOH	17054.4, 1910d.3, See Appendix (end of this form) for exemptions.	
2. Other Information IMA, OTHER MASONRY: Test or Special Inspection				
<input type="checkbox"/> a.				

[illegible]

ISA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS/SIGNATURE, 2022 CBC			
As Issued Number:	School Name:	School District:	
03-12345	Neenah-Sawdonville PC	Neenah-Sawdonville PC	
DSA File Number:	Increment Number:	Date Created:	
		2024-05-11 08:55:36	
Name of Architect or Engineering General responsible (change)			
Name of Structural Engineer (When structural Engineer has been delegated):			
Signature of Architect or Structural Engineer:			
Notes: To facilitate DSA electronic mark-ups and identification stamp application, DSA recommends submitting secured electronic or digital signatures.			
		<div style="border: 1px solid black; padding: 5px; text-align: center;"> DSA STAMP </div>	

[illegible][illegible][illegible][illegible]

DSA 103-22-12 LIST OF REQUIRED VERIFIED REPORTS, CBC 2022		
Inspection Number: 000117	School Name: Newcomb/Boyd PC Incident Number:	School District: Newcomb/Boyd PC Date Created: 2023-01-11 10:55:36
<p>1. Structural Damage; Inspection Laboratory Verified Report Form DSA 291</p> <p>2. Post-Installed Anchors; Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292</p> <p>3. Welding Inspection; Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292</p> <p>4. Welding Inspection; Laboratory Verified Report Form DSA 291, or, for independently contracting SI, Special Inspection Verified Report Form DSA 292</p>		

[illegible]

DSA 103-22- LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC DSA 103-2-1405 and 1406, Tables 2 and 4				School Name: School District: School Address:
DS# 1228 DS# 1229 DS# 1230	Item# Item Name Item Description	School District: Inspector Name: Inspector Title:	School District: Inspector Name: Inspector Title:	
III. STRUCTURAL MASONRY Item# _____				
Test or Special Inspection Test or Special Inspection _____				
1. All exterior walls, chimneys, and other masonry structures shall be constructed in accordance with the requirements of the International Building Code (IBC) and the International Masonry Institute (IMI) Code of Practice for Masonry Construction.	Periodic	Performed	21024, 4. See References and Notes 21024.4, 21024.5, 21024.6, 21024.7, 21024.8, 21024.9, 21024.10, 21024.11, 21024.12, 21024.13, 21024.14, 21024.15, 21024.16, 21024.17, 21024.18, 21024.19, 21024.20, 21024.21, 21024.22, 21024.23, 21024.24, 21024.25, 21024.26, 21024.27, 21024.28, 21024.29, 21024.30, 21024.31, 21024.32, 21024.33, 21024.34, 21024.35, 21024.36, 21024.37, 21024.38, 21024.39, 21024.40, 21024.41, 21024.42, 21024.43, 21024.44, 21024.45, 21024.46, 21024.47, 21024.48, 21024.49, 21024.50, 21024.51, 21024.52, 21024.53, 21024.54, 21024.55, 21024.56, 21024.57, 21024.58, 21024.59, 21024.60, 21024.61, 21024.62, 21024.63, 21024.64, 21024.65, 21024.66, 21024.67, 21024.68, 21024.69, 21024.70, 21024.71, 21024.72, 21024.73, 21024.74, 21024.75, 21024.76, 21024.77, 21024.78, 21024.79, 21024.80, 21024.81, 21024.82, 21024.83, 21024.84, 21024.85, 21024.86, 21024.87, 21024.88, 21024.89, 21024.90, 21024.91, 21024.92, 21024.93, 21024.94, 21024.95, 21024.96, 21024.97, 21024.98, 21024.99, 21024.100, 21024.101, 21024.102, 21024.103, 21024.104, 21024.105, 21024.106, 21024.107, 21024.108, 21024.109, 21024.110, 21024.111, 21024.112, 21024.113, 21024.114, 21024.115, 21024.116, 21024.117, 21024.118, 21024.119, 21024.120, 21024.121, 21024.122, 21024.123, 21024.124, 21024.125, 21024.126, 21024.127, 21024.128, 21024.129, 21024.130, 21024.131, 21024.132, 21024.133, 21024.134, 21024.135, 21024.136, 21024.137, 21024.138, 21024.139, 21024.140, 21024.141, 21024.142, 21024.143, 21024.144, 21024.145, 21024.146, 21024.147, 21024.148, 21024.149, 21024.150, 21024.151, 21024.152, 21024.153, 21024.154, 21024.155, 21024.156, 21024.157, 21024.158, 21024.159, 21024.160, 21024.161, 21024.162, 21024.163, 21024.164, 21024.165, 21024.166, 21024.167, 21024.168, 21024.169, 21024.170, 21024.171, 21024.172, 21024.173, 21024.174, 21024.175, 21024.176, 21024.177, 21024.178, 21024.179, 21024.180, 21024.181, 21024.182, 21024.183, 21024.184, 21024.185, 21024.186, 21024.187, 21024.188, 21024.189, 21024.190, 21024.191, 21024.192, 21024.193, 21024.194, 21024.195, 21024.196, 21024.197, 21024.198, 21024.199, 21024.200, 21024.201, 21024.202, 21024.203, 21024.204, 21024.205, 21024.206, 21024.207, 21024.208, 21024.209, 21024.210, 21024.211, 21024.212, 21024.213, 21024.214, 21024.215, 21024.216, 21024.217, 21024.218, 21024.219, 21024.220, 21024.221, 21024.222, 21024.223, 21024.224, 21024.225, 21024.226, 21024.227, 21024.228, 21024.229, 21024.230, 21024.231, 21024.232, 21024.233, 21024.234, 21024.235, 21024.236, 21024.237, 21024.238, 21024.239, 21024.240, 21024.241, 21024.242, 21024.243, 21024.244, 21024.245, 21024.246, 21024.247, 21024.248, 21024.249, 21024.250, 21024.251, 21024.252, 21024.253, 21024.254, 21024.255, 21024.256, 21024.257, 21024.258, 21024.259, 21024.260, 21024.261, 21024.262, 21024.263, 21024.264, 21024.265, 21024.266, 21024.267, 21024.268, 21024.269, 21024.270, 21024.271, 21024.272, 21024.273, 21024.274, 21024.275, 21024.276, 21024.277, 21024.278, 21024.279, 21024.280, 21024.281, 21024.282, 21024.283, 21024.284, 21024.285, 21024.286, 21024.287, 21024.288, 21024.289, 21024.290, 21024.291, 21024.292, 21024.293, 21024.294, 21024.295, 21024.296, 21024.297, 21024.298, 21024.299, 21024.300, 21024.301, 21024.302, 21024.303, 21024.304, 21024.305, 21024.306, 21024.307, 21024.308, 21024.309, 21024.310, 21024.311, 21024.312, 21024.313, 21024.314, 21024.315, 21024.316, 21024.317, 21024.318, 21024.319, 21024.320, 21024.321, 21024.322, 21024.323, 21024.324, 21024.325, 21024	

[illegible]

DSA 103-22: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (OTHER) 2022 CBC			
Question Number: 0000000000	School Name: Nevada Countywide PC	School District: Nevada-Southwestern PC	
DSA ID Number: 0000000000	Increment Number: 0000000000	Date Created: 2022-05-10 10:03:36	
VI. OTHER			
<input type="checkbox"/> Test or Special Inspection	Type	Performed By	Code References and Notes
<input type="checkbox"/> a. Lead test for identified problem	Test	LDR	1709A.2, 1709A.3. Testing is not required for DSA 103-22 if a product with a valid evaluation service report (ESR) is used. If a product that can be justified by structural calculations.
<input type="checkbox"/> b. Insulation torque for non-bolt holes	Continuous	SP	1709A.2 is covered by the code referenced in Essential Service Facility Projects (ESFP) 0250. Essential services were used, verified by SP testing results, and per 2.1.10.5. Communication Towers, Pole and Buildings. The code is not required for Essential Services Communication Tower ESFP. Note ESFP may use a third-party code reference to DSA.
<input type="checkbox"/> c.			

NCSA 103-22 LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (SOILS), 2022 CCRB			
Project Name: 1705A TOSCA 1705A TOSCA		School District: Newcombs Creek Date Created: 2023-05-29 09:38:30	
Inspector: DGA #12217 Newcombs Creek Inspector Number:		School Name: Newcombs Creek Date Created: 2023-05-29 09:38:30	
Test or Special Inspection	Type	Performed By	Code References and Notes
SS. RETAINING WALLS			
Test or Special Inspection	Type	Performed By	FIGURE C-1 "By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03.".
a. Placement, compaction and inspection of backfill	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
b. Construction soil reinforcement and/or chainage devices	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
c. Sequence retaining walls, inspect placement of earth-shield components, etc.	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
d. Segment retaining walls, inspect placement of earth-shield components, etc.	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
e. Masonry retaining walls.	Provide test area	Inspectors per FIGURE C-1	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
SS. OTHER SOILS			
Test or Special Inspection	Type	Performed By	FIGURE C-1 "By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
a. Soil Improvements	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
b. Soil Improvements	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."
c.	Continuous	GK*	"By geotechnical engineer or other person or his or her qualified representative. See Section B-4.03."

ASA 103-22- LISTS OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC					
TMS 602-16 Table 4 Items 1 and 2					
Inspection Number		School District		School Districts	
Inspection Number		Newco Secondary PC		Newco Secondary PC	
Inspection Number		Inspection Number		Inspection Number	
Test or Test	Description	Type	Performed By	Code Reference and Remarks	
1	Verify size, grade, quantity of reinforcement, connections, and anchor bolts. Verify condition of bars and connections.	Periodic	SI	TMS 602-16 Table 4 Items 1 & 2c.	
2	Inspect placement of reinforcement for bar bolls, and connections.	Continuous	SI	TMS 602-16 Table 4 Item 2a.	
3	Placement, consolidation, and reinspection of concrete.	Continuous	SI	TMS 602-16 Table 4 Item 1 & 2b.	
4	Inspect placement of masonry units and construction of mortar joints.	Periodic	SI	TMS 602-16 Table 4 Item 1.	
5	Verify placement, construction and protection of masonry and mortar joints. Verify mortar joints below 40° (9 for hot weather temperature above 50° F).	Periodic	SI*	TMS 602-16 Table 4 Item 1. May be performed by the project inspector when specifically approved by ISA.	
6	Inspect type, size, installation of anchors and all other items to be embedded in masonry including other details.	Continuous	SI	TMS 602-16 Table 4 Item 2a.	
7	Inspect type, size, installation of reinforcement bars and other connections.	Continuous	SI	TMS 602-16 Table 4 Item 2a.	
8	Inspect joint work, including mortar protrusions, joints and placement of joint.	Continuous	SI	TMS 602-16 Table 4 Item 2a.	
9	Watching of reinforcing work.			TMS 602-16 Table 4 Item 3a. Provide special inspection for STEEL, Concrete, S/C/M, & Jut anchor (SI/MSI) & 10 below.	

[illegible][illegible][illegible]

DSA 103-122-2: LISTING OF STRUCTURAL TESTS & SPECIAL INSPECTIONS (MASONRY), 2022 CBC							
DSA 103-122-2, Tables 1 and 2		School Name: New Brunswick PC		School District: New Brunswick PC			
Job Number: 04-122-0000		Inspector Name: DGA-0000		Inspector Number: 0000000000			
M1. MINERAL WOOL BLOSSOM PROTECTION							
Test or Special Inspection	Type	Performed By	Code References and Notes				
1. Verify projections of steel reinforcement and/or anchor/masonry connections at joints	Periodic	SI	TMS-602-16 Table 3 Item 5 and Table 3 Item 14, 16, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742				

[illegible][illegible]

NOTE: THE DSA 103 SHOWN INCLUDES MINIMUM SPECIAL INSPECTION REQUIREMENTS AND IS PROVIDED AS EXAMPLE. ADDITIONAL TESTING AND INSPECTIONS MAY BE REQUIRED BEYOND THE SCOPE OF THE SCOREBOARD. A FINAL DSA 103 FORM SHALL BE SUBMITTED BY THE DESIGN PROFESSIONAL OF RECORD AS PART OF THE SITE SPECIFIC SUBMITTAL REQUIREMENTS. THE DESIGN PROFESSIONAL OF RECORD IS RESPONSIBLE FOR REVIEWING PROJECT SPECIFIC SPECIAL INSPECTION REPORTS. SSG STRUCTURAL ENGINEERS OR NEVCO ARE NOT RESPONSIBLE FOR PROVIDING THE PROJECT DSA 103 OR REVIEWING SPECIAL INSPECTION REPORTS.

DSA 103 CAN BE FOUND AT: <https://forms.dgs.ca.gov/content/forms/at/dgs/dsa/form-103/public/dsa-form-103-22.html>

APPLICATION# 02-122089		IDENTIFICATION STAMP DIV. OF THE STATE ARCHITECT	
APP: 02-122089 INC:		REVISED FOR:	
CS <input checked="" type="checkbox"/>	FLS <input checked="" type="checkbox"/>	ACS <input checked="" type="checkbox"/>	IPDS <input checked="" type="checkbox"/>
DATE: 4/3/2024			
<div><div>SSG</div><div>structural engineers</div></div>			
<div><div><div>REGISTERED PROFESSIONAL ENGINEER</div><div>Michael E. Farnoff</div><div>No. 5405</div><div>STATE OF CALIFORNIA</div><div>STRUCTURAL</div></div><div>08/09/21</div></div>			
PC SEJOR SEAL			
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301 East Harris Avenue, Greenville, Illinois 62246

Phone: (618) 664-0360

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PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required.

EXAMPLE DSA 103 - TESTING AND INSPECTIONS

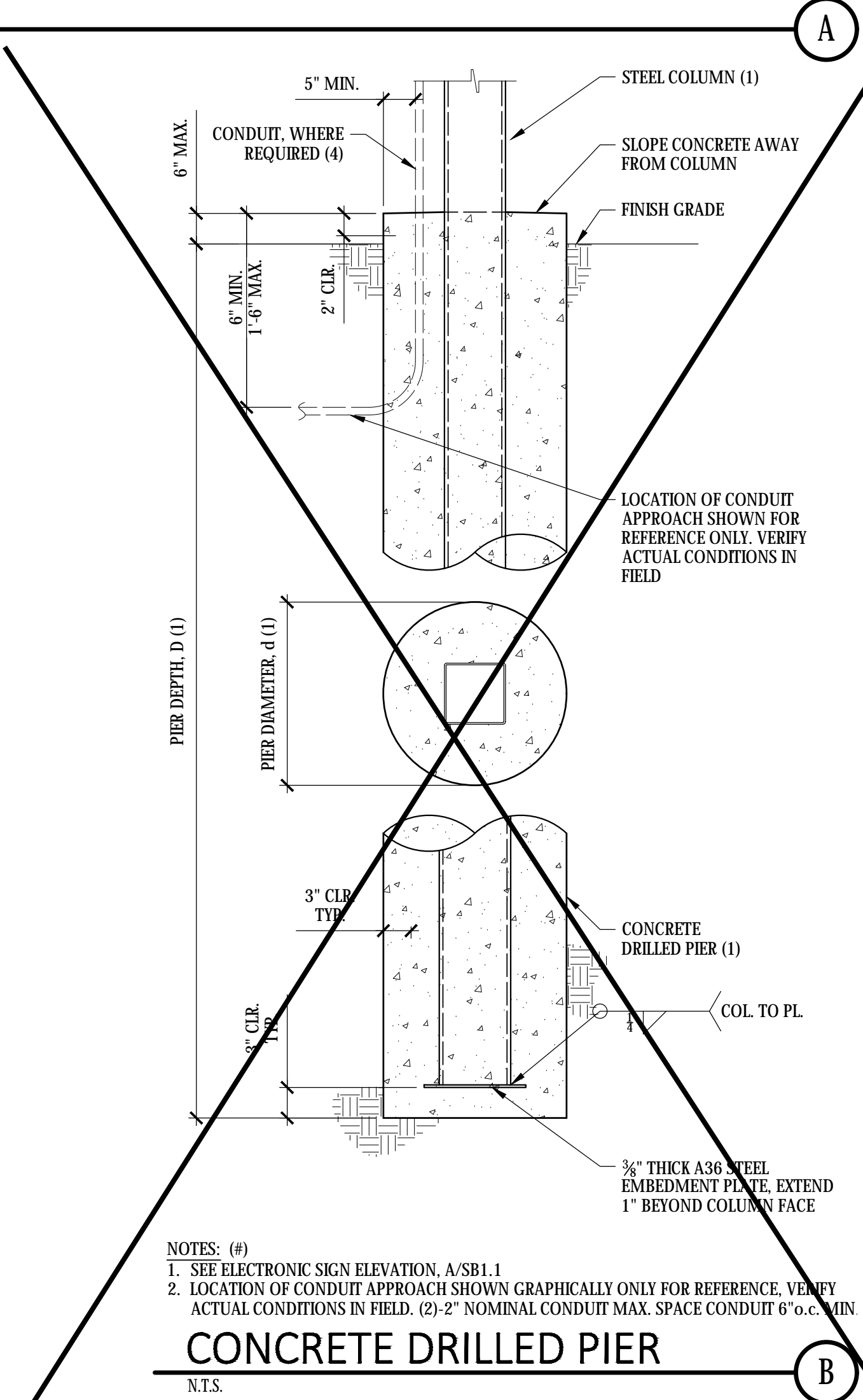
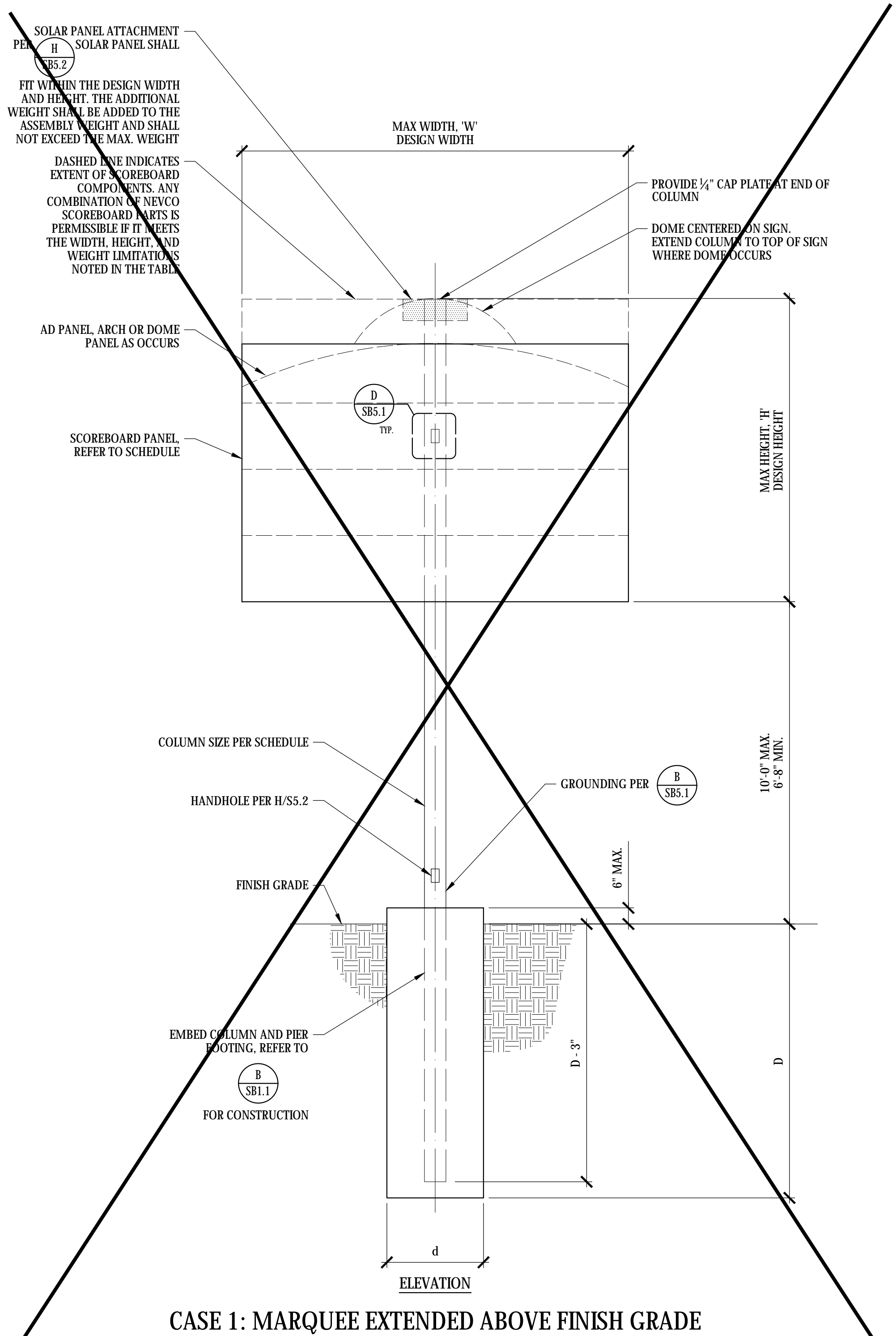
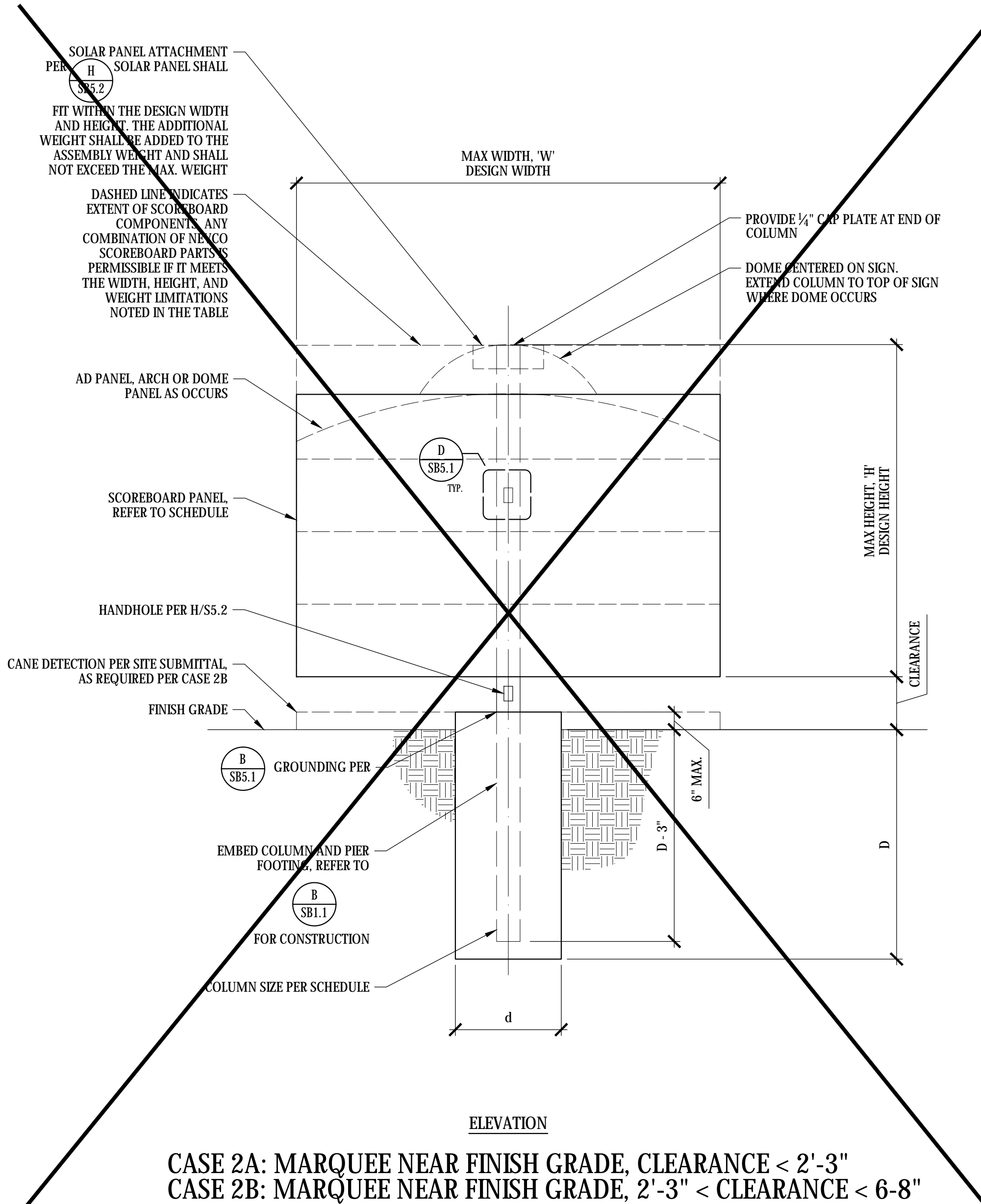
SHEET INFORMATION	
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	0000

ONE COLUMN ASSEMBLY - CASE 1								
ASSEMBLY CRITERIA			PIER FOOTING CRITERIA					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	6'-6"	N/R	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	9'-0"	N/R	N/R

NOTES:
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06

ONE COLUMN ASSEMBLY - CASE 2A ⁽²⁾ AND CASE 2B ⁽²⁾⁽³⁾								
ASSEMBLY CRITERIA			PIER FOOTING CRITERIA					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	5'-3"	N/R	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	6'-6"	N/R	N/R

NOTES:
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06
2. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.
3. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD



ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

N.T.S.

APPLICATION#
02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122089 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5485
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC SEOR REAL

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nevco

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APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CAYLOR R. RUSSELL
No. 52386
EXPIRES 03-31-2025
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC

MARQUEE
CAISSON -
EMBEDDED

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

SSG JOB # S23109

SHEET

SB1.1

ONE COLUMN ASSEMBLY - CASE 1							
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	TRANS. REINF. (1)
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	6'-6"	#4 @ 4 $\frac{1}{2}$ " o.c.
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	9'-0"	#4 @ 4 $\frac{1}{2}$ " o.c.

NOTES: (#)
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

ONE COLUMN ASSEMBLY - CASE 2A ⁽³⁾ AND CASE 2B ⁽³⁾⁽⁴⁾							
ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	TRANS. REINF. (1)
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	5'-3"	#4 @ 4 $\frac{1}{2}$ " o.c.
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30"Ø	6'-6"	#4 @ 4 $\frac{1}{2}$ " o.c.

NOTES: (#)
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION
3. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN
4. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD

ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

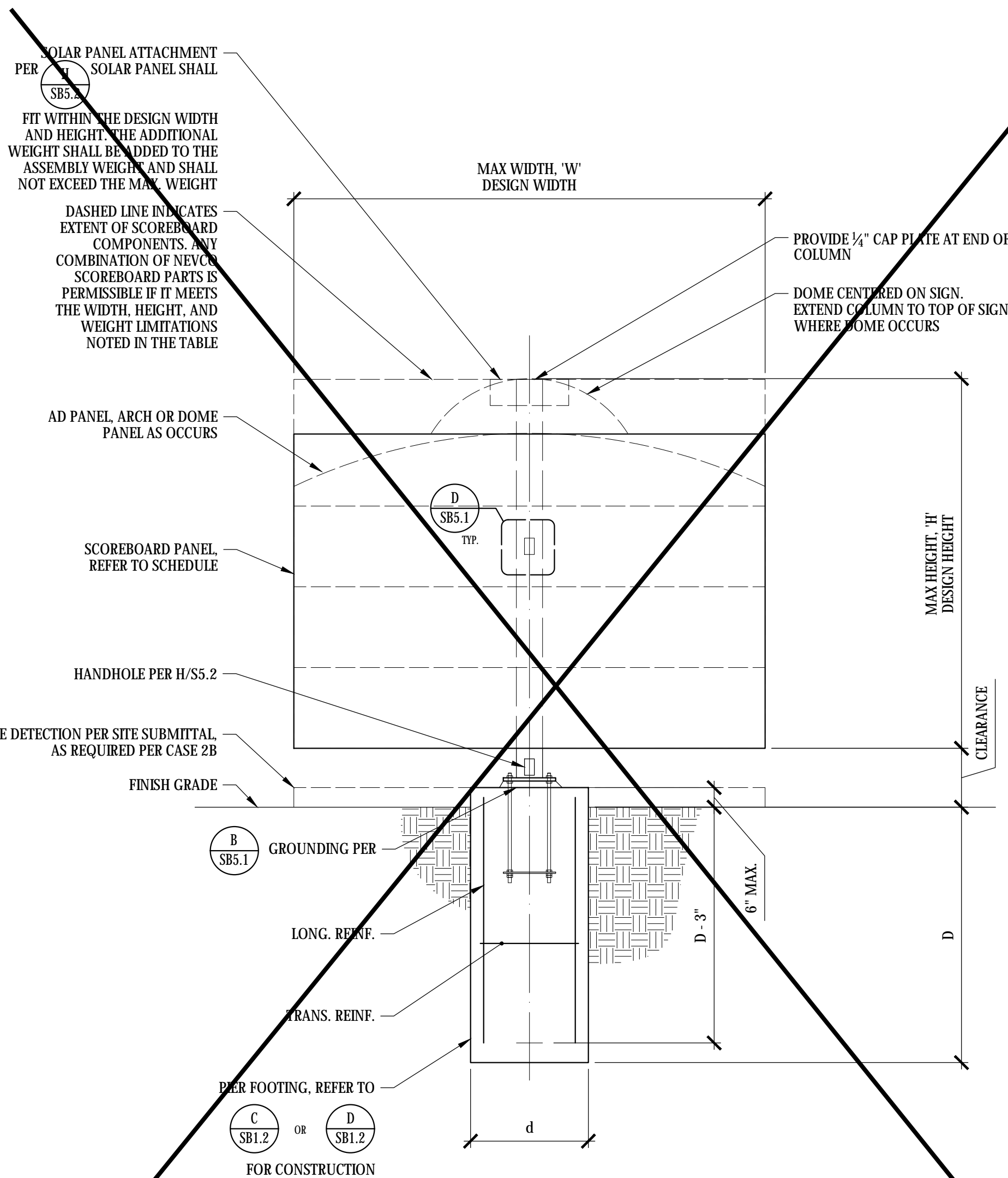
NTS.

COLUMN SIZE	BASE PLATE				ANCHOR ROD			
	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	DEPTH, X	EMBED.
HSS8x8x $\frac{3}{8}$	1"	16"	16"	3/8" FILLET	4-1 $\frac{1}{2}$ "Ø	F1554	2 $\frac{1}{2}$ "	1 $\frac{1}{2}$ "

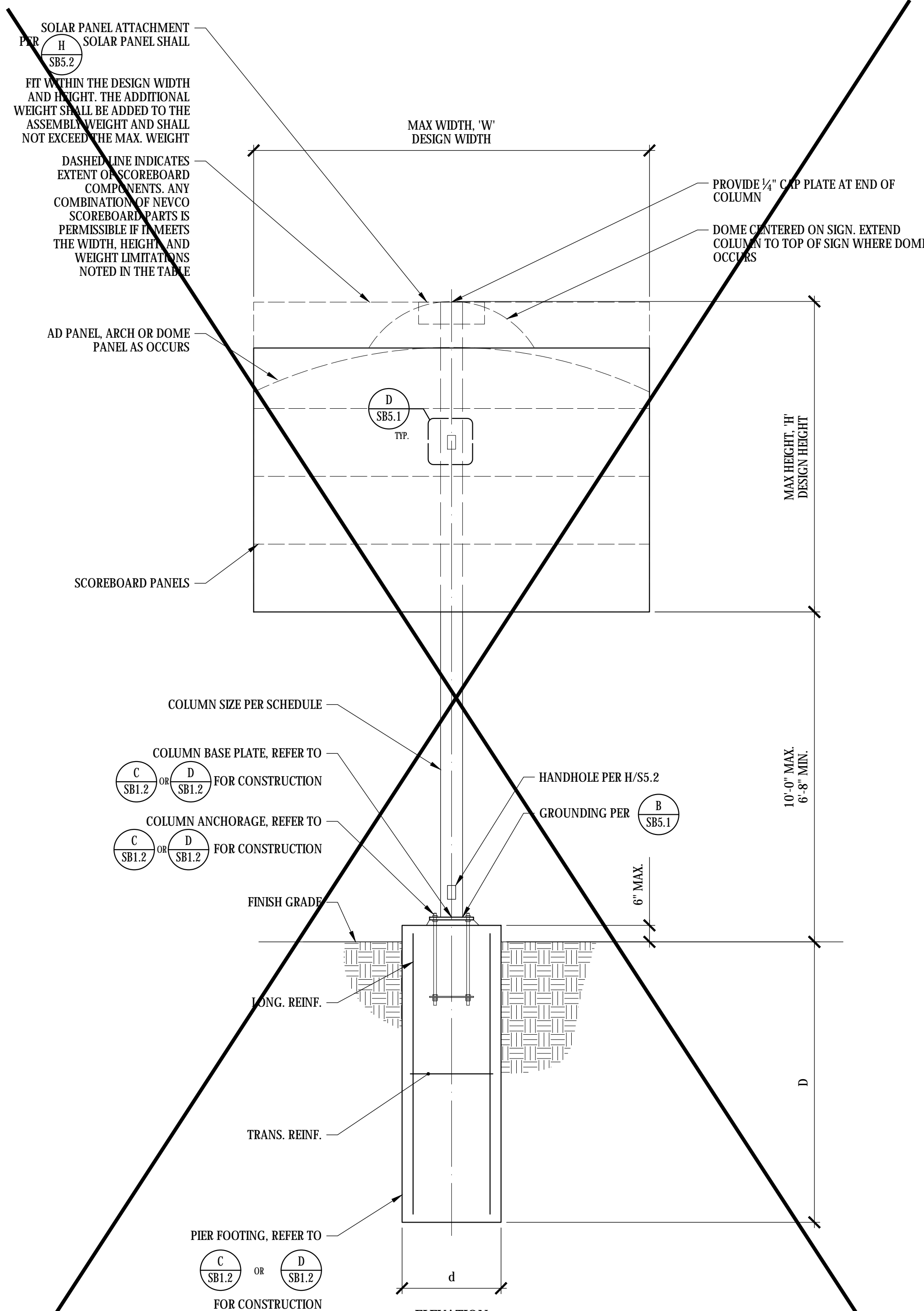
NOTES: (#)
1. SEE SCOREBOARD ELEVATION, A/SB1.2

BASE PLATE

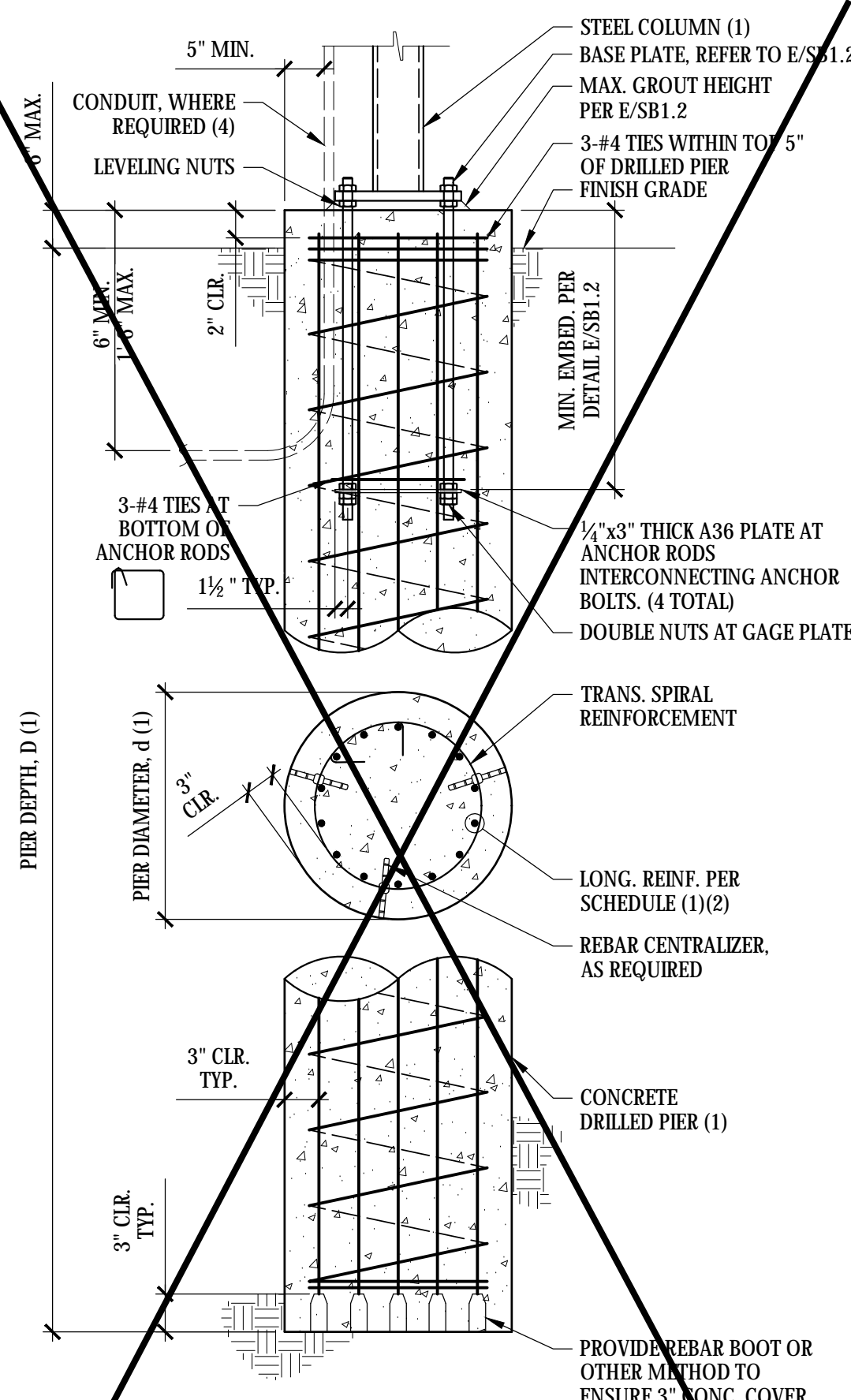
NTS.



CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"
CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3" < CLEARANCE < 6'-8"



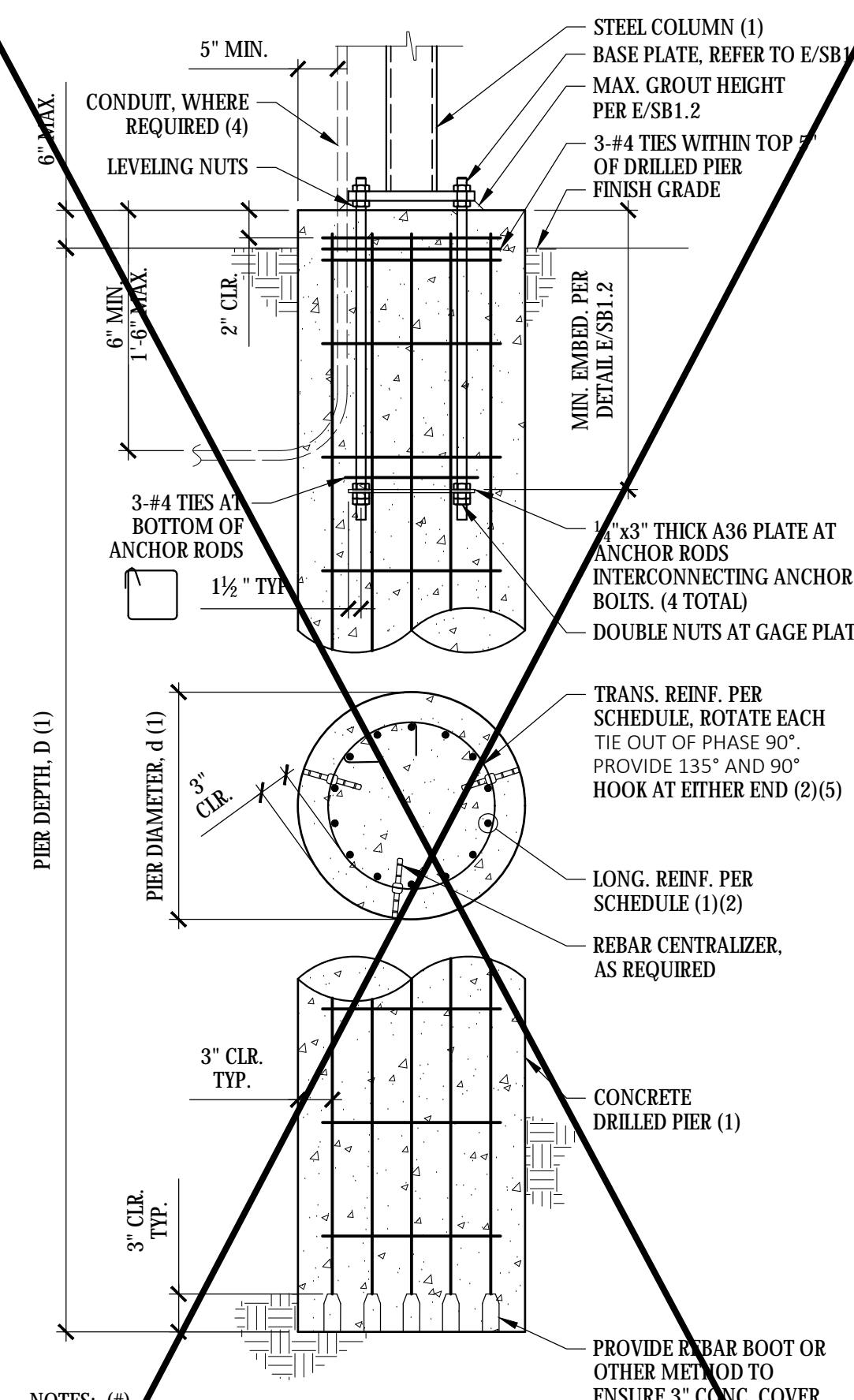
CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE



NOTES: (#)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB1.2
2. SEE DETAILS B/SB1.2 FOR REINFORCEMENT BEND & TIE REQUIREMENTS
3. DO NOT SPLICE REINFORCEMENT
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2)-2" NOMINAL CONDUIT MAX. SPACE CONDUIT Ø < 3" MIN.

CONCRETE DRILLED PIER

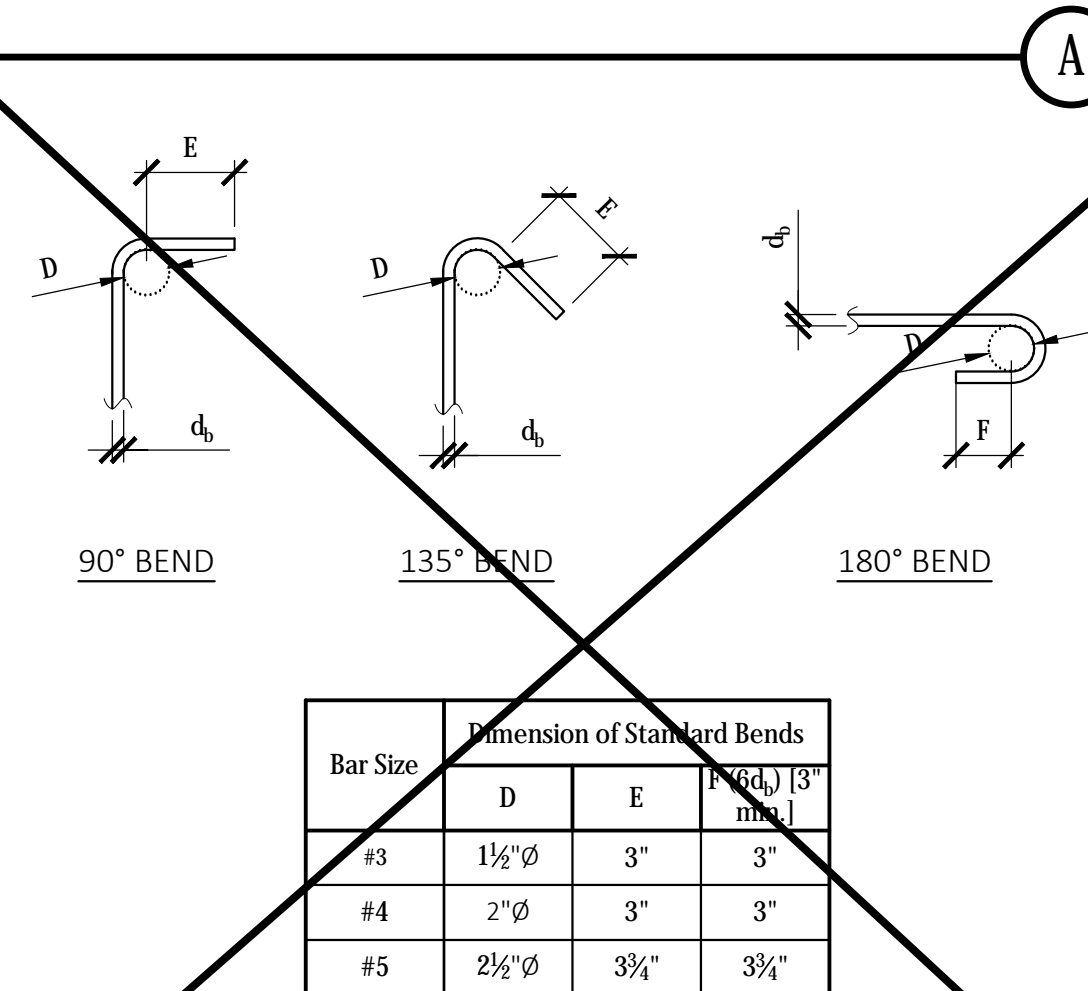
NTS.



NOTES: (#)
1. SEE ELECTRONIC SIGN ELEVATION, A/SB1.2
2. SEE DETAILS B/SB1.2 FOR REINFORCEMENT BEND & TIE REQUIREMENTS
3. DO NOT SPLICE REINFORCEMENT
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2)-2" NOMINAL CONDUIT MAX. SPACE CONDUIT Ø < 3" MIN.
5. WE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT EITHER END (2)(5) OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

CONCRETE DRILLED PIER

NTS.



TIE AND STIRRUP BENDS

NTS.

APPLICATION#
02-122089

IDENTIFICATION STAMP
APP: 02-122089 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5485
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023

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APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122089 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT
CODE: 2022

A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5485
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023

MARQUEE
CAISSON -
BOLTED

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB1.2

ONE COLUMN ASSEMBLY - CASE 2A ⁽²⁾ AND CASE 2B ⁽²⁾⁽³⁾							
ASSEMBLY CRITERIA					MAT FOOTING CRITERIA		
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	WIDTH, A	DEPTH, B	LENGTH, L
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x8	4'-6"	2'-6"	4'-6"
10'-0"		1,355 lbs.	≤ 8'-0"	HSS8x8x8	6'-0"	2'-6"	6'-0"

ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION

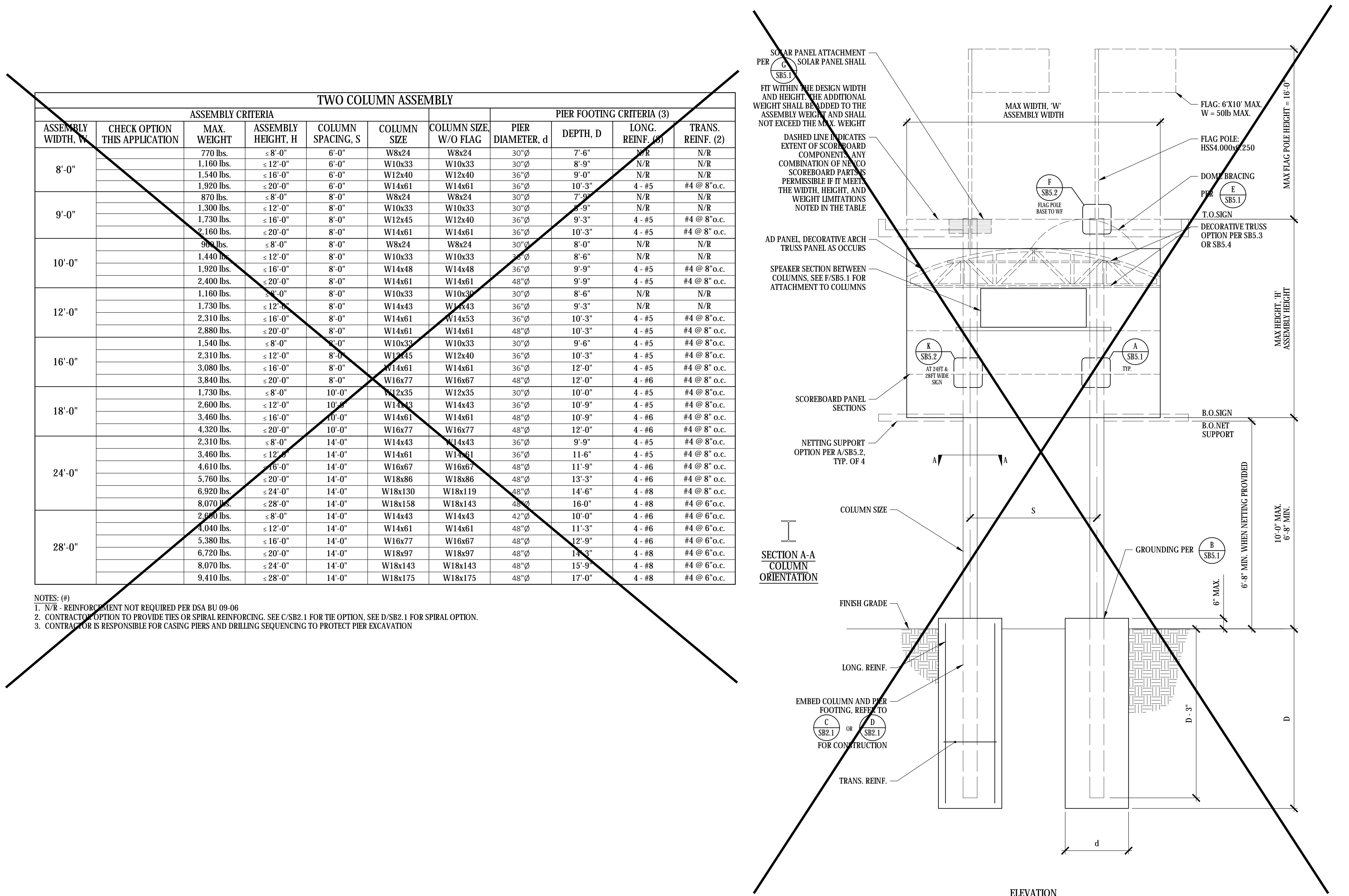


MAT FOOTING CONSTRUCTION AND ANCHORAGE

NOTES: (#)
1. SEE SCOREBOARD ELEVATION, A/SB1.3

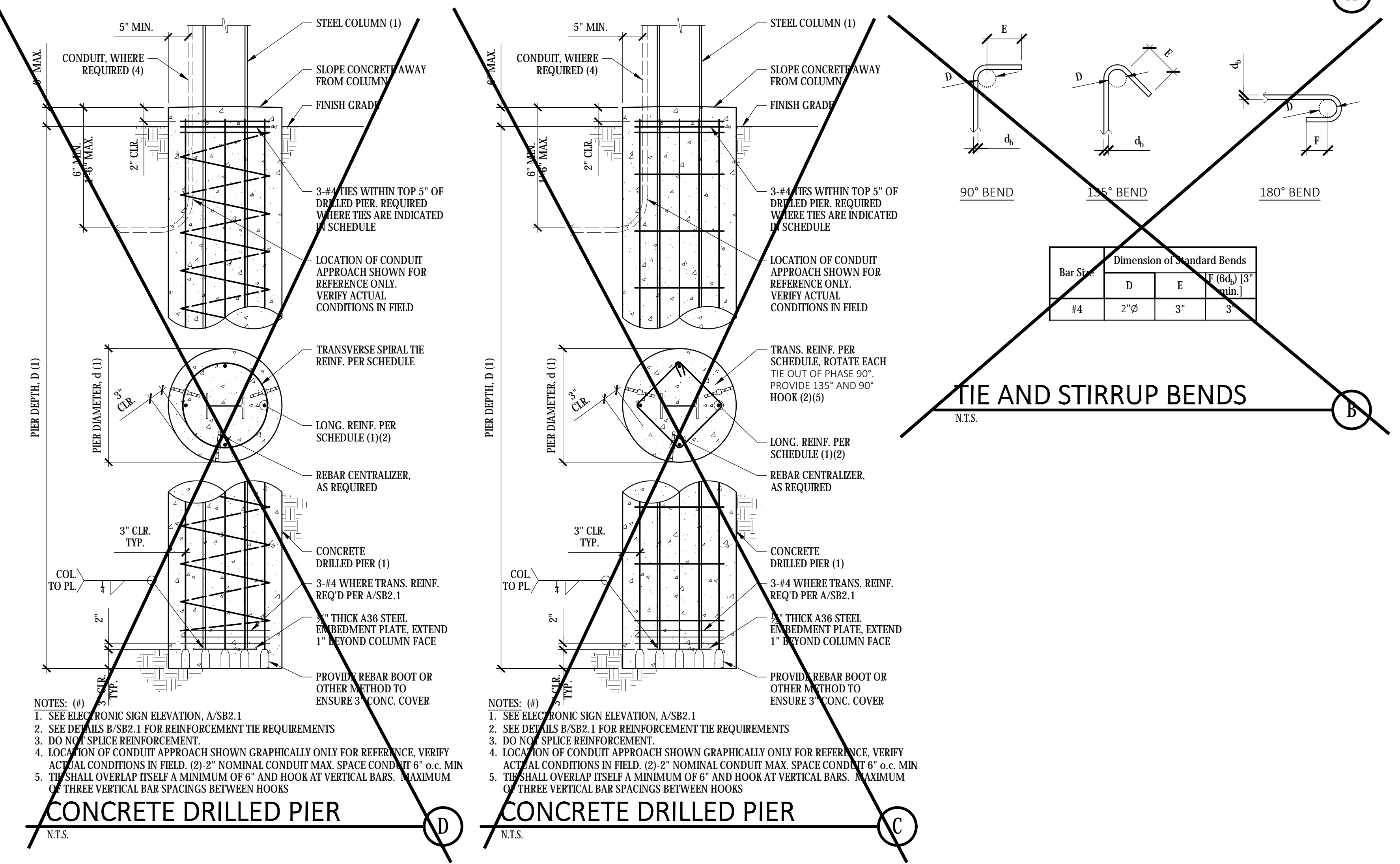


Bar Size	Dimension of Standard Bends		
	D	E	F (6d) [3"] mm
#3	1½"Ø	3"	3"
#4	2"Ø	3"	3"
#5	2½"Ø	3¾"	3¾"



TWO COLUMN SCOREBOARD INSTALLATION

N.T.S.



APPLICATION#
02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122089 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARKIN
No. 5405
STATE OF CALIFORNIA
PC SEOR REAL
08.09.2023

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APPROVED
DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

DSA STAMP

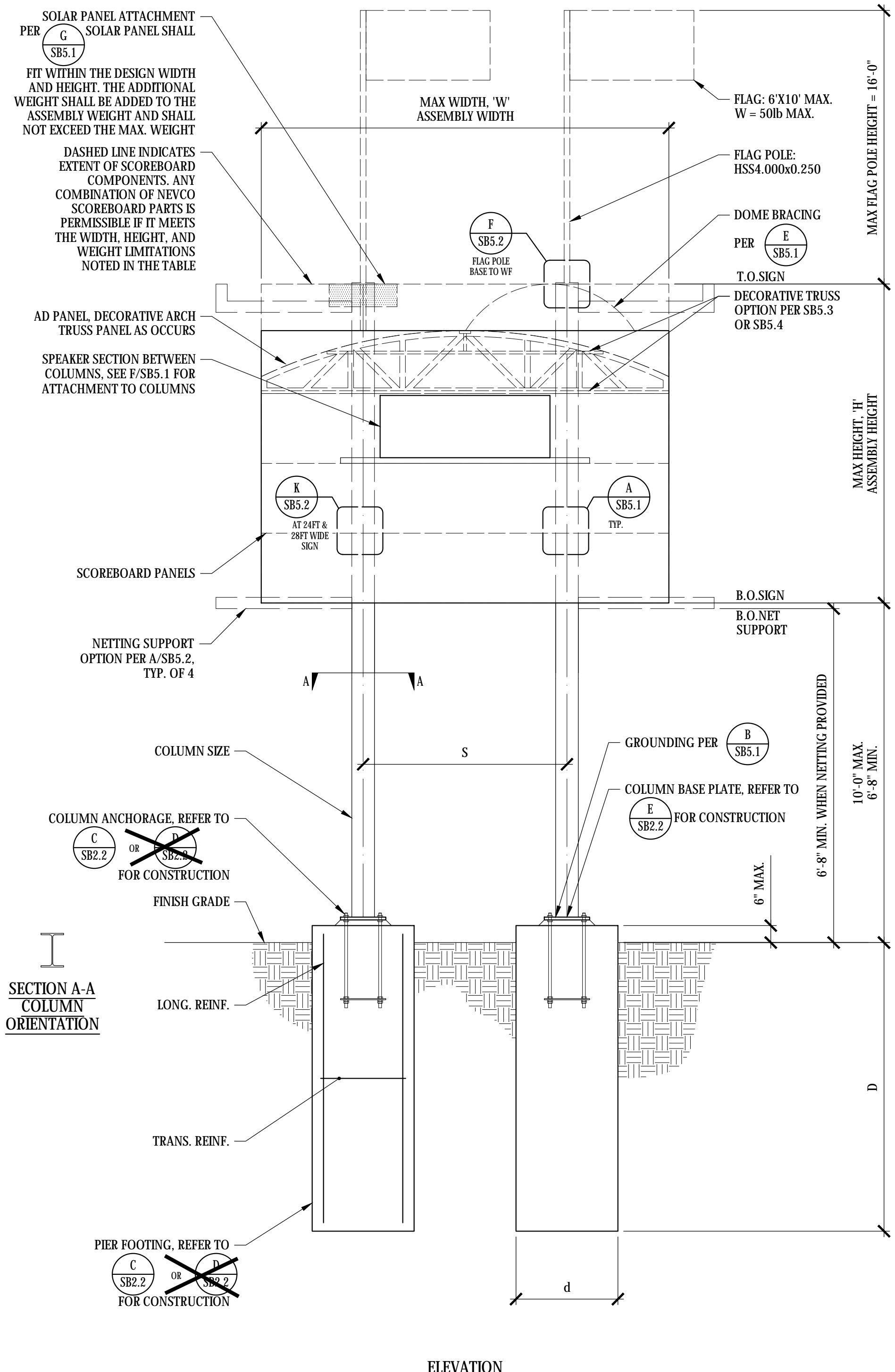
PRE-CHECK (PC) DOCUMENT
CODE: 2022
A separate project application
for construction is required.

TWO COLUMN
CAISSON -
EMBEDDED

SHEET INFORMATION
DATE 08.09.2023
DRAWN JMK
CHECKED MEP
SSG JOB # S23109
SHEET SB2.1

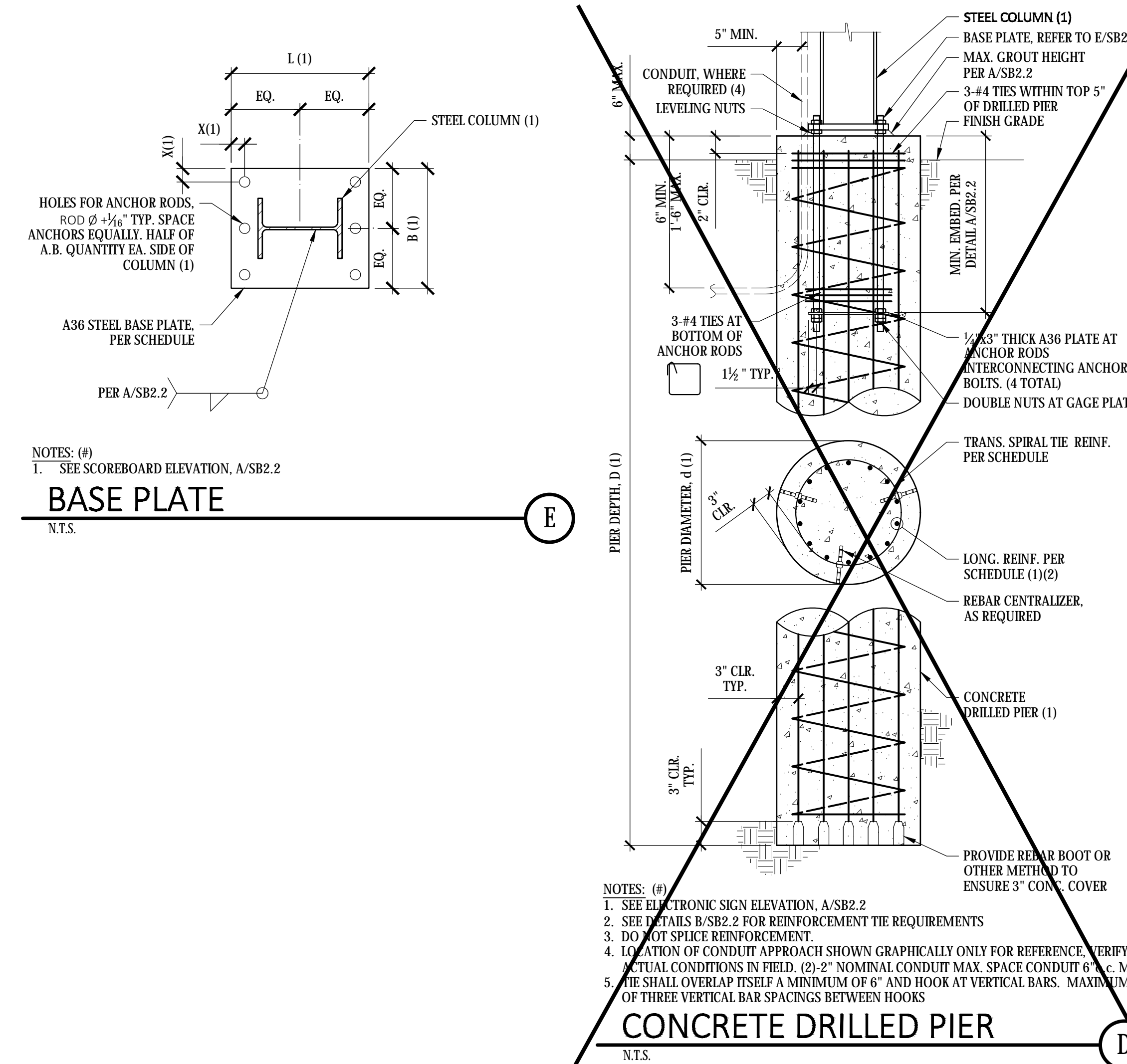
TWO COLUMN ASSEMBLY																			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA				PIER FOOTING CRITERIA (2)				BASE PLATE				QUANTITY & DIAMETER	GRADE	ANCHOR RODS			
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L			WELD	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
8'-0"		720 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	36'-0"	7'-0"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	36'-0"	8'-0"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	36'-0"	9'-0"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		1,540 lbs.	≤ 20'-0"	6'-0"	W14x61	W14x61	42'-0"	9'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
9'-0"		820 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	36'-0"	7'-3"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36'-0"	8'-3"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x45	36'-0"	9'-3"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		1,730 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	42'-0"	10'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
10'-0"		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	36'-0"	7'-6"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36'-0"	8'-6"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,920 lbs.	≤ 16'-0"	8'-0"	W12x48	W12x48	36'-0"	9'-9"	8- #6	#4 @ 4½" o.c.	1"	24"	24"	¾"	(4) - 1½"	F1554 - GR.105	2½"	2"	48"
		1,920 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48'-0"	9'-9"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.105	2½"	2"	48"
12'-0"		1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36'-0"	8'-0"	8- #6	#4 @ 4½" o.c.	1"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x33	W14x33	48'-0"	9'-3"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	48"
		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48'-0"	10'-3"	8- #6	#4 @ 4½" o.c.	1"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		2,310 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48'-0"	10'-3"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
16'-0"		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36'-0"	8'-9"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	48"
		2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	36'-0"	10'-3"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	48"
		2,990 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48'-0"	12'-0"	8- #6	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		2,990 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48'-0"	12'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
18'-0"		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	36'-0"	9'-0"	8- #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.36	2½"	2"	48"
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x38	W14x38	48'-0"	10'-0"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
		3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	48'-0"	10'-9"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		3,990 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x77	48'-0"	13'-0"	12- #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(6) - 1½"	F1554 - GR.55	3"	2"	64"
24'-0"		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	36'-0"	9'-9"	8- #6	#4 @ 4½" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		3,460 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	36'-0"	11'-9"	8- #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"	F1554 - GR.55	2½"	2"	64"
		4,610 lbs.	≤ 16'-0"	14'-0"	W16x67	W16x67	36'-0"	11'-9"	12- #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
		5,760 lbs.	≤ 20'-0"	14'-0"	W18x86	W18x86	48'-0"	12'-9"	12- #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(6) - 1½"	F1554 - GR.55	3"	2"	64"
28'-0"		6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	W18x119	48'-0"	14'-6"	12- #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"	F1554 - GR.105	3"	2"	64"
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	54'-0"	16'-0"	12- #8	#4 @ 6" o.c.	2½"	24"	36"	CJP	(6) - 2"	F1554 - GR.105	3"	2"	64"
		2,690 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	42'-0"	10'-0"	8- #7	#4 @ 4½" o.c.	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	64"
		4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	48'-0"	11'-3"	8- #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.55	3"	2"	64"
28'-0"		5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x67	48'-0"	12'-0"	12- #8	#4 @ 6" o.c.	2"	24"	30"	½"	(6) - 1½"	F1554 - GR.55	3"	2"	64"
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	48'-0"	14'-3"	12- #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"	F1554 - GR.105	3"	2"	64"
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	54'-0"	15'-9"	12- #8	#4 @ 6" o.c.	2½"	24"	36"	CJP	(6) - 2"	F1554 - GR.105	4"	2½"	64"
		9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	54'-0"	16'-9"	12- #8	#4 @ 6" o.c.	3"	24"	36"	CJP	(6) - 2"	F1554 - GR.105	4"	2½"	64"

NOTES: (4)
1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SR2.2 FOR TIE OPTION. SEE D/SR2.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.

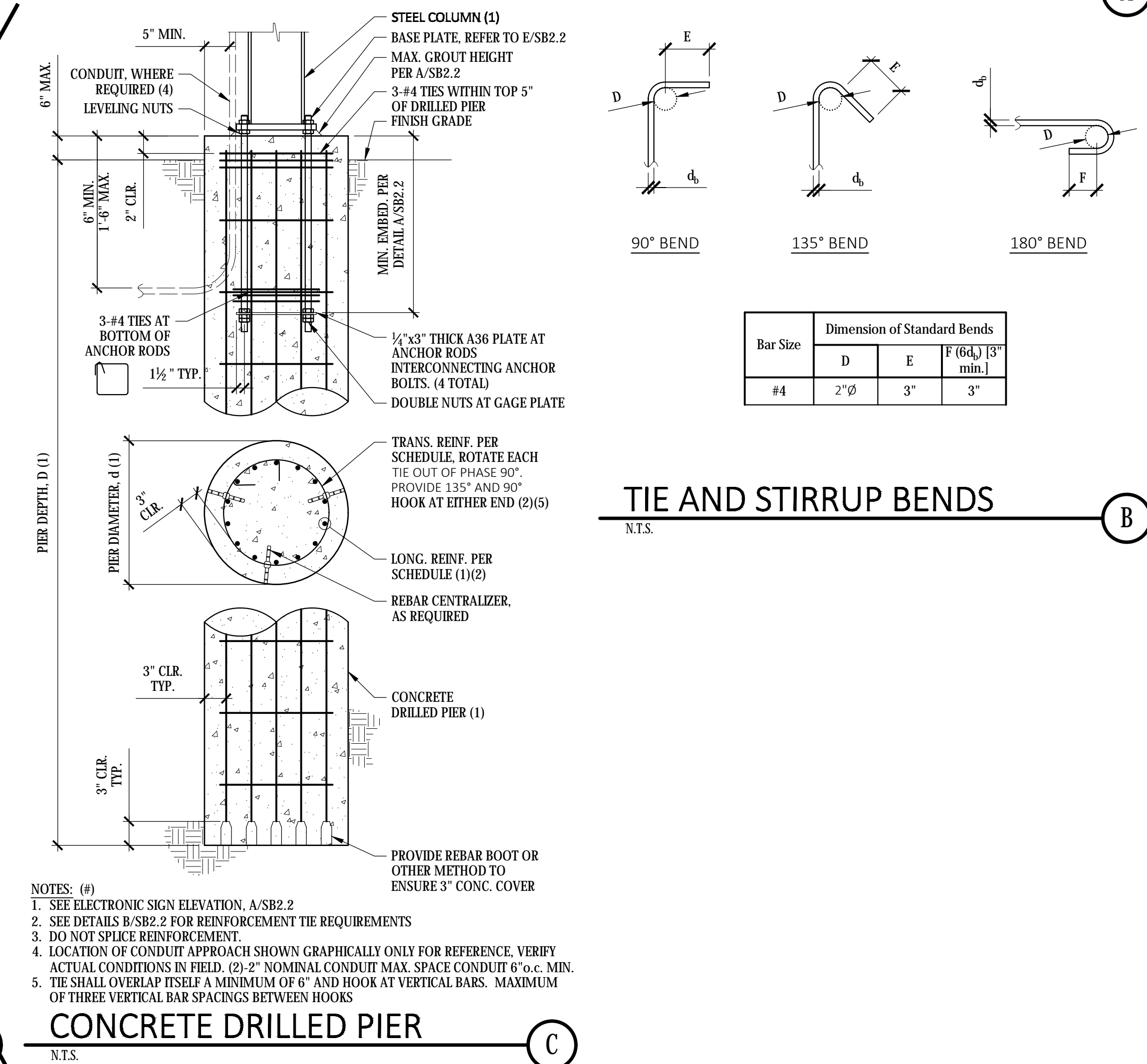


TWO COLUMN SCOREBOARD INSTALLATION

NTS



NOTES: (4)
1. SEE ELEVATION SIGN ELEVATION, A/SR2.2
2. SEE DETAILS B/SR2.2 FOR REINFORCEMENT TIE REQUIREMENTS
3. DO NOT SPICE REINFORCEMENT.
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.
5. THE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS



APPLICATION# 02-122089

IDENTIFICATION STAMP

APP: 02-122089 INC.

REVIEWED FOR

SS

FLS

ACS

DATE: 4/3/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER

Michael E. Parvizi

No. 5405

STATE OF CALIFORNIA

PC SEOR SEAL

08.09.2023

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nevco

301 East Harris Avenue, Greenville, Illinois 62246

Phone: (618) 664-0960

www.nevco.com

APPROVED

DIV. OF THE STATE ARCHITECT

APP: 04-122089 PC

REVIEWED FOR

SS

FLS

ACS

CG

DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT

CODE: 2022

A separate project application for construction is required.

TWO COLUMN CAISSON - BOLTED

DATE: 08.09.2023

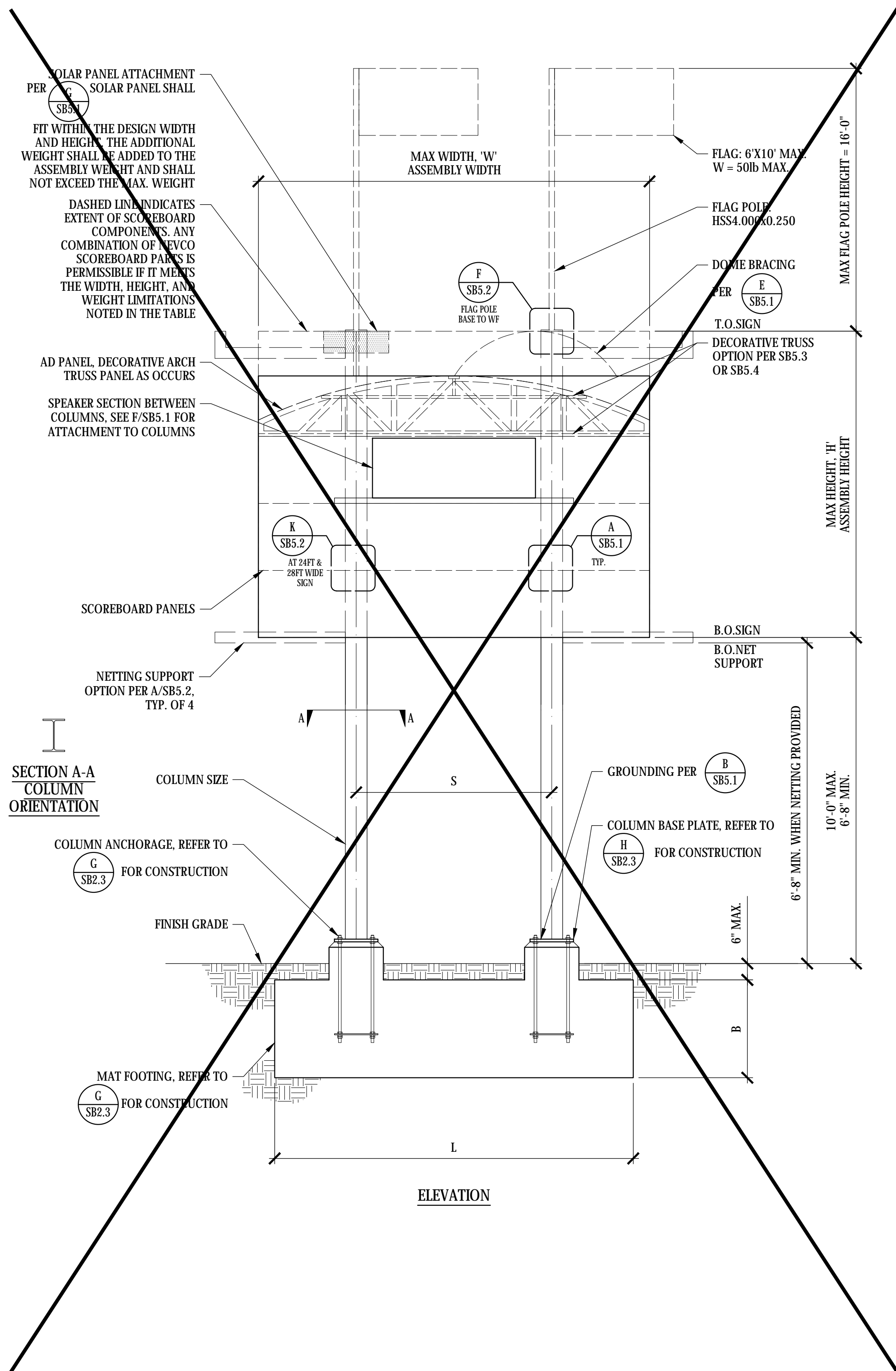
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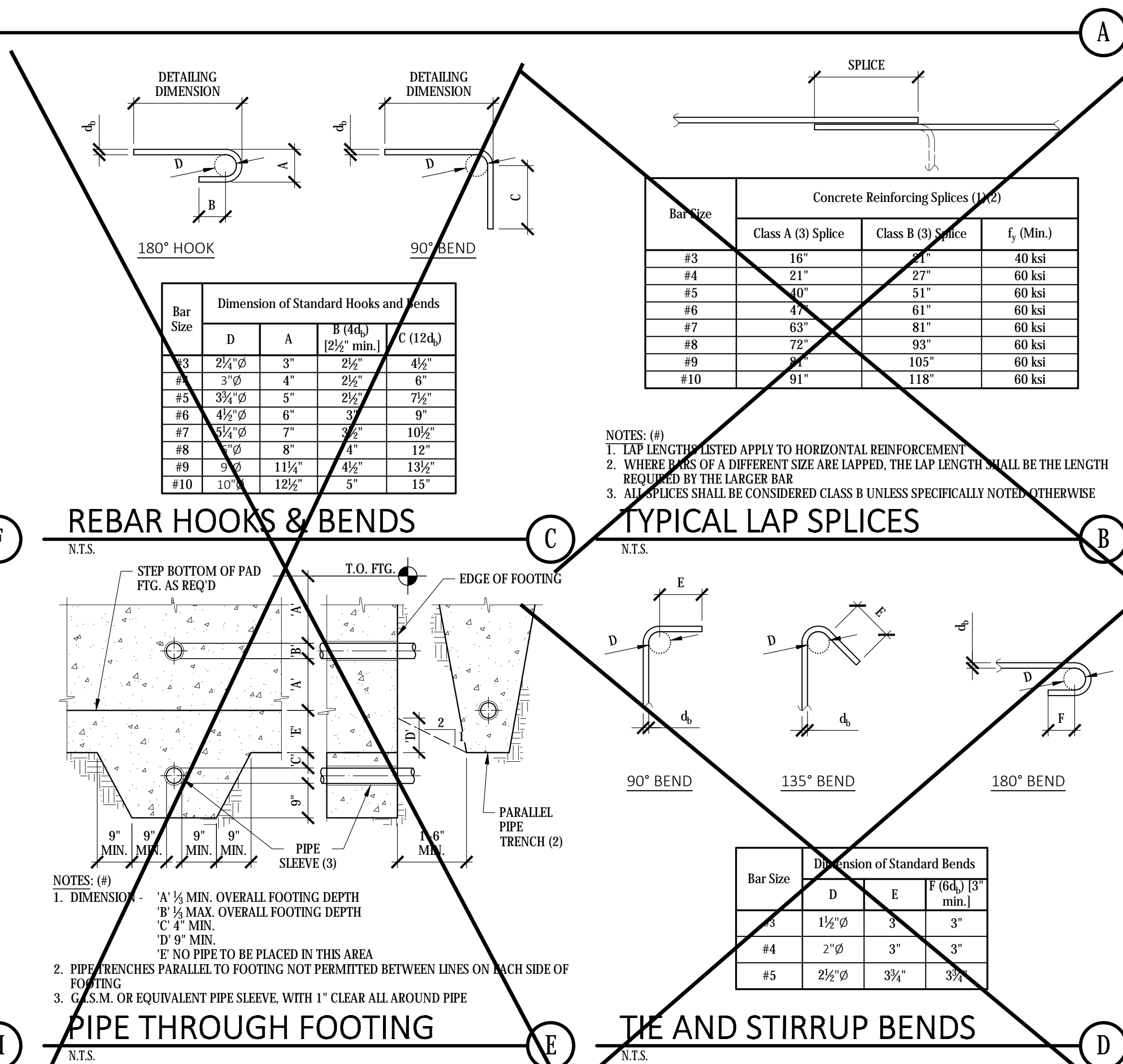
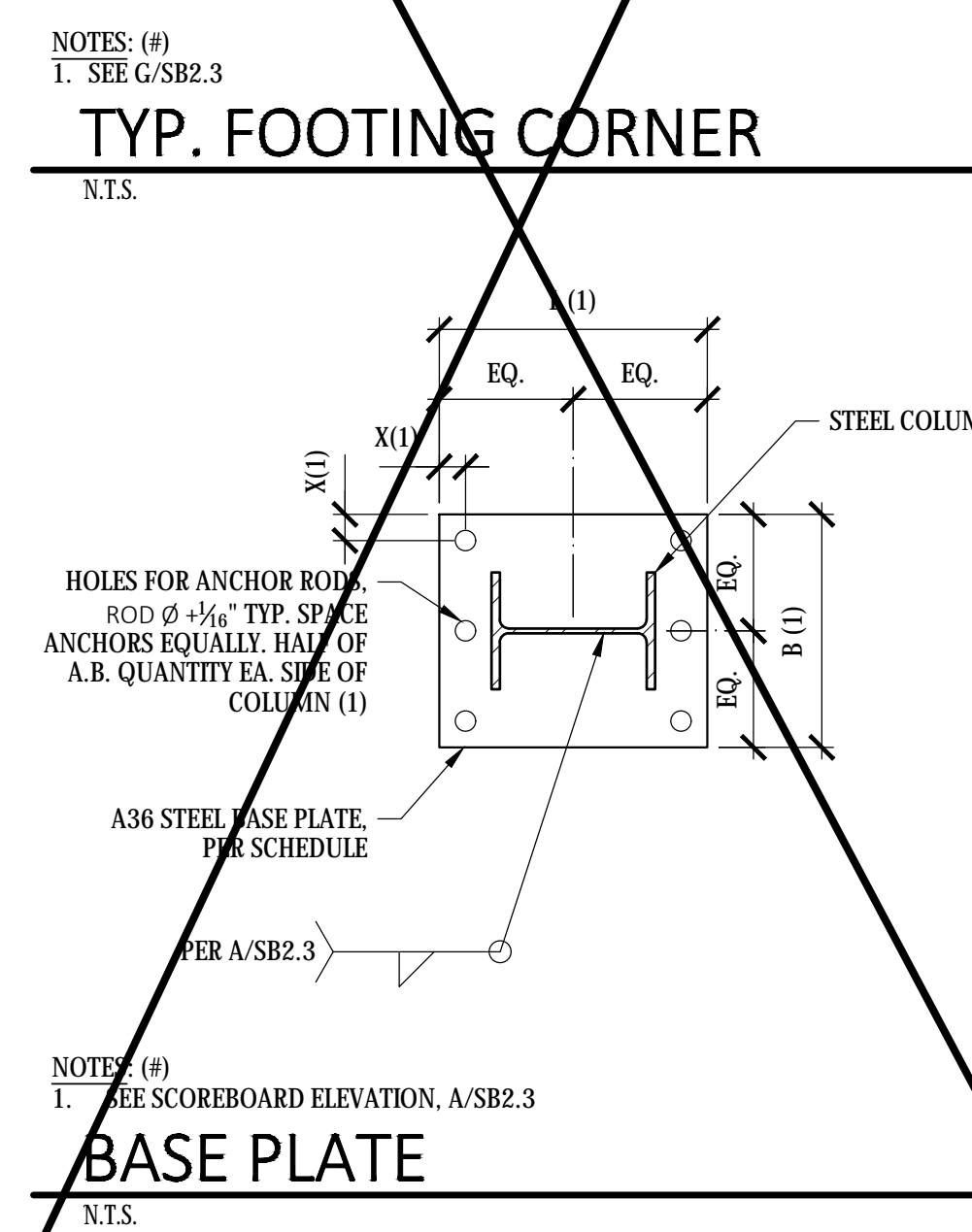
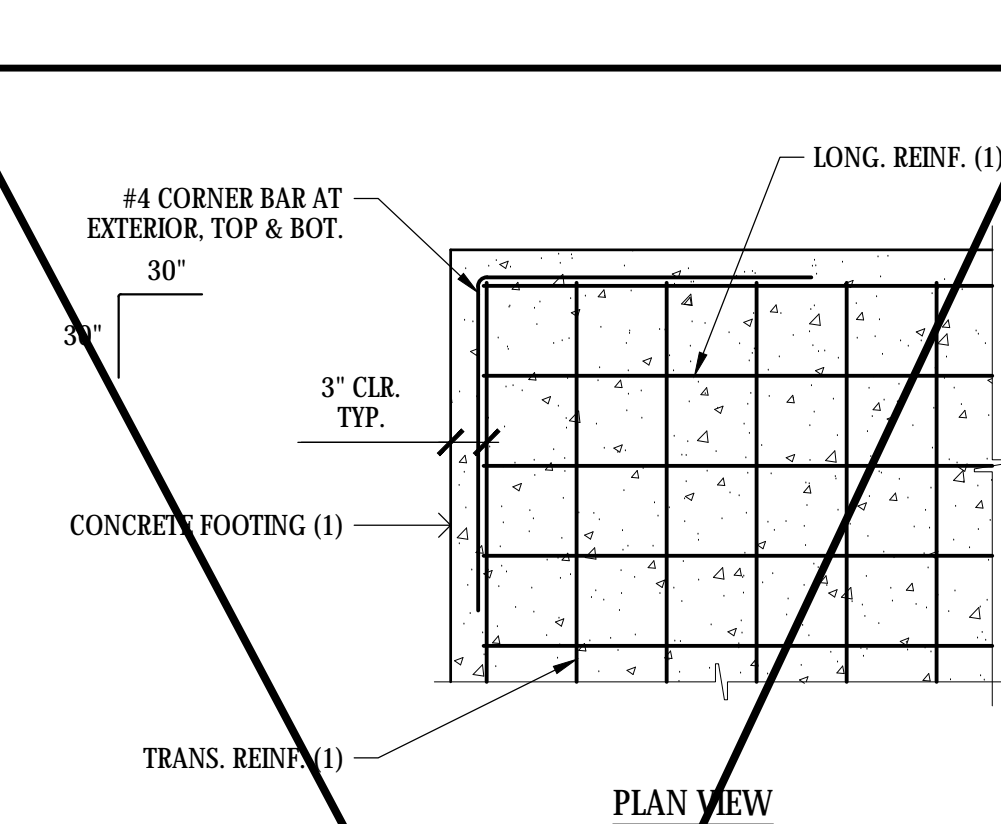
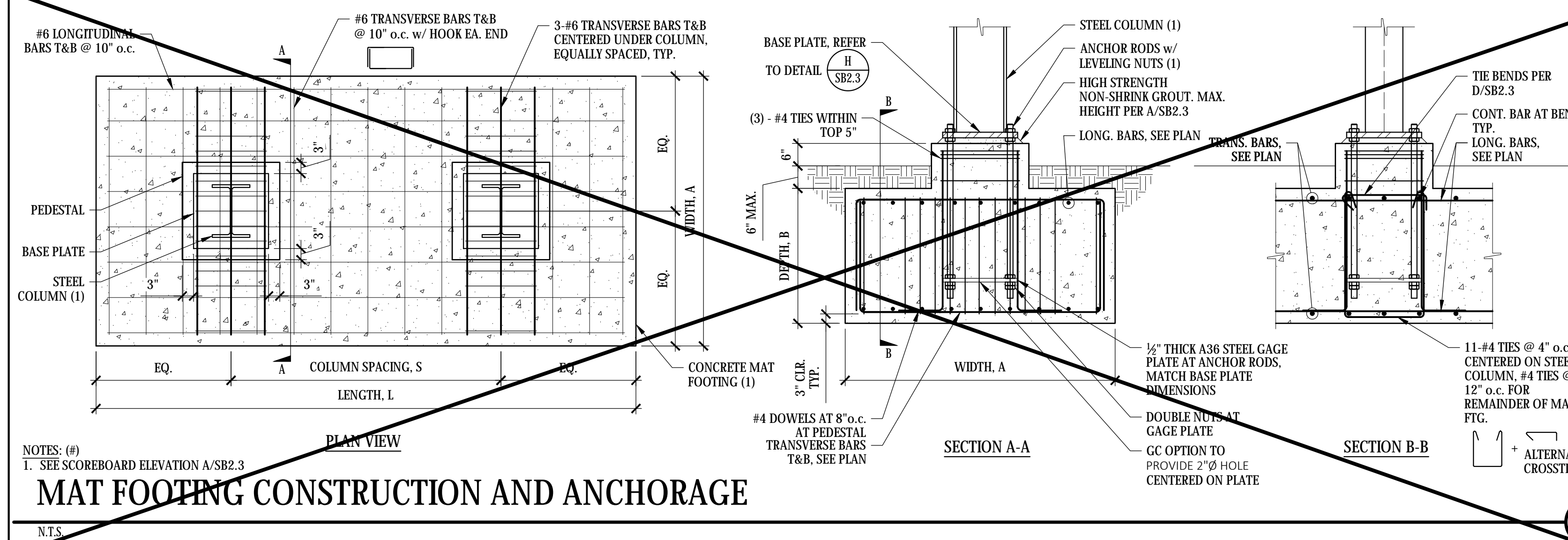
SSG JOB #: S23109

SHEET: SB2.2

TWO COLUMN ASSEMBLY																			
ASSEMBLY CRITERIA							MAT FOOTING CRITERIA				BASE PLATE				ANCHOR RODS				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED	
8'-0"		770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	7'-0"	3'-0"	10'-0"	1"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	8'-0"	3'-0"	12'-0"	1 1/8"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	9'-0"	3'-0"	13'-0"	1 1/8"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		1,920 lbs.	≤ 20'-0"	6'-0"	W14x61	W14x61	11'-0"	3'-0"	14'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		870 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	7'-0"	3'-0"	11'-0"	1"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
9'-0"		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	13'-0"	1 1/8"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x40	9'-0"	3'-0"	15'-0"	1 1/8"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		2,160 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	10'-0"	3'-0"	16'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	7'-0"	3'-0"	11'-0"	1"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	13'-0"	1 1/8"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
10'-0"		1,920 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	9'-0"	3'-0"	14'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.105	2 1/4"	2"	30"	
		2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	11'-0"	3'-0"	16'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.105	2 1/4"	2"	30"	
		1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	7'-0"	3'-0"	12'-0"	1"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	W14x43	9'-0"	3'-0"	12'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	10'-0"	3'-0"	14'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
12'-0"		2,880 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	11'-0"	3'-0"	17'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	7'-0"	3'-0"	13'-0"	1"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	9'-0"	3'-0"	15'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-0"	3'-0"	17'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	11'-0"	3'-0"	20'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.105	2 1/4"	2"	30"	
16'-0"		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	8'-0"	3'-0"	13'-0"	1"	20"	20"	3/8"	(4) 1/4"Ø	F1554 - GR.36	2 1/4"	2"	30"	
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	9'-0"	3'-0"	15'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	11'-0"	3'-0"	16'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		4,320 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x77	13'-0"	3'-0"	17'-0"	1 1/8"	24"	30"	3/8"	(6) 1/4"Ø	F1554 - GR.55	3"	2"	30"	
		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	8'-0"	3'-0"	17'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
24'-0"		3,460 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	9'-0"	3'-0"	19'-0"	1 1/8"	24"	24"	3/8"	(6) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		4,610 lbs.	≤ 16'-0"	14'-0"	W16x67	W16x67	11'-0"	3'-0"	20'-0"	1 1/8"	24"	30"	3/8"	(4) 1/4"Ø	F1554 - GR.55	3"	2"	30"	
		5,760 lbs.	≤ 20'-0"	14'-0"	W18x86	W18x86	13'-0"	3'-0"	21'-0"	1 1/8"	24"	30"	3/8"	(6) 1/4"Ø	F1554 - GR.55	3"	2"	30"	
		6,920 lbs.	≤ 24'-0"	14'-0"	W18x130	W18x119	14'-0"	4'-0"	22'-0"	2"	24"	30"	CIP	(6) 1/4"Ø	F1554 - GR.105	3"	2"	36"	
		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	15'-0"	4'-0"	23'-0"	2 1/8"	24"	36"	CIP	(6) 1/4"Ø	F1554 - GR.105	4"	2 1/4"	36"	
28'-0"		2,400 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	8'-0"	3'-0"	18'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.55	2 1/4"	2"	30"	
		4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	10'-0"	3'-0"	19'-0"	1 1/8"	24"	24"	3/8"	(4) 1/4"Ø	F1554 - GR.55	3"	2"	30"	
		5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x67	11'-0"	3'-0"	22'-0"	2"	24"	30"	CIP	(6) 1/4"Ø	F1554 - GR.55	3"	2"	30"	
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	13'-0"	3'-0"	23'-0"	2"	24"	30"	1/2"	(6) 1/4"Ø	F1554 - GR.105	3"	2"	30"	
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	14'-0"	4'-0"	24'-0"	2 1/8"	24"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/4"	36"	
	9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	15'-0"	4'-0"	24'-0"	3"	24"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/4"	36"		



N.T.S.



Bar Size	Concrete Reinforcing Splices (1)(2)			f_u (Min.)
	Class A (3) Splice	Class B (3) Splice		
#3	16"	21"		40 ksi
#4	21"	27"		60 ksi
#5	40"	51"		60 ksi
#6	47"	61"		60 ksi
#7	63"	81"		60 ksi
#8	72"	93"		60 ksi
#9	81"	105"		60 ksi
#10	91"	118"		60 ksi

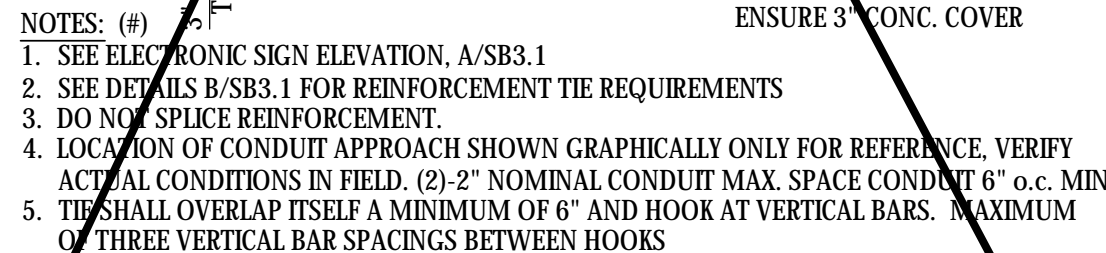
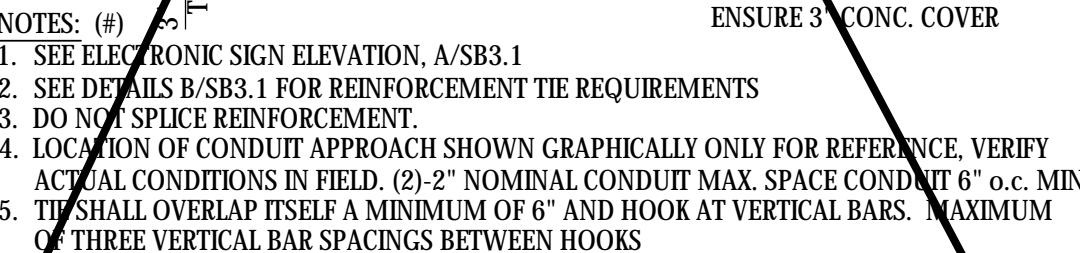
Bar Size	Dimension of Standard Bends		
	D	E	F (6d.) [3 min.]
#3	1½"Ø	3"	3"
#4	2"Ø	3"	3"
#5	2½"Ø	3¾"	3¾"

NOTES:

1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06
2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB3.1 FOR TIE OPTION, SEE D/SB3.1 FOR SPIRAL OPTION
3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



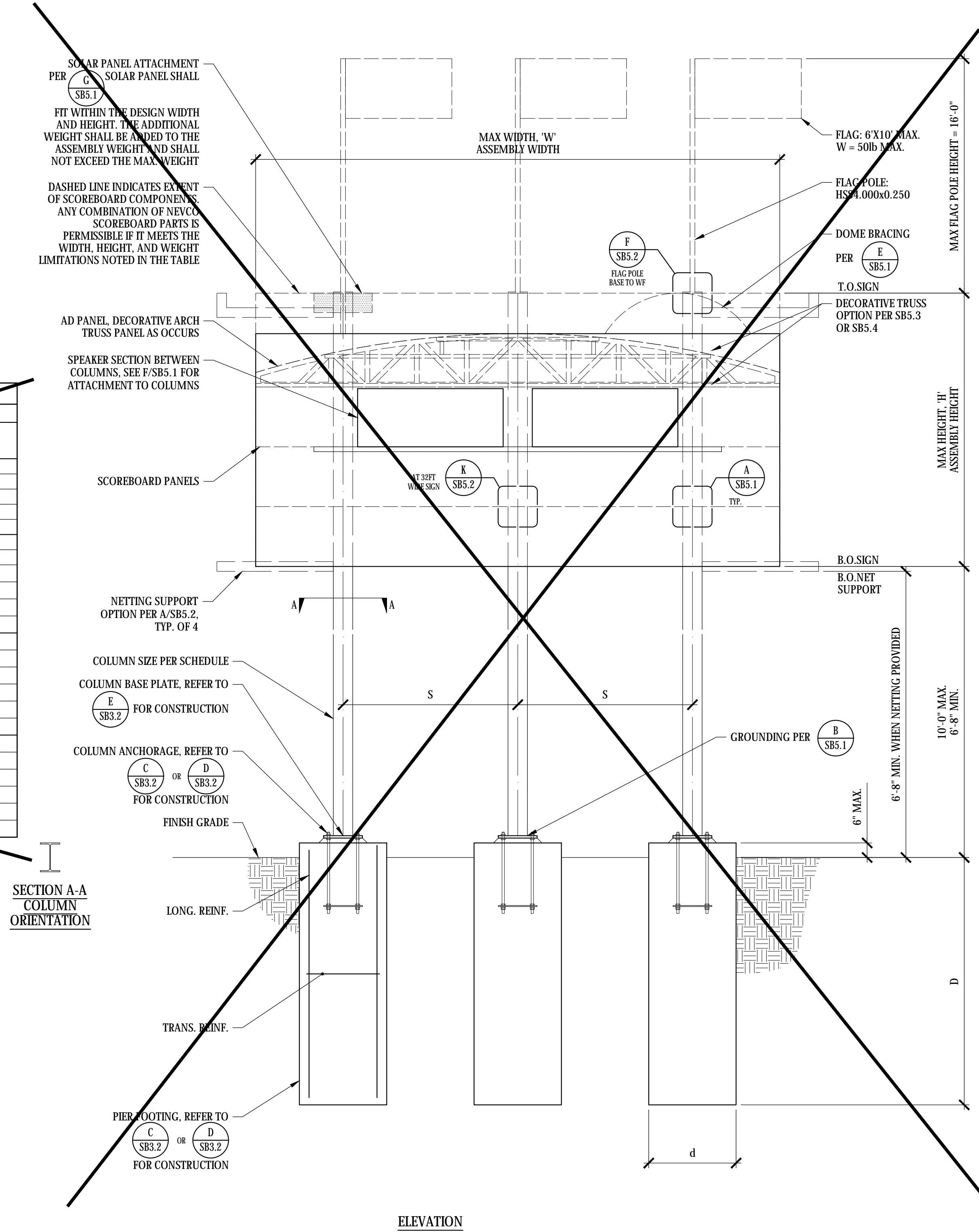
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TIE AND STIRRUP BENDS

THREE COLUMN ASSEMBLY																	
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER FOOTING CRITERIA (2)			BASE PLATE			ANCHOR RODS				
		MAX. WEIGHT	ASSEMBLY HEIGHT, H				PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, A
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.36	2½"
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	36"Ø	10'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	42"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"
		4,800 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	11'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(6) - 1½"Ø	F1554 - GR.55	3"
24'-0"		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	48"Ø	12'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(6) - 1½"Ø	F1554 - GR.55	3"
		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"
		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	42"Ø	10'-6"	8 - #8	#4 @ 6" o.c.	1½"	20"	20"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-9"	12 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"
28'-0"		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(6) - 1½"Ø	F1554 - GR.105	2½"
		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	48"Ø	13'-3"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	48"Ø	14'-3"	14 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"
32'-0"		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	48"Ø	9'-9"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-6"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	½"	(4) - 1½"Ø	F1554 - GR.55	3"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-0"	14 - #8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		8,070 lbs.	≤ 24'-0"	10'-0"	W18x106	W18x97	48"Ø	13'-9"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
32'-0"		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	48"Ø	15'-0"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.105	3"
		3,080 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-0"	8 - #6	#4 @ 4½" o.c.	1½"	20"	20"	¾"	(4) - 1½"Ø	F1554 - GR.55	2½"
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-3"	8 - #8	#4 @ 6" o.c.	1½"	24"	24"	¾"	(6) - 1½"Ø	F1554 - GR.55	2½"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	11'-9"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	¾"	(4) - 1½"Ø	F1554 - GR.55	3"
32'-0"		7,680 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	48"Ø	13'-0"	14 - #8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
		9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	48"Ø	14'-6"	14 - #9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"
32'-0"		10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	54"Ø	15'-6"	14 - #9	#4 @ 6" o.c.	2½"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.35	4"

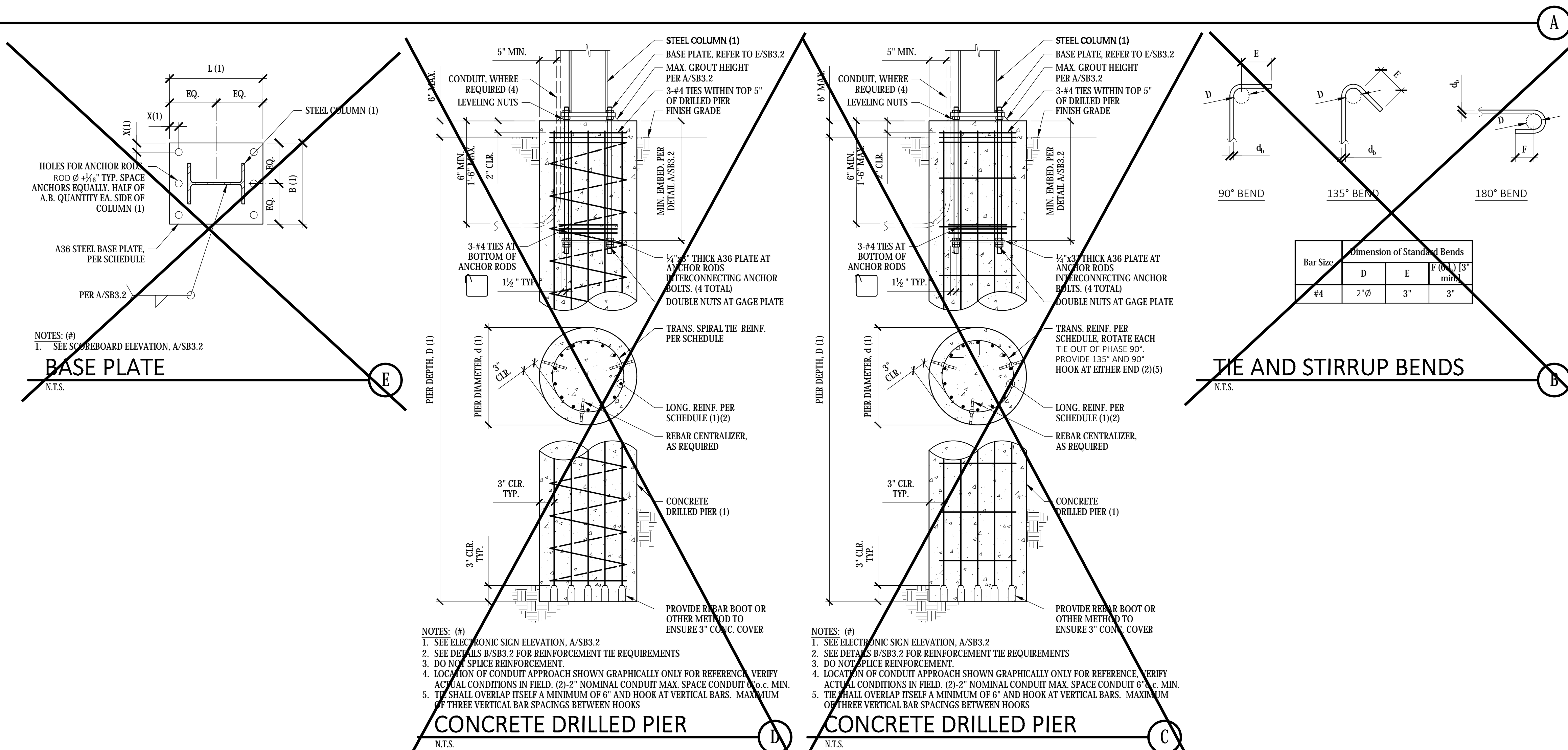
NOTES:
1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB3.2 FOR THE OPTION, SEE D/SB3.2 FOR SPIRAL OPTION.
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



ELEVATION

THREE COLUMN SCOREBOARD INSTALLATION

N.T.S.



CONCRETE DRILLED PIER

N.T.S.

CONCRETE DRILLED PIER

N.T.S.

APPLICATION#
02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122089 INC:
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARSONS
No. 5405
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC SEOR SEAL

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THANK YOU FOR YOUR INTEREST IN SELECT SCOREBOARD PRODUCTS

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APP: 04-122377 PC
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

DSA STAMP

PRE-CHECK (PC) DOCUMENT
CODE: 2022

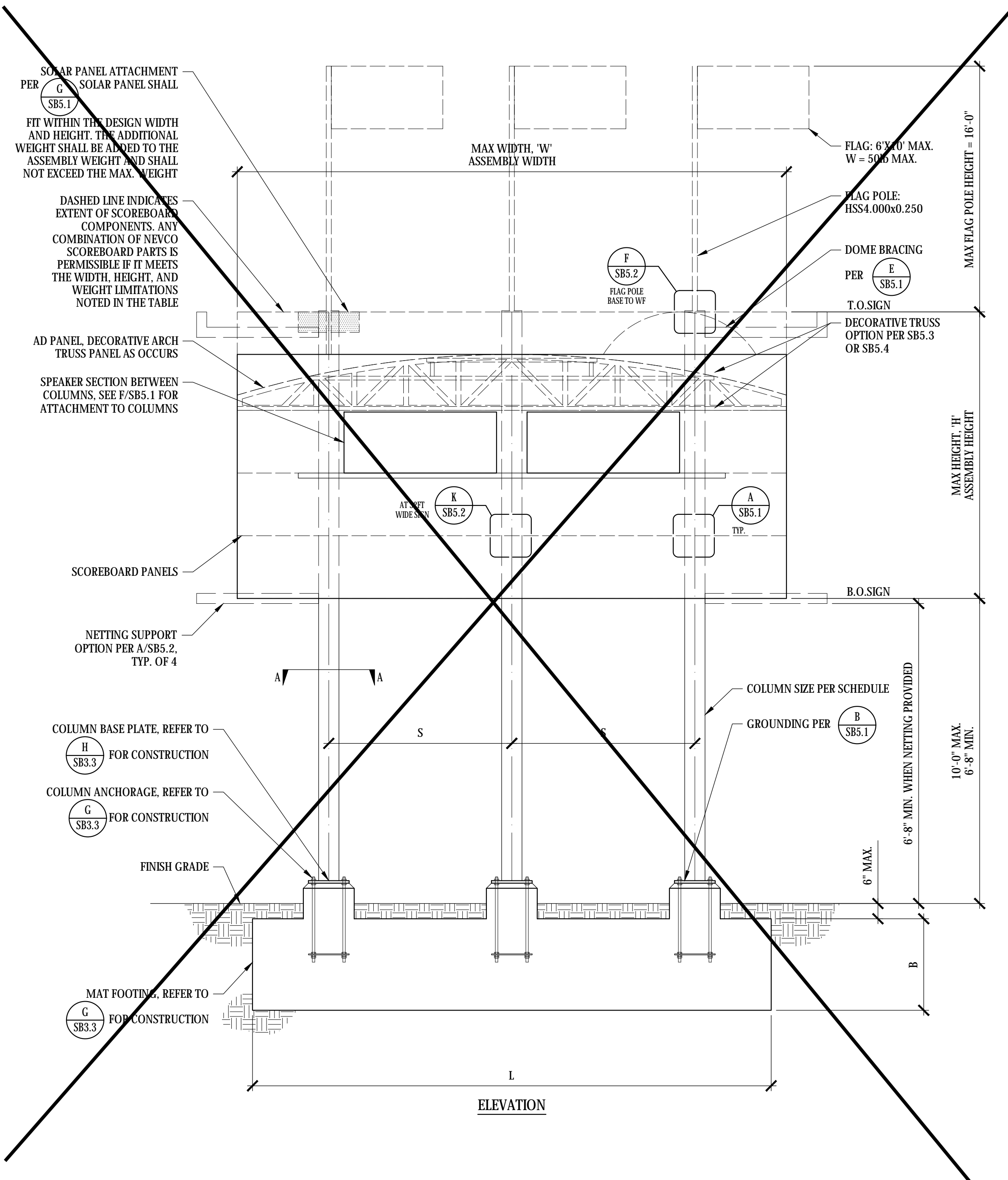
A separate project application
for construction is required.

REGISTERED PROFESSIONAL ENGINEER
CALIFORNIA
NO. 52386
EXPIRES
12-31-2024
MICHAEL E. PARSONS
DATE SIGNED: 08.09.2023

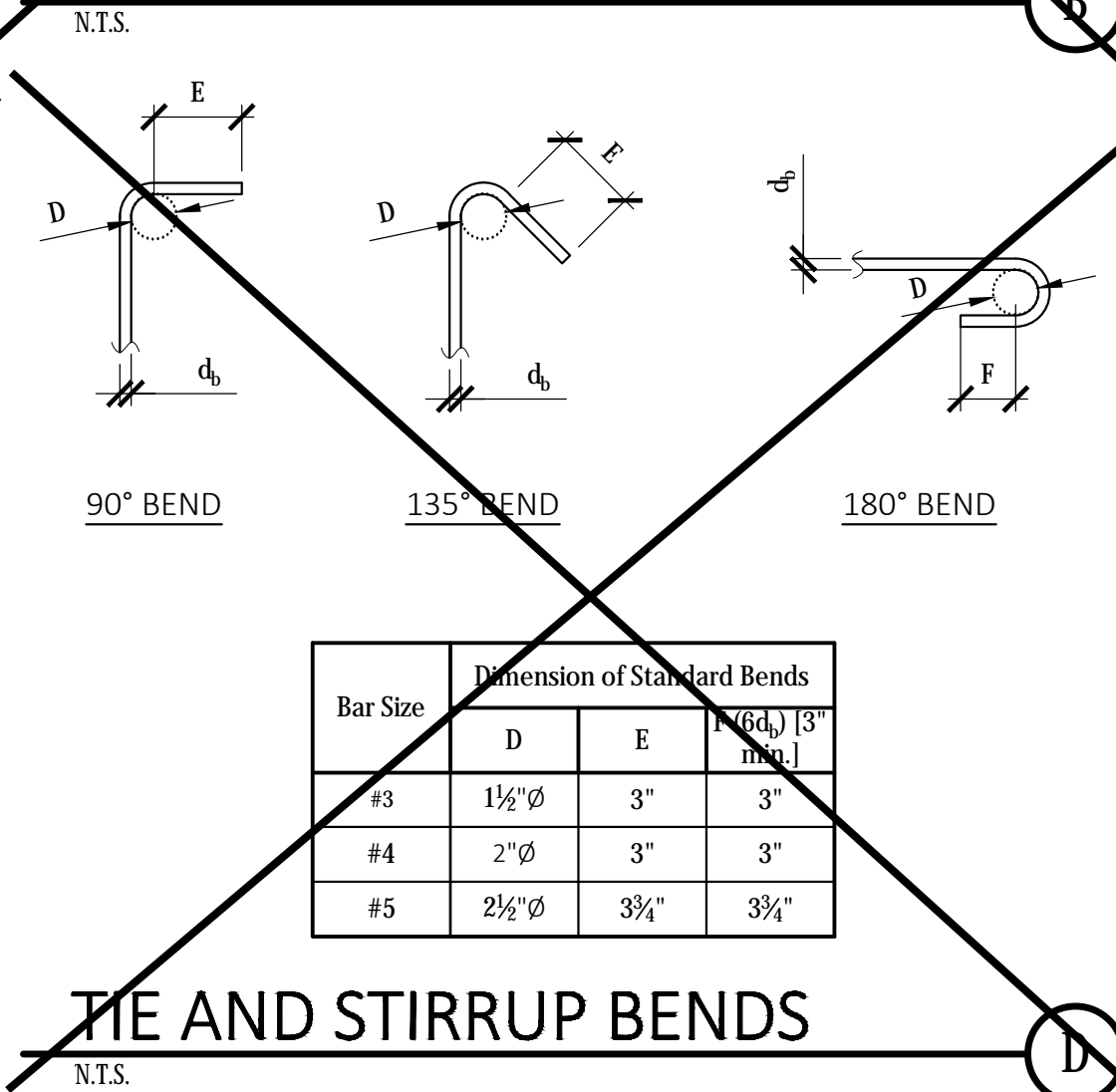
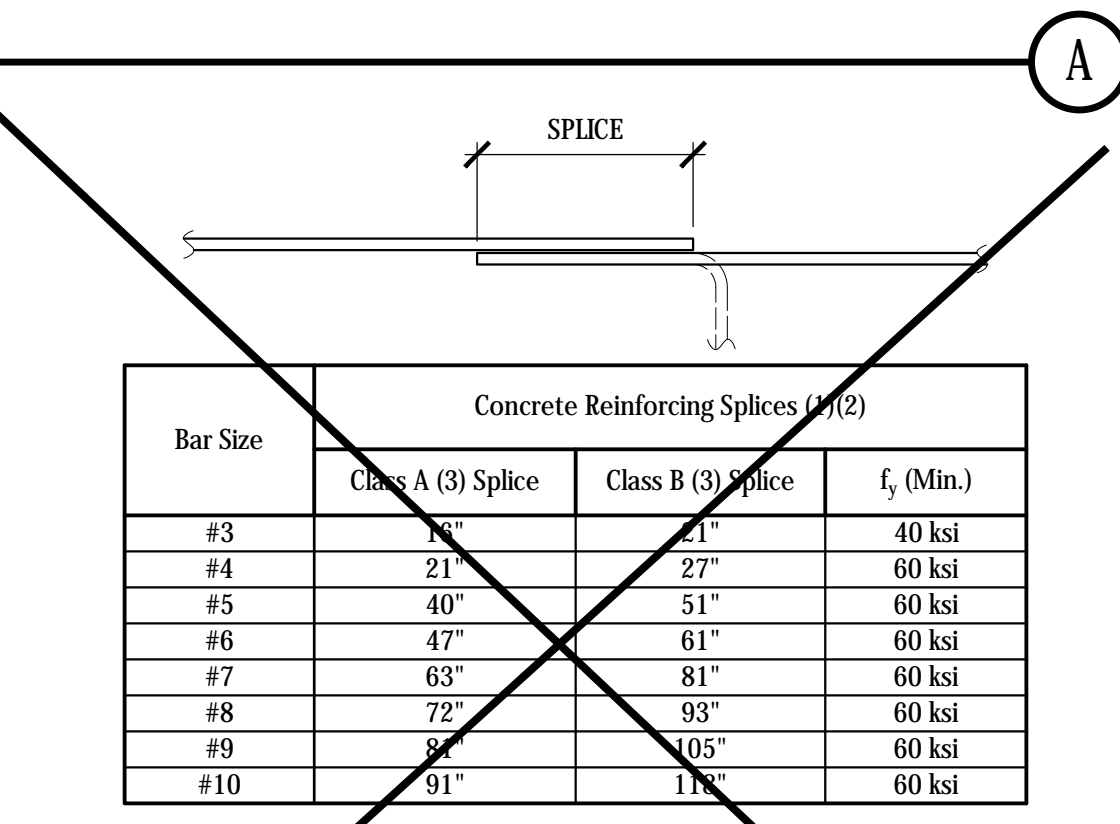
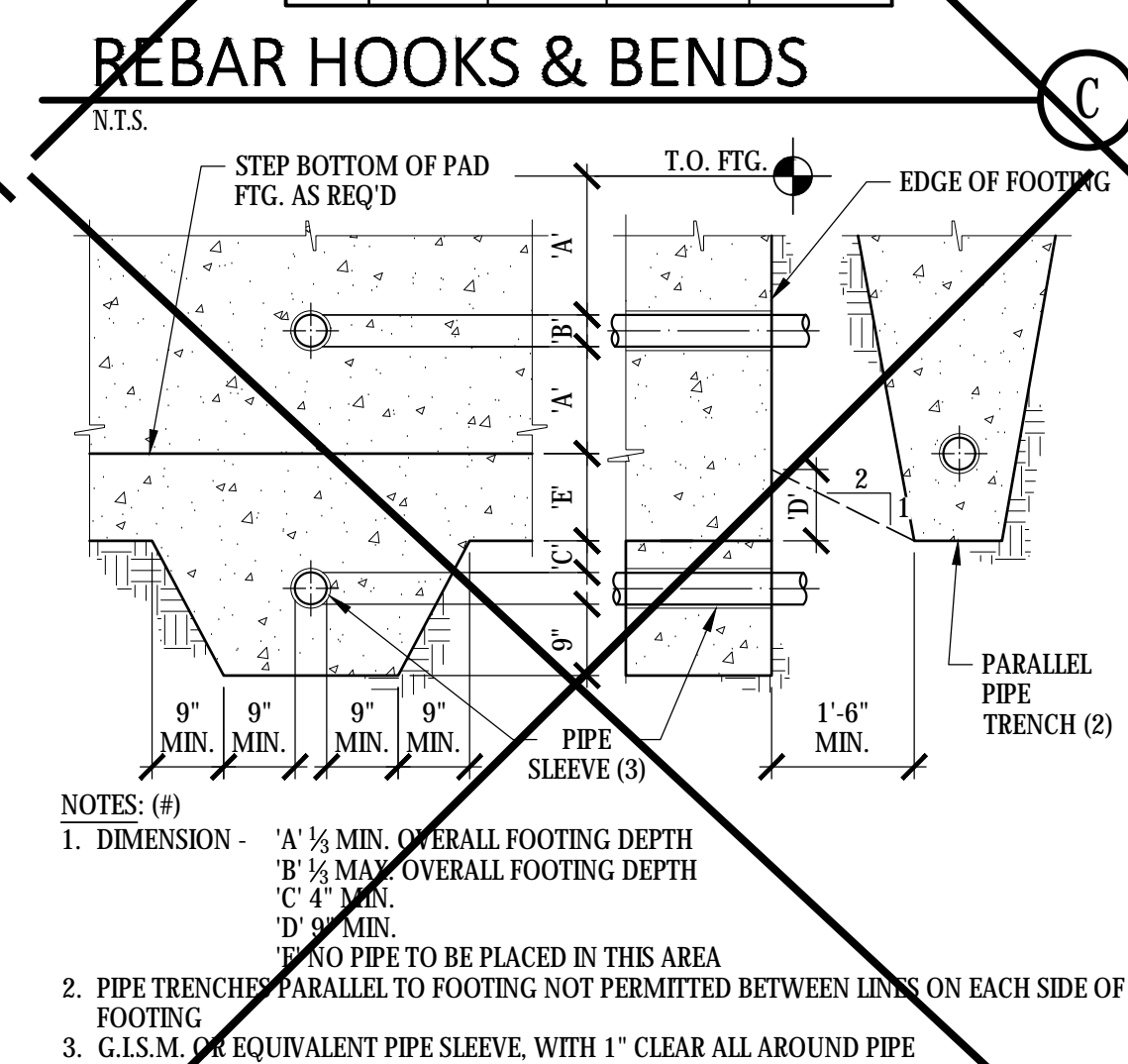
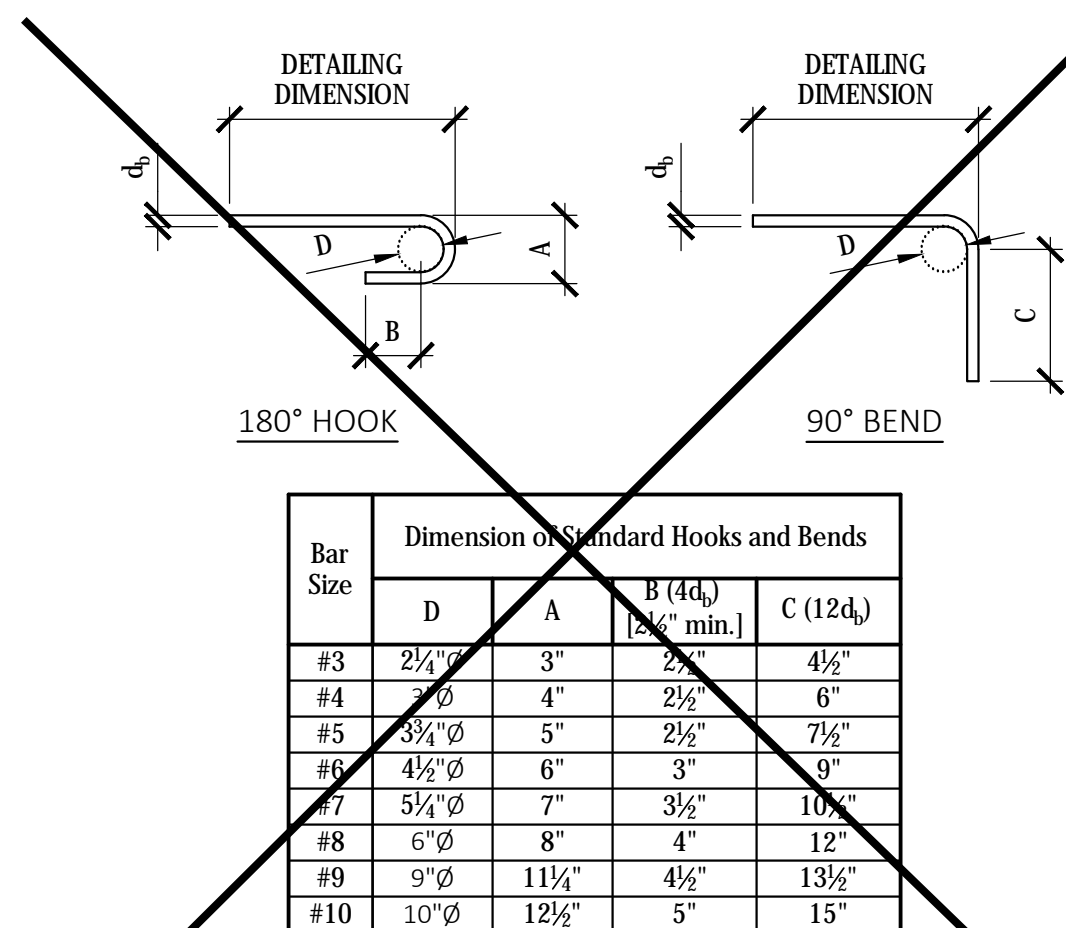
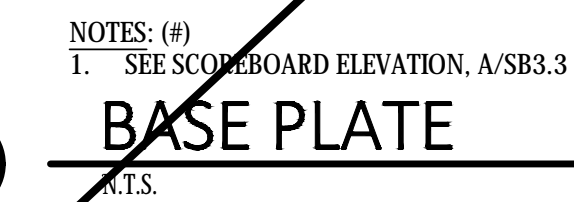
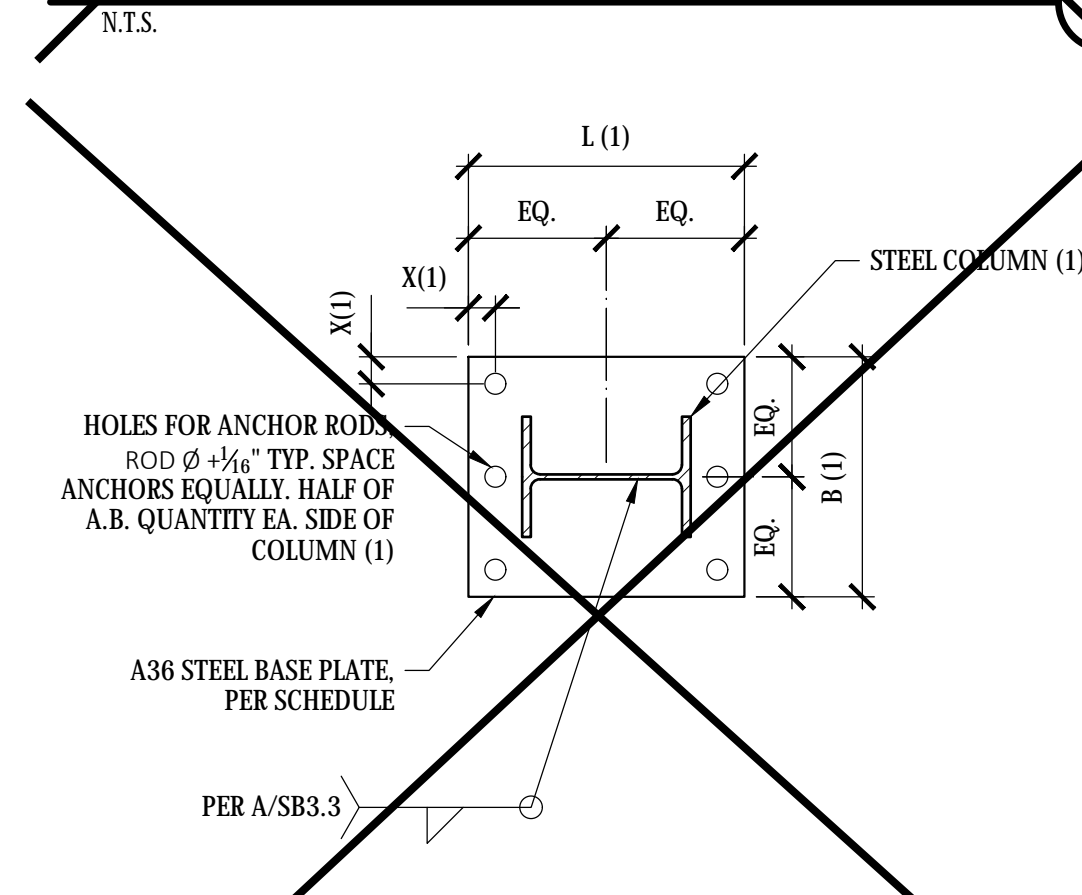
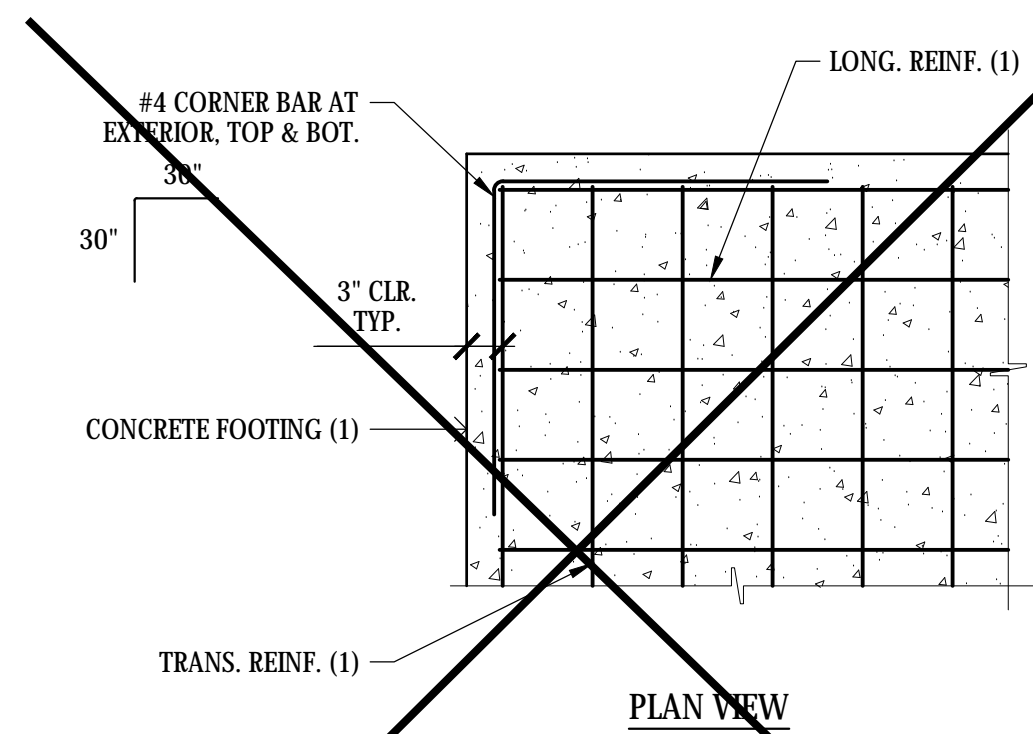
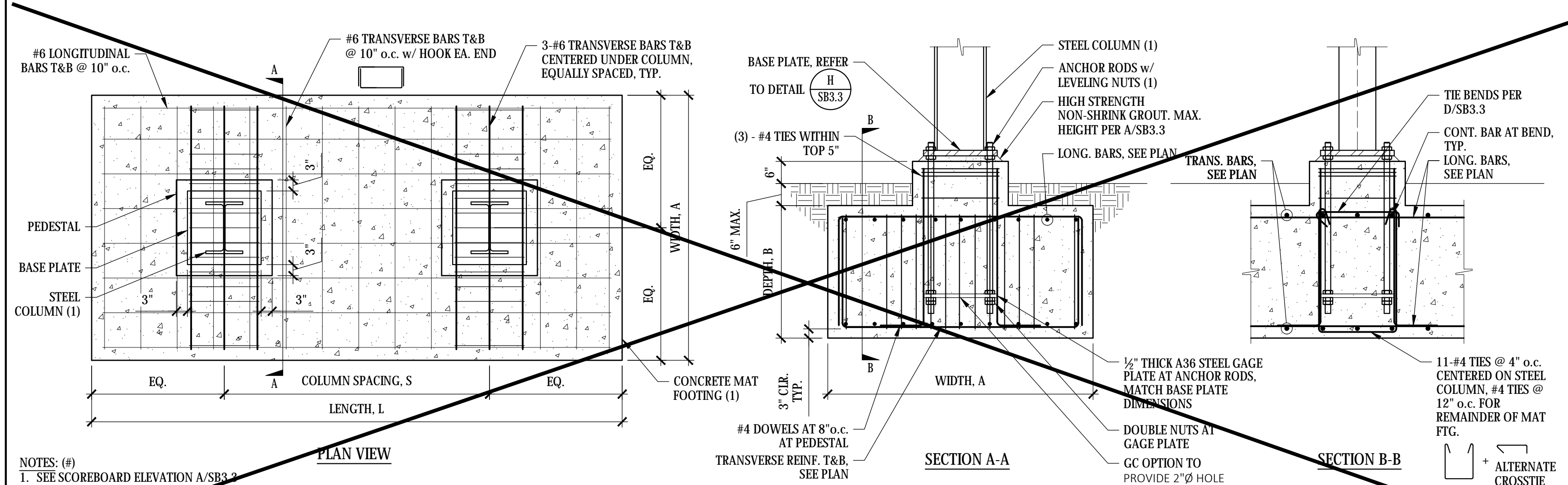
THREE COLUMN
CAISSON -
BOLTED

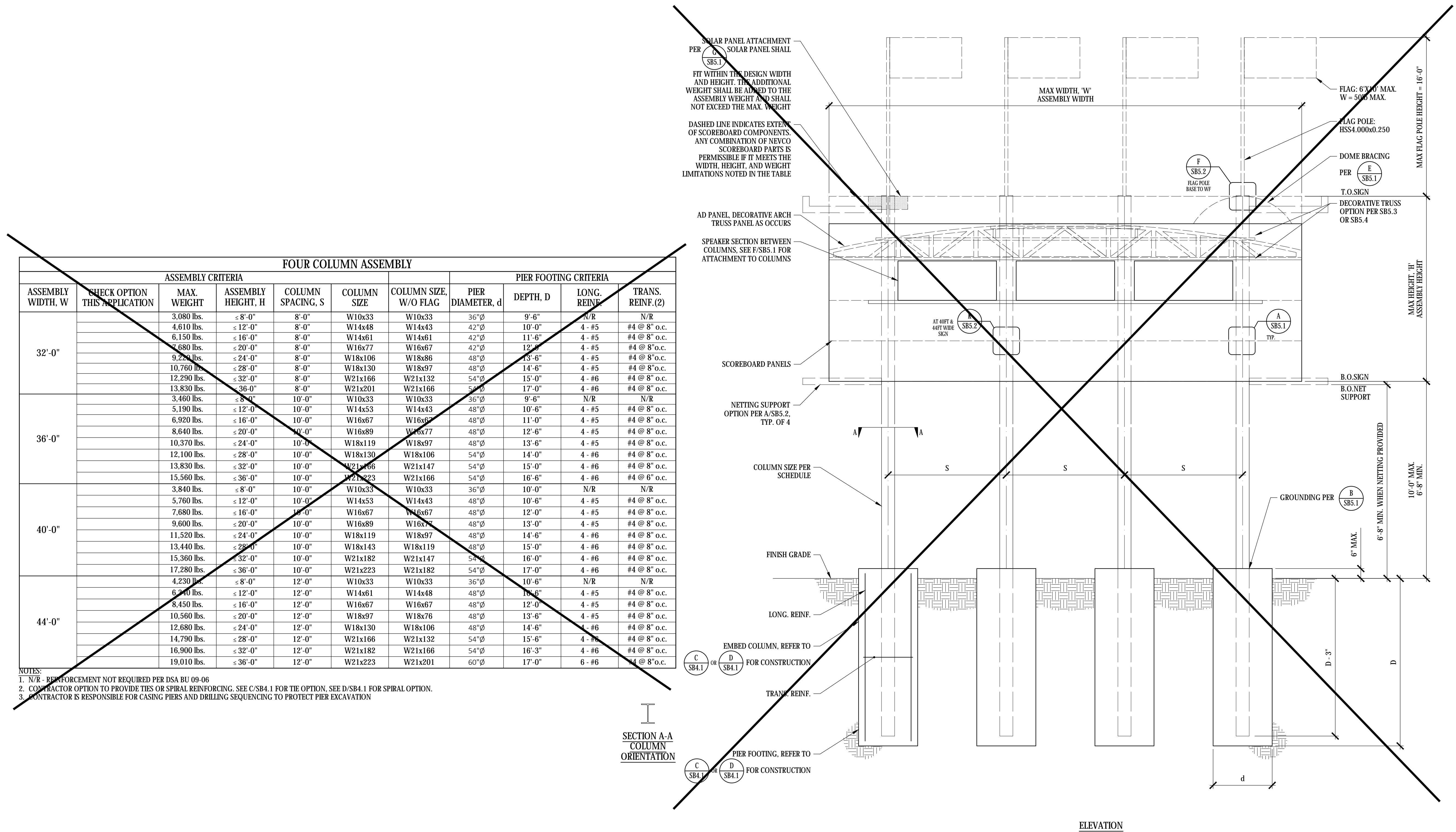
SHEET INFORMATION
DATE 08.09.2023
DRAWN JMK
CHECKED MEP
SSG JOB # S23109
SHEET SB3.2

THREE COLUMN ASSEMBLY																		
ASSEMBLY CRITERIA						MAT FOOTING CRITERIA				BASE PLATE				ANCHOR RODS				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	8'-0"	3'-0"	20'-0"	1½"	20"	20"	¾"	(4) - 1¼"	F1554 - GR.36	2½"	2"	30"
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	9'-0"	3'-0"	23'-0"	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	11'-0"	3'-0"	24'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		4,800 lbs.	≤ 20'-0"	8'-0"	W16x67	W16x67	12'-0"	3'-0"	27'-0"	1½"	30"	30"	1"	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	14'-0"	3'-0"	27'-0"	1½"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
24'-0"		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	23'-0"	1½"	20"	20"	¾"	(4) - 1¼"	F1554 - GR.55	2½"	2"	30"
		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	9'-0"	3'-0"	26'-0"	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	28'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	12'-0"	3'-0"	30'-0"	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.105	2½"	2"	30"
		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	14'-0"	3'-0"	30'-0"	1½"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
28'-0"		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	15'-0"	3'-0"	30'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1½"	20"	20"	¾"	(4) - 1¼"	F1554 - GR.55	2½"	2"	30"
		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	9'-0"	3'-0"	29'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	30'-0"	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-6"	3'-0"	32'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
32'-0"		8,070 lbs.	≤ 24'-0"	10'-0"	W18x97	W18x97	14'-0"	3'-0"	33'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"
		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	15'-0"	3'-0"	33'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.105	3"	2"	30"
		3,080 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1½"	20"	20"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-6"	3'-0"	30'-0"	1½"	24"	24"	¾"	(4) - 1½"	F1554 - GR.55	2½"	2"	30"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	11'-0"	3'-0"	32'-0"	1½"	24"	30"	¾"	(4) - 1½"	F1554 - GR.55	3"	2"	30"
	7,080 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	13'-0"	3'-6"	32'-0"	1½"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	30"	
	9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	14'-0"	4'-0"	33'-0"	2"	24"	30"	CIP	(4) - 1½"	F1554 - GR.55	3"	2"	36"	
	10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	15'-0"	4'-0"	33'-0"	2½"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2½"	36"	



N.T.S



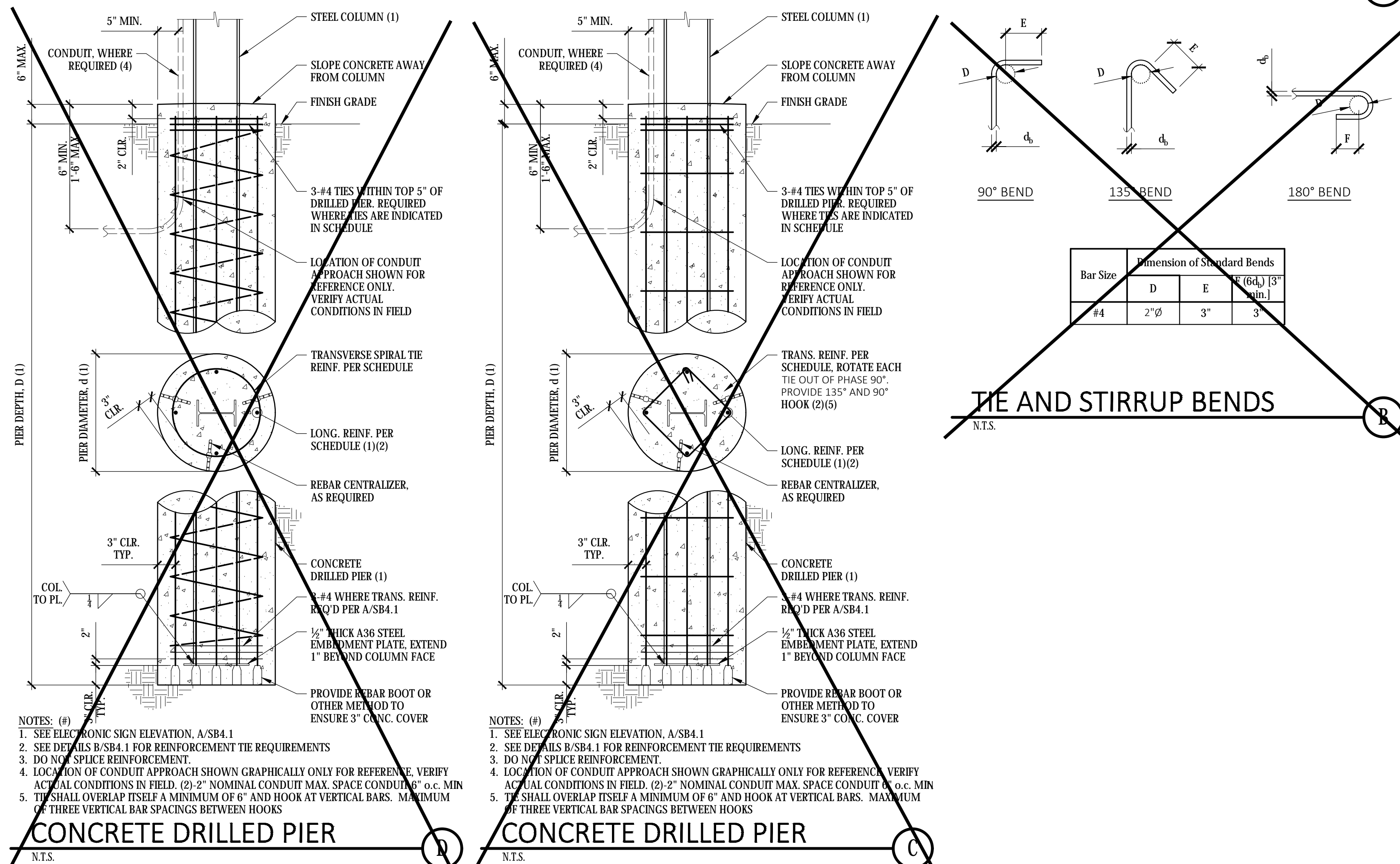


FOUR COLUMN ASSEMBLY									
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA			PIER FOOTING CRITERIA				
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.
32'-0"		3,080 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R
		4,610 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	42"Ø	10'-0"	4-#5
		6,150 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	42"Ø	11'-6"	4-#5
		7,680 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	42"Ø	12'-6"	4-#5
		9,220 lbs.	≤ 24'-0"	8'-0"	W18x106	W18x86	48"Ø	13'-6"	4-#5
		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x97	48"Ø	14'-8"	4-#5
36'-0"		12,290 lbs.	≤ 32'-0"	8'-0"	W21x166	W21x132	54"Ø	15'-0"	4-#6
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	54"Ø	17'-0"	4-#6
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	4-#5
		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-0"	4-#5
		8,640 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	4-#5
40'-0"		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	48"Ø	13'-6"	4-#5
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	54"Ø	14'-0"	4-#6
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	54"Ø	15'-0"	4-#6
		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	54"Ø	16'-6"	4-#6
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	10'-0"	N/R
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	4-#5
44'-0"		7,680 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	12'-0"	4-#5
		9,600 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	13'-0"	4-#5
		11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	48"Ø	14'-8"	4-#6
		13,440 lbs.	≤ 28'-0"	10'-0"	W18x143	W18x119	48"Ø	15'-0"	4-#6
		15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	54"Ø	16'-0"	4-#6
		17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	54"Ø	17'-0"	4-#6
		4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-6"	N/R
		6,240 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	11'-6"	4-#5
		8,450 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	12'-0"	4-#5
		10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	48"Ø	13'-6"	4-#5
		12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	48"Ø	14'-6"	4-#6
		14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	54"Ø	15'-6"	4-#6
		16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	54"Ø	16'-3"	4-#6
		19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	60"Ø	17'-0"	4-#6

NOTES: (1) N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06
(2) CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB4.1 FOR TIE OPTION. SEE D/SB4.1 FOR SPIRAL OPTION.
(3) CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

FOUR COLUMN SCOREBOARD INSTALLATION

N.T.S.



- NOTES: (1) SEE ELECTRONIC SIGN ELEVATION, A/SB4.1
(2) SEE DETAILS B/SB4.1 FOR REINFORCEMENT TIE REQUIREMENTS
(3) DO NOT SPLICE REINFORCEMENT.
(4) LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2\"/>

APPLICATION#
02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT
APP: 02-122089 INC.
REVIEWED FOR
SS ☒ FLS ☒ ACS ☒
DATE: 4/3/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. FARVER
No. 5405
STATE OF CALIFORNIA
PC SEOR SEAL
08.09.2023

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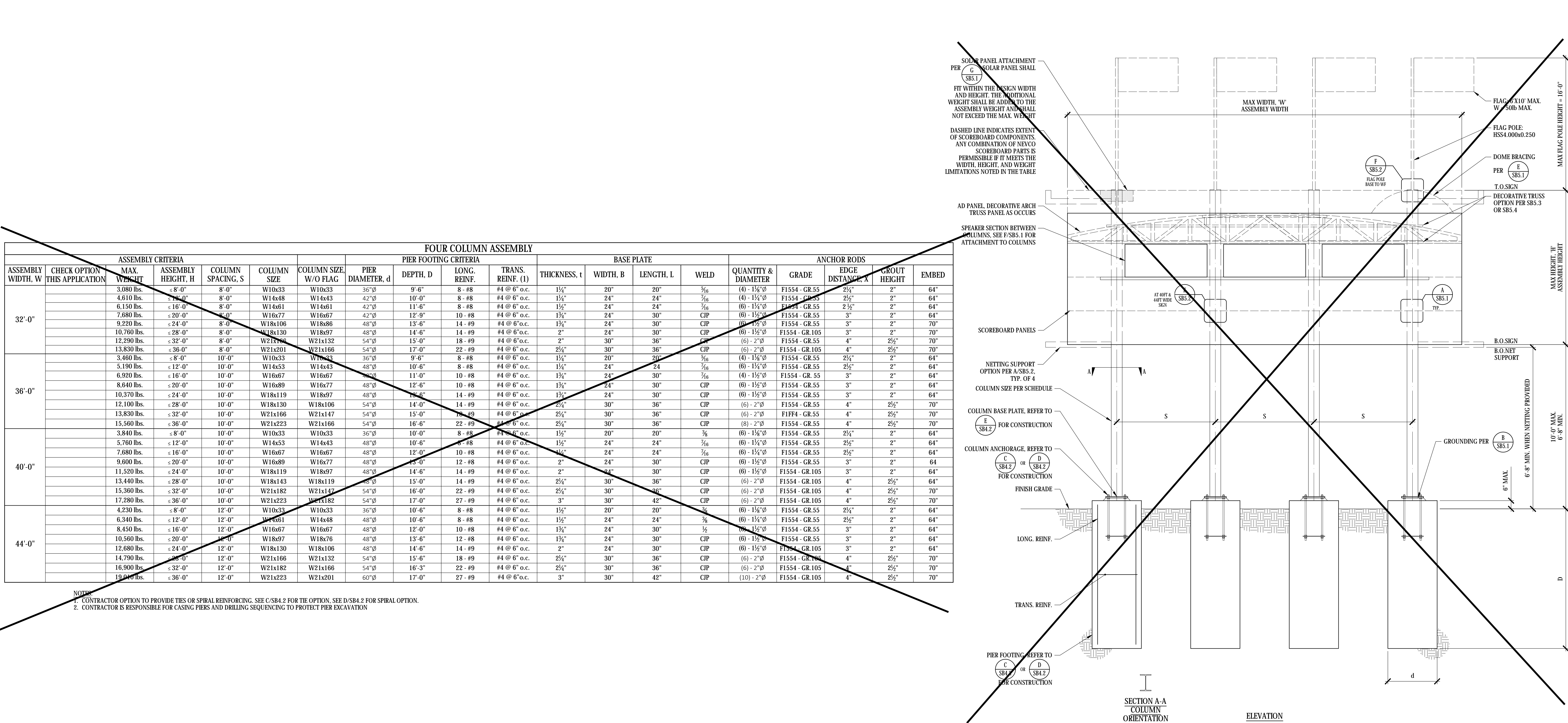
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DIV. OF THE STATE ARCHITECT
APP: 04-122377 PC
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SS ☒ FLS ☒ ACS ☒ CG ☐
DATE: 09/20/2023

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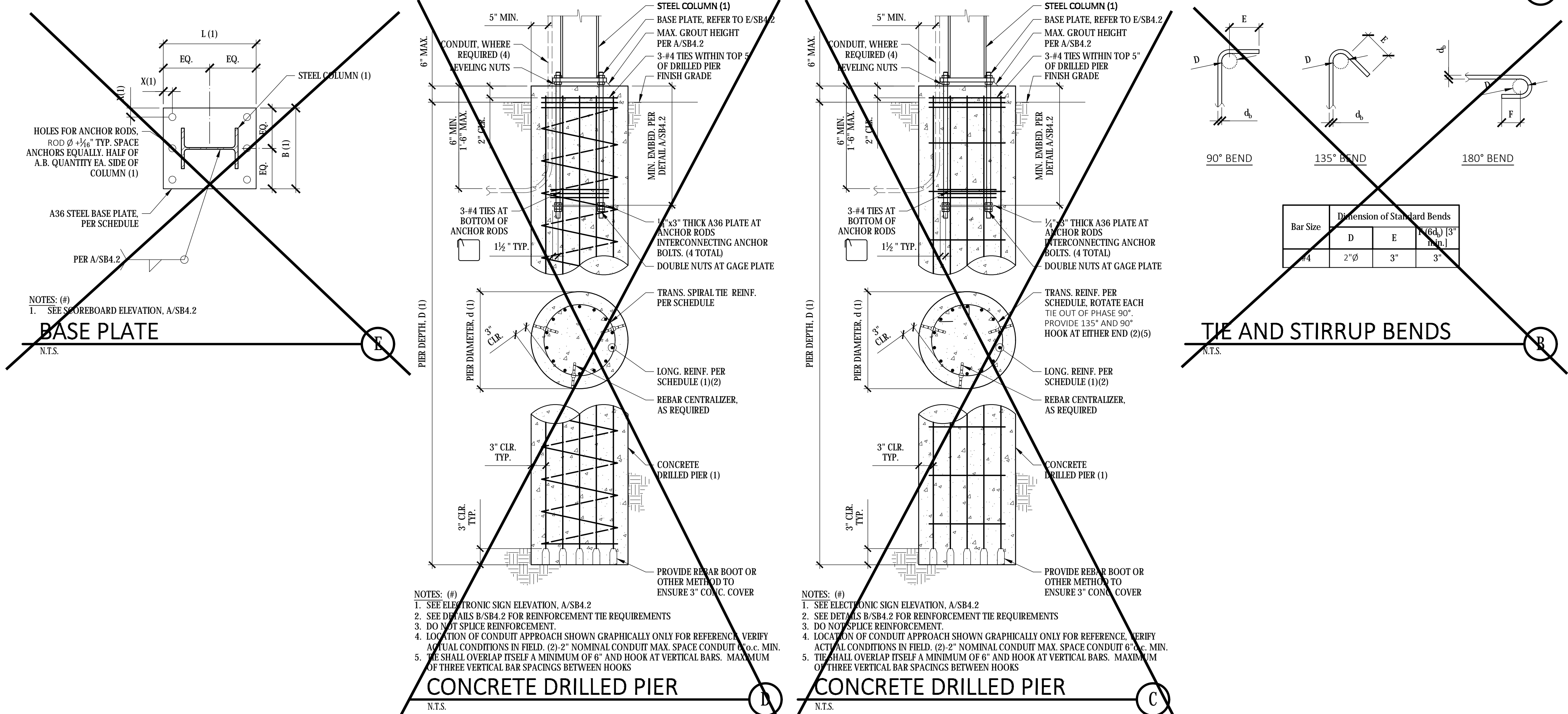
FOUR COLUMN
CAISSON -
EMBEDDED

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB4.1



FOUR COLUMN SCOREBOARD INSTALLATION

NTS



APPLICATION# 02-122089

DIV. OF THE STATE ARCHITECT

APP: 02-122089 INC.

REVIEWED FOR

SS ☒ FLS ☒ ACS ☒

DATE: 4/3/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER

Michael E. Parviz

No. 5405

STRUCTURAL

STATE OF CALIFORNIA

DATE SIGNED: 08.09.2023

PC SEOR REAL

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REGISTERED PROFESSIONAL ENGINEER

CAIYUO R. FANG

No. 52386

EXPIRES 12/31/2025

STRUCTURAL

STATE OF CALIFORNIA

FOUR COLUMN CAISSON - BOLTED

SHEET INFORMATION

DATE: 08.09.2023

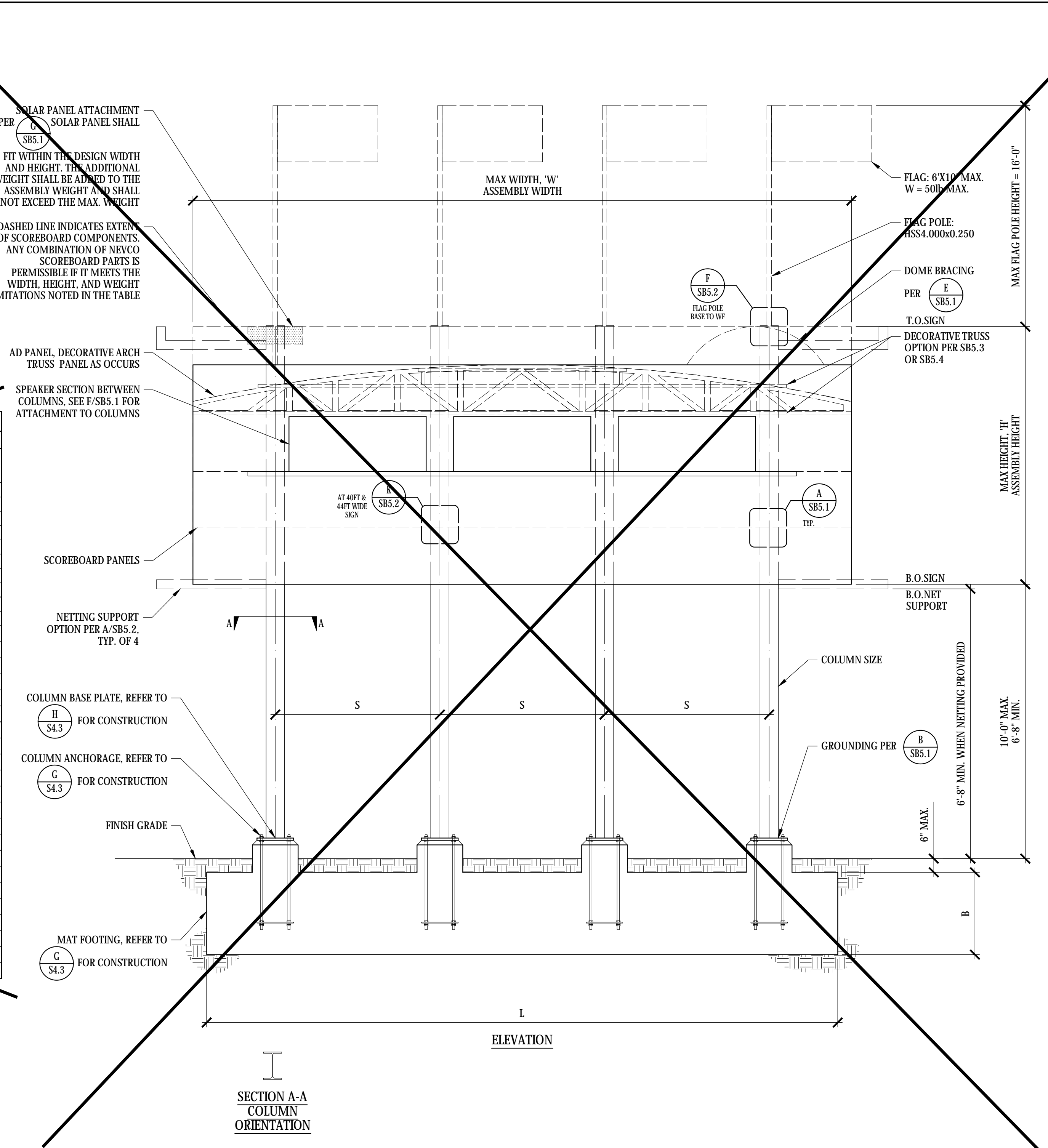
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SSG JOB #: S23109

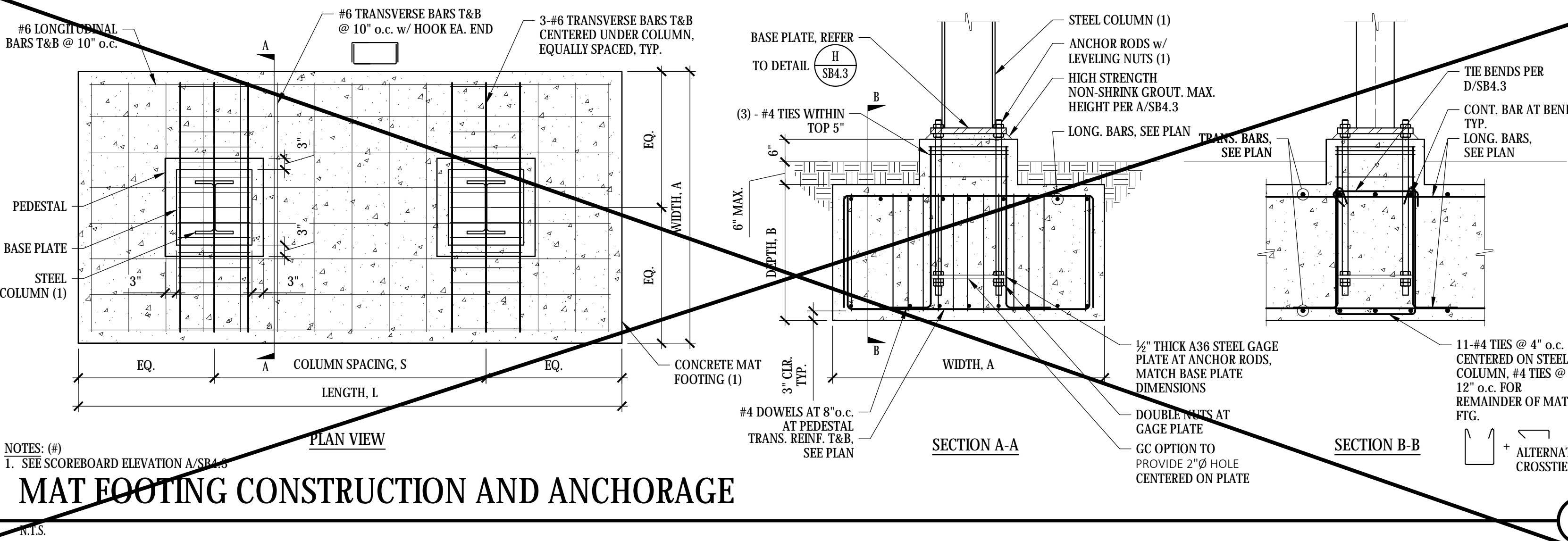
SHEET: SB4.2

FOUR COLUMN ASSEMBLY														ANCHOR RODS				
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	MAT FOOTING CRITERIA				BASE PLATE		WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
		MAX. WEIGHT	ASSEMBLY HEIGHT, H				WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L						
32'-0"		3,080 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1 1/4"	20"	20"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		4,610 lbs.	≤ 8'-0"	8'-0"	W14x48	W14x43	9'-0"	3'-0"	30'-0"	1 1/4"	24"	24"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		6,150 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	35'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		7,690 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	12'-6"	3'-0"	36'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		9,220 lbs.	≤ 24'-0"	8'-0"	W18x106	W18x86	14'-0"	3'-0"	39'-0"	1 1/4"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x97	15'-0"	3'-6"	39'-0"	2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.105	3"	2"	30"
36'-0"		12,290 lbs.	≤ 32'-0"	8'-0"	W21x196	W21x132	17'-0"	4'-0"	42'-0"	2"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	18'-0"	4'-0"	46'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	7'-6"	3'-0"	36'-0"	1 1/4"	20"	20"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	36'-0"	1 1/4"	24"	24"	3/16	(4) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	36'-0"	1 1/2"	24"	30"	3/8	(4) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		8,640 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-0"	3'-0"	40'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
40'-0"		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	14'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	16'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	18'-0"	4'-0"	40'-0"	2 1/2"	30"	36"	CIP	(8) - 2"Ø	F1554 - GR.55	4"	2 1/2"	36"
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	36'-0"	1 1/2"	20"	20"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
44'-0"		7,680 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		9,600 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	13'-0"	3'-0"	40'-0"	2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.105	3"	2"	30"
		13,440 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	14'-0"	4'-0"	42'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	16'-0"	4'-0"	44'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	18'-0"	4'-0"	46'-0"	3"	30"	42"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	7'-0"	3'-0"	42'-0"	1 1/2"	20"	20"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		6,340 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-0"	3'-0"	43'-0"	1 1/2"	24"	24"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	2 1/2"	2"	30"
		8,450 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	10'-0"	3'-0"	45'-0"	1 1/2"	24"	30"	3/8	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	12'-0"	3'-6"	46'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.55	3"	2"	30"
		12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	14'-0"	3'-6"	47'-0"	2"	24"	30"	CIP	(6) - 1 1/2"Ø	F1554 - GR.105	3"	2"	30"
		14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	15'-0"	4'-0"	48'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	30"
		16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	16'-0"	4'-0"	49'-0"	2 1/4"	30"	36"	CIP	(6) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"
		19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	18'-0"	4'-0"	52'-0"	3"	30"	42"	CIP	(10) - 2"Ø	F1554 - GR.105	4"	2 1/2"	36"

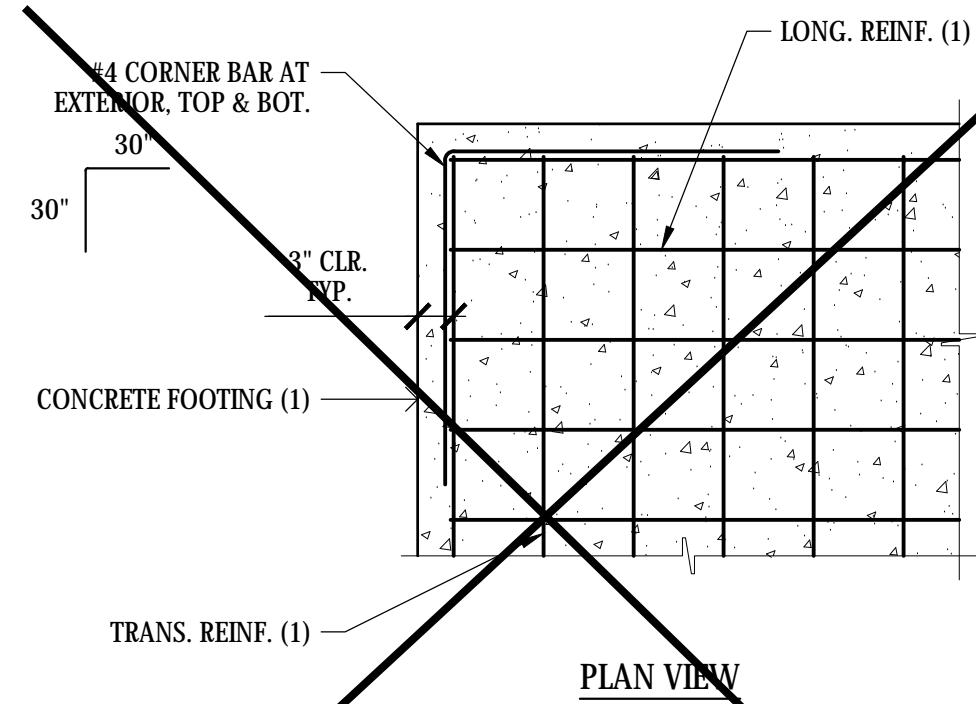


FOUR COLUMN SCOREBOARD INSTALLATION

N.T.S.



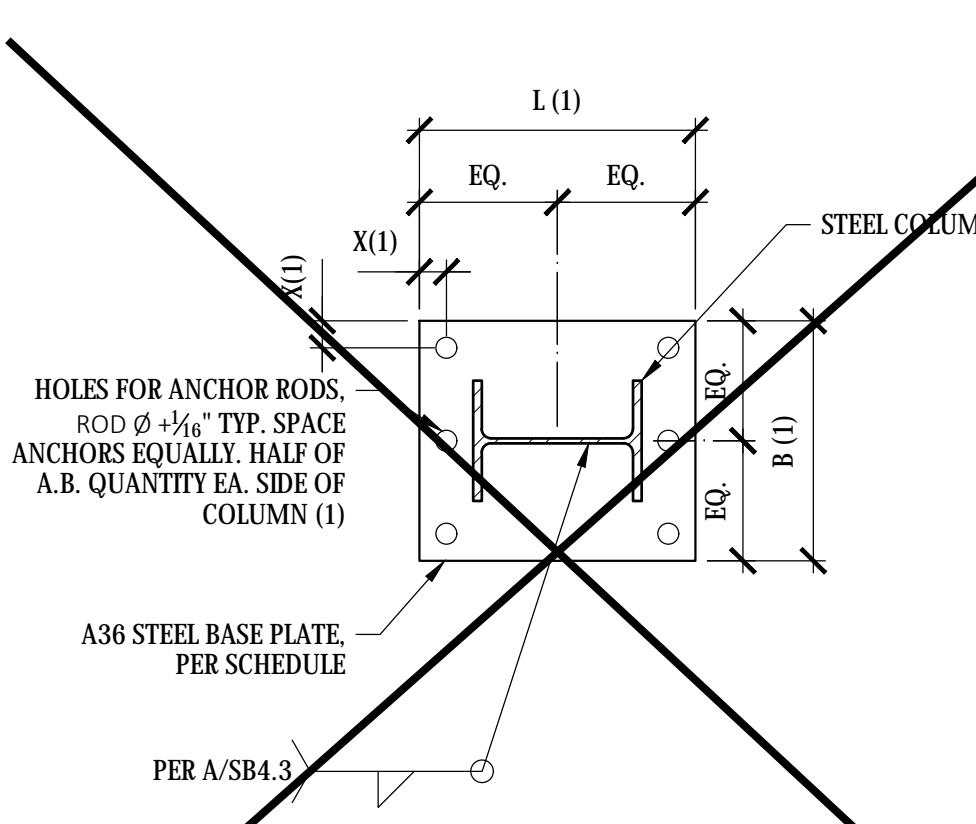
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PLAN VIEW

TYP. FOOTING CORNER

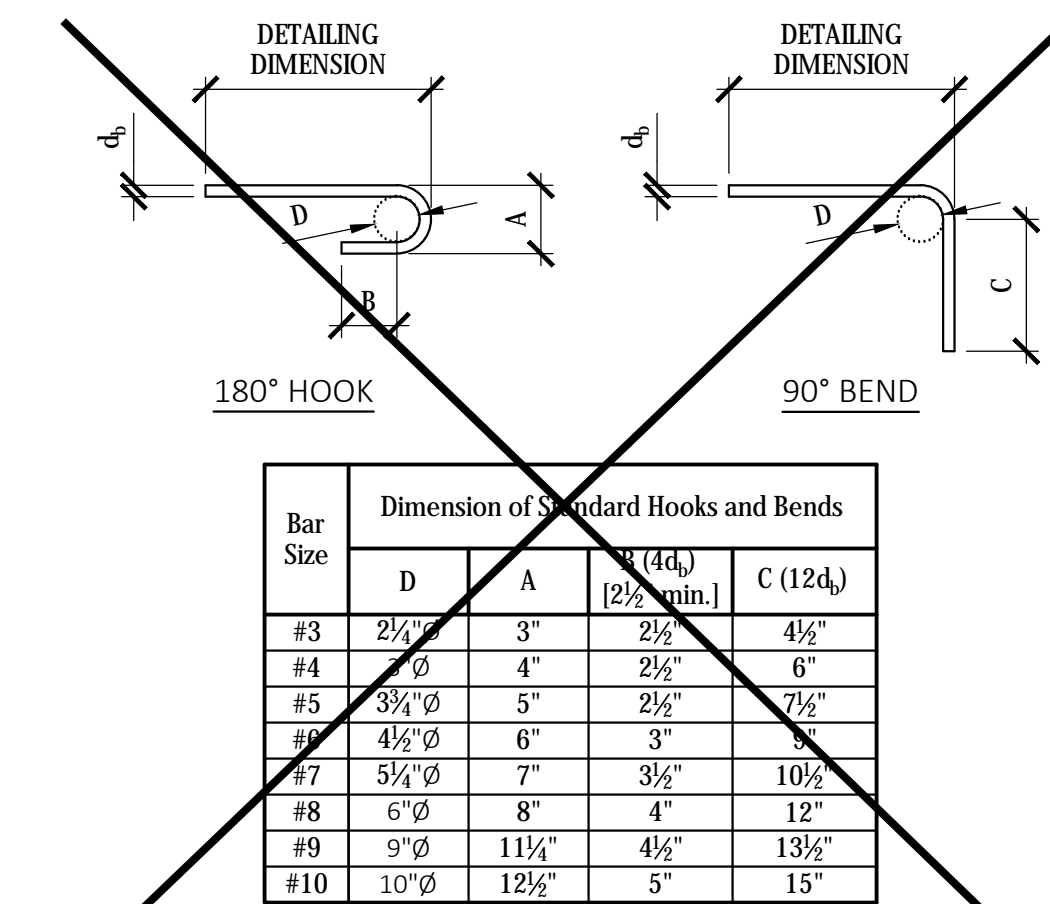
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SECTION A-A

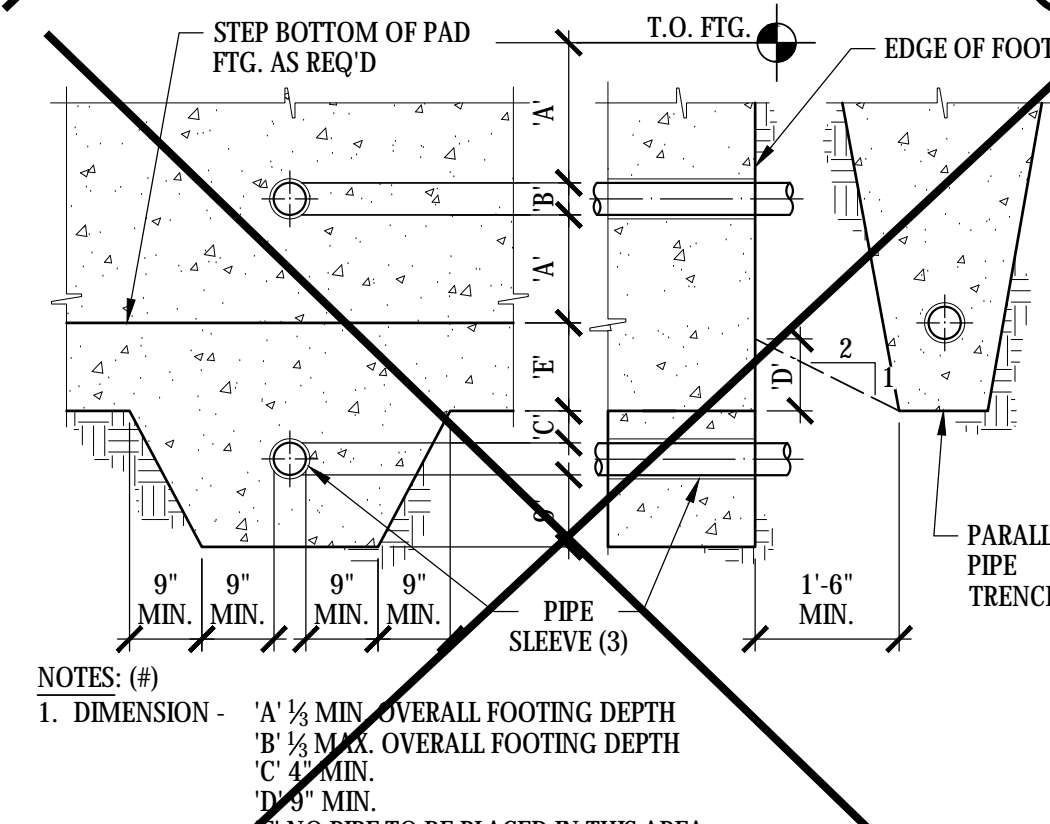
BASE PLATE

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REBAR HOOKS & BENDS

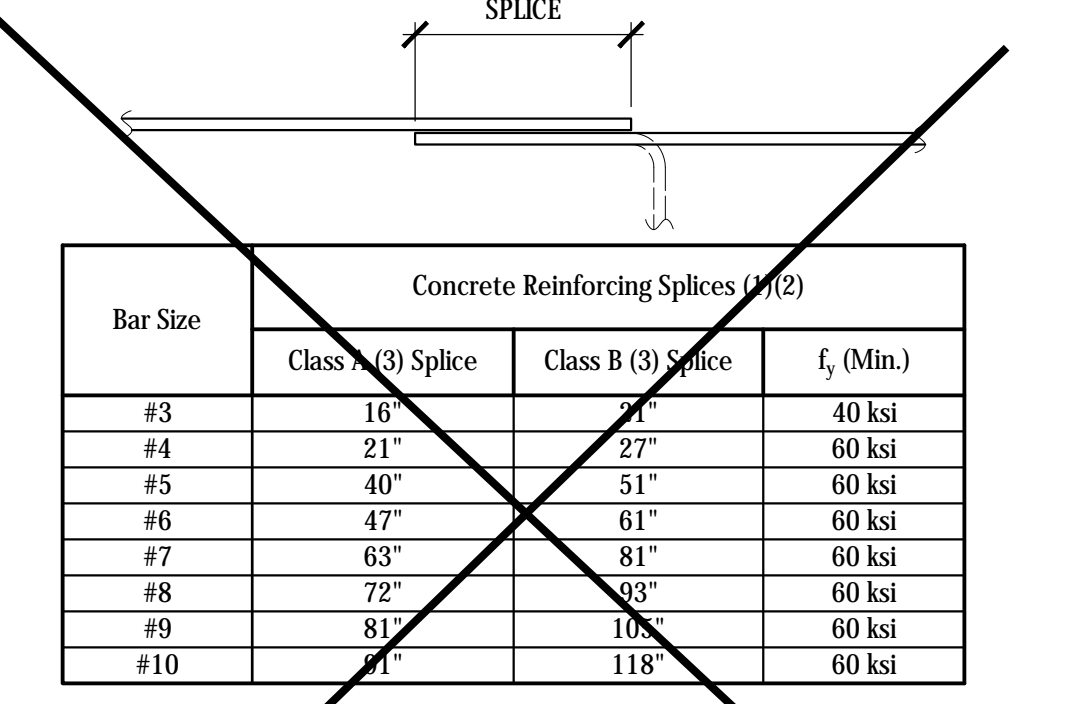
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DETAILING DIMENSION

PIPE THROUGH FOOTING

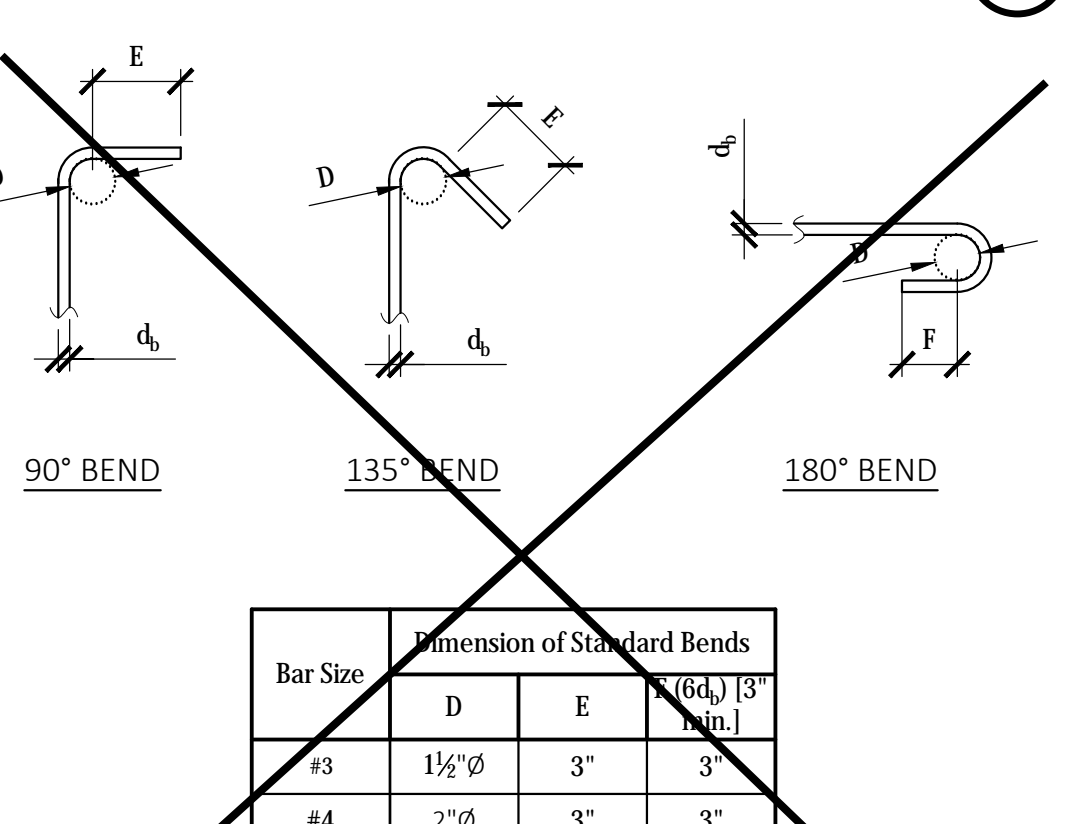
N.T.S.



DETAILING DIMENSION

TYPICAL LAP SPLICES

N.T.S.



DETAILING DIMENSION

TIE AND STIRRUP BENDS

N.T.S.

APPLICATION#
02-122089

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APP: 02-122089 INC.
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DATE: 4/3/2024

SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARVIZ
No. 5405
STATE OF CALIFORNIA
DATE REIGNED
08.09.2023

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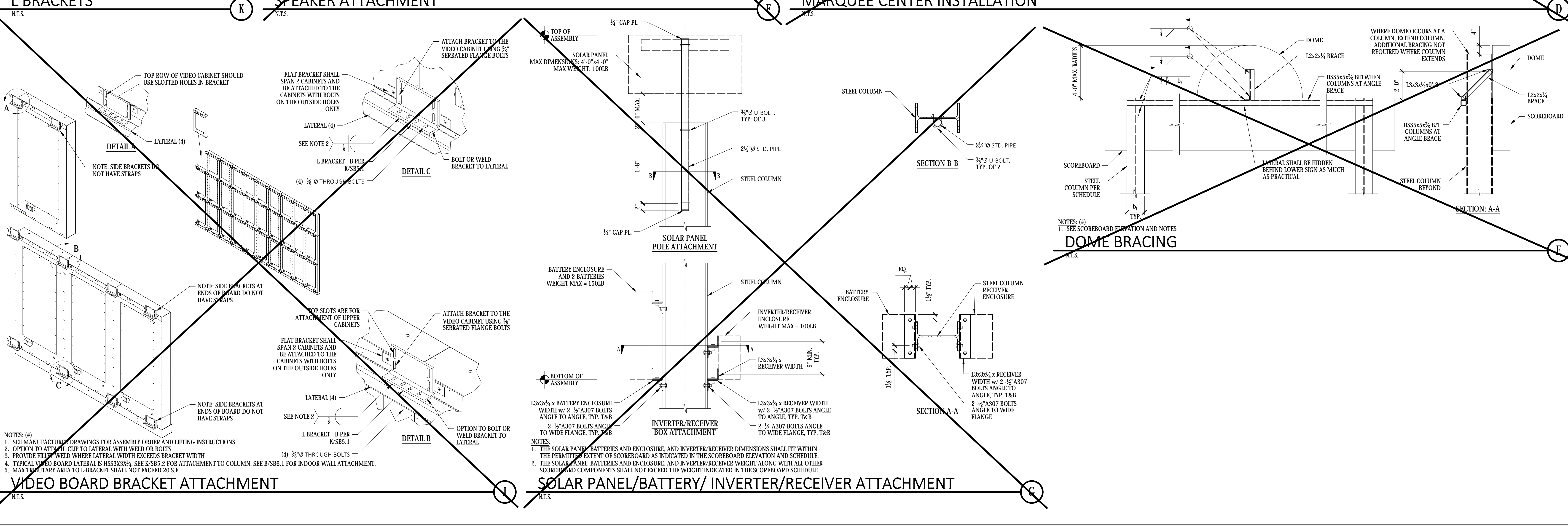
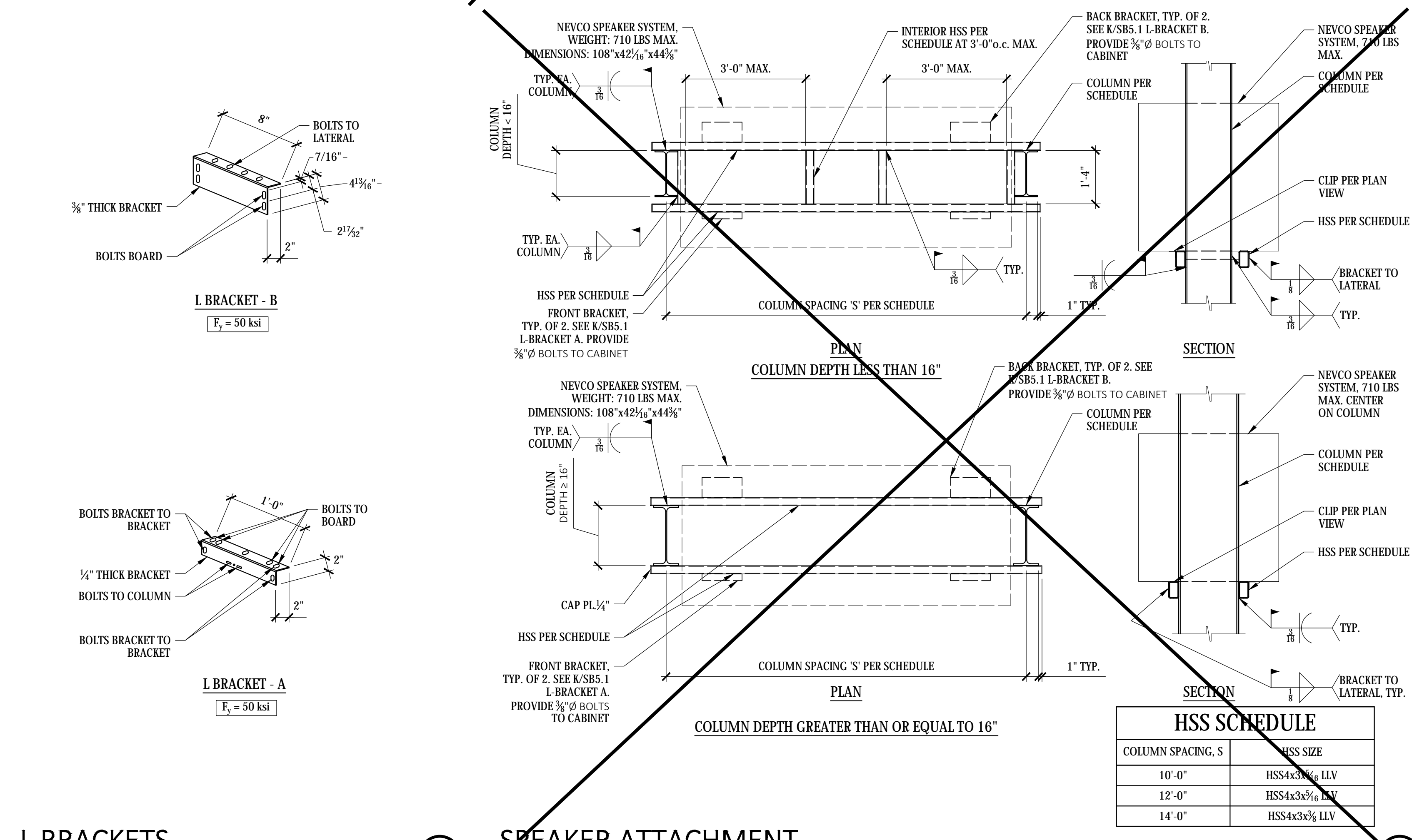
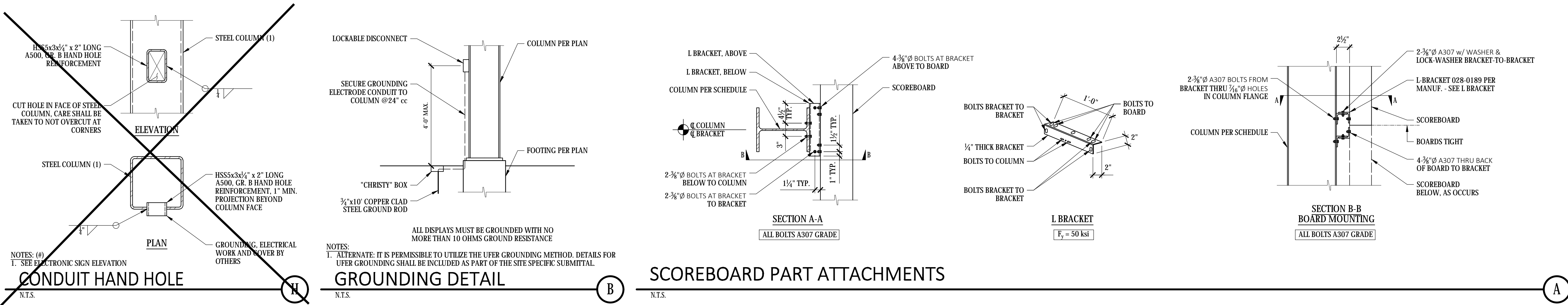
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A separate project application
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REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. PARVIZ
No. 52386
STATE OF CALIFORNIA
DATE REIGNED
08.09.2023

SHEET INFORMATION
DATE 08.09.2023
DRAWN JMK
CHECKED MEP
SSG JOB # S23109
SHEET SB4.3



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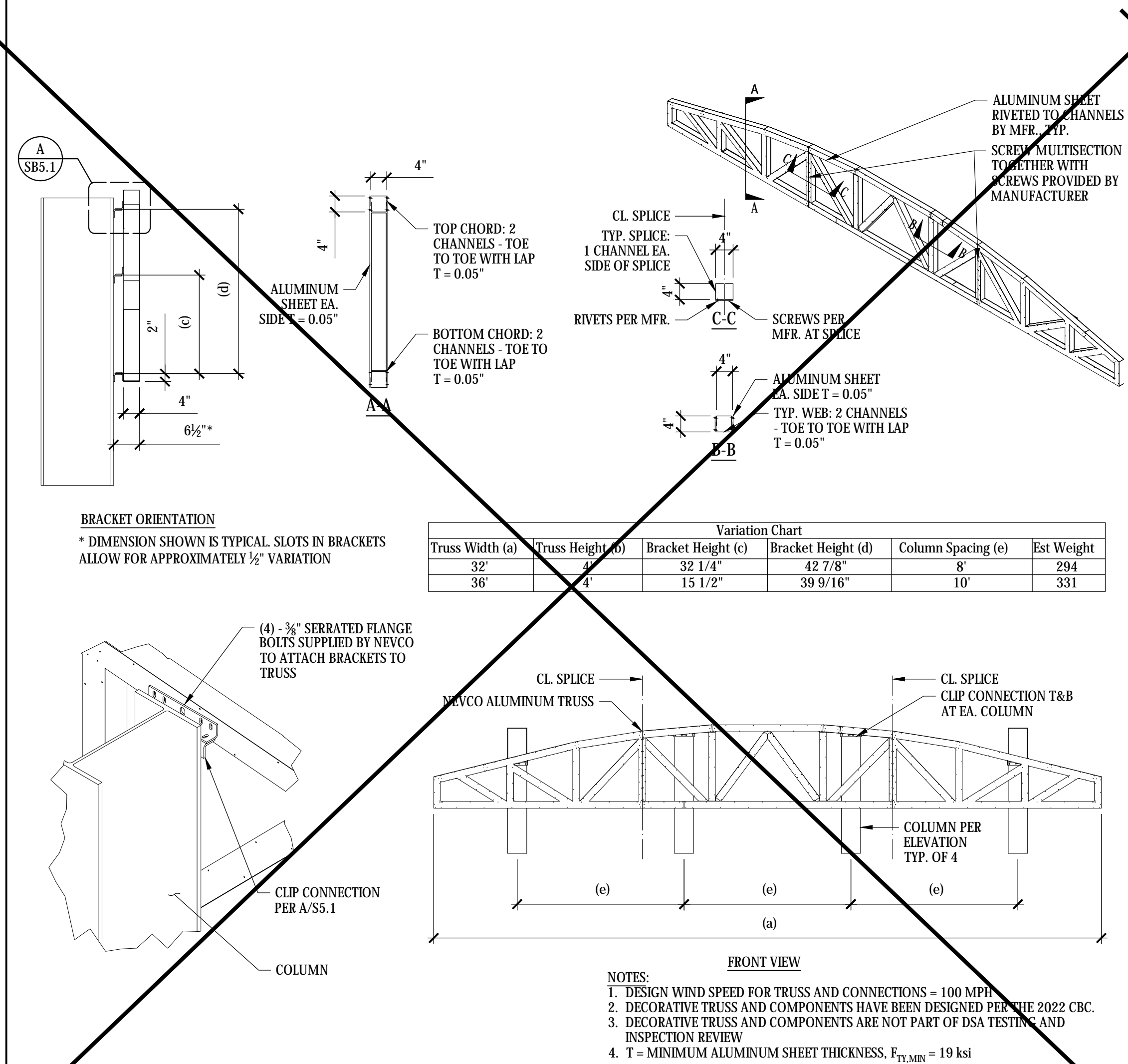
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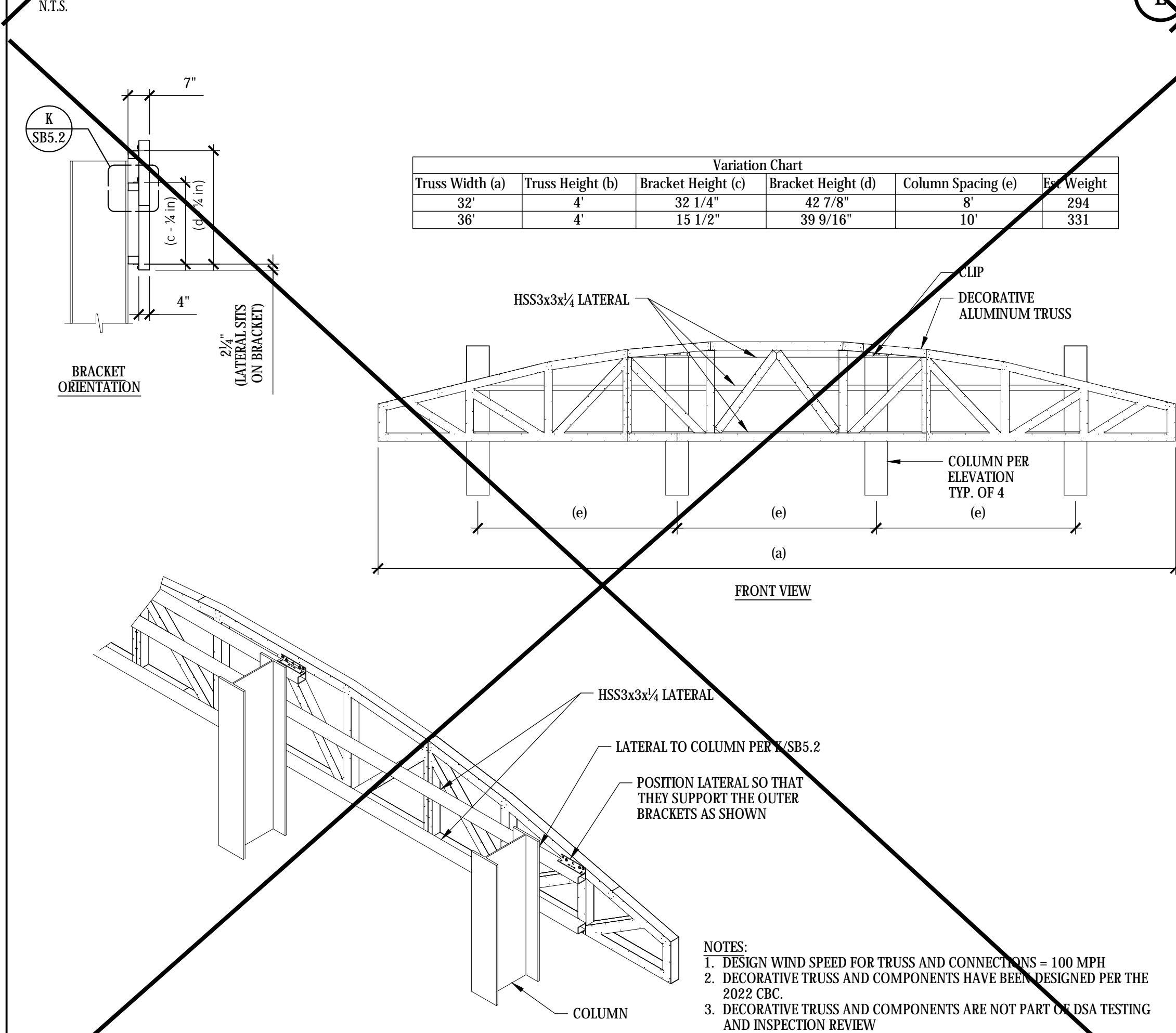
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ATTACHMENT
DETAILS

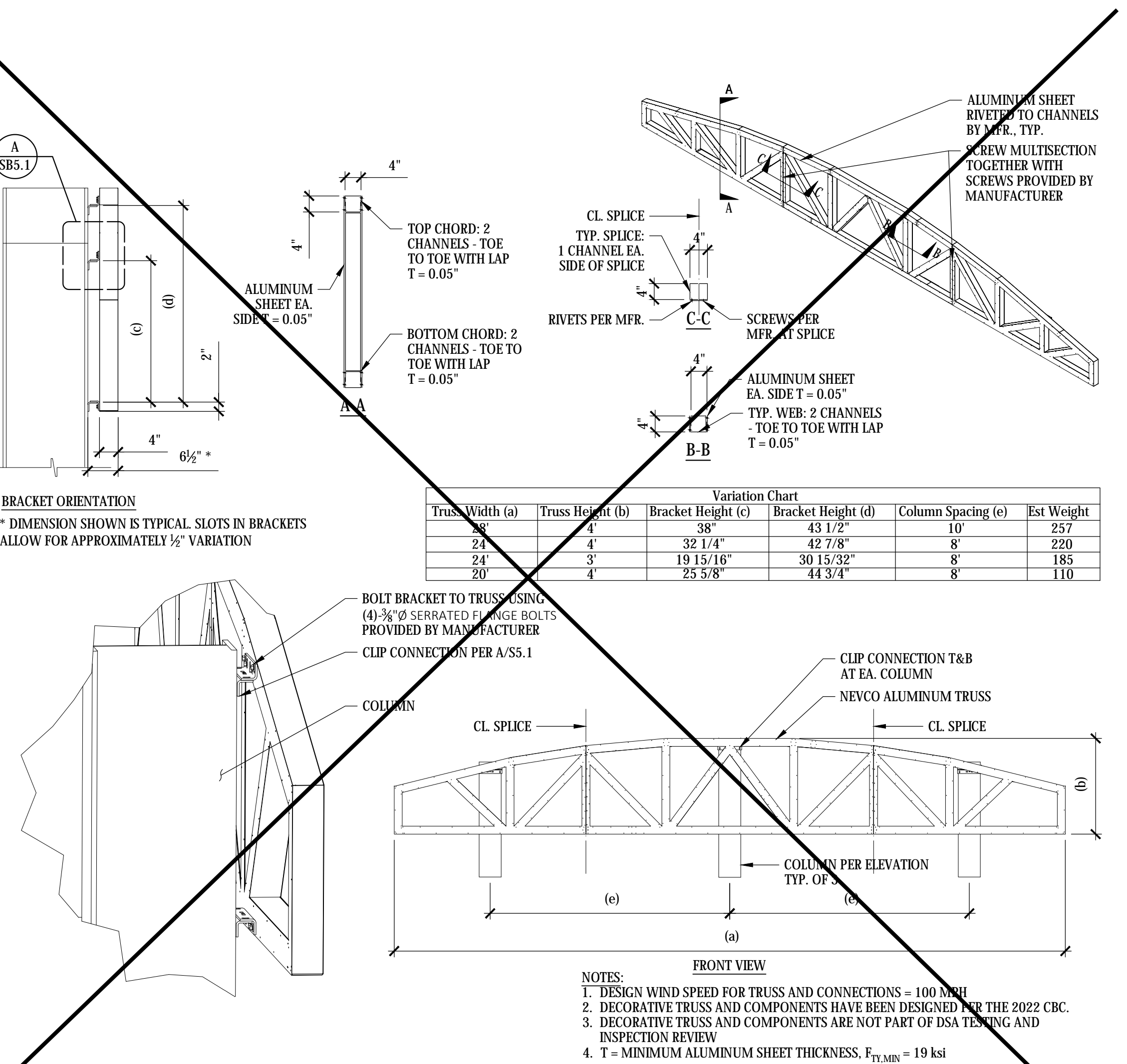
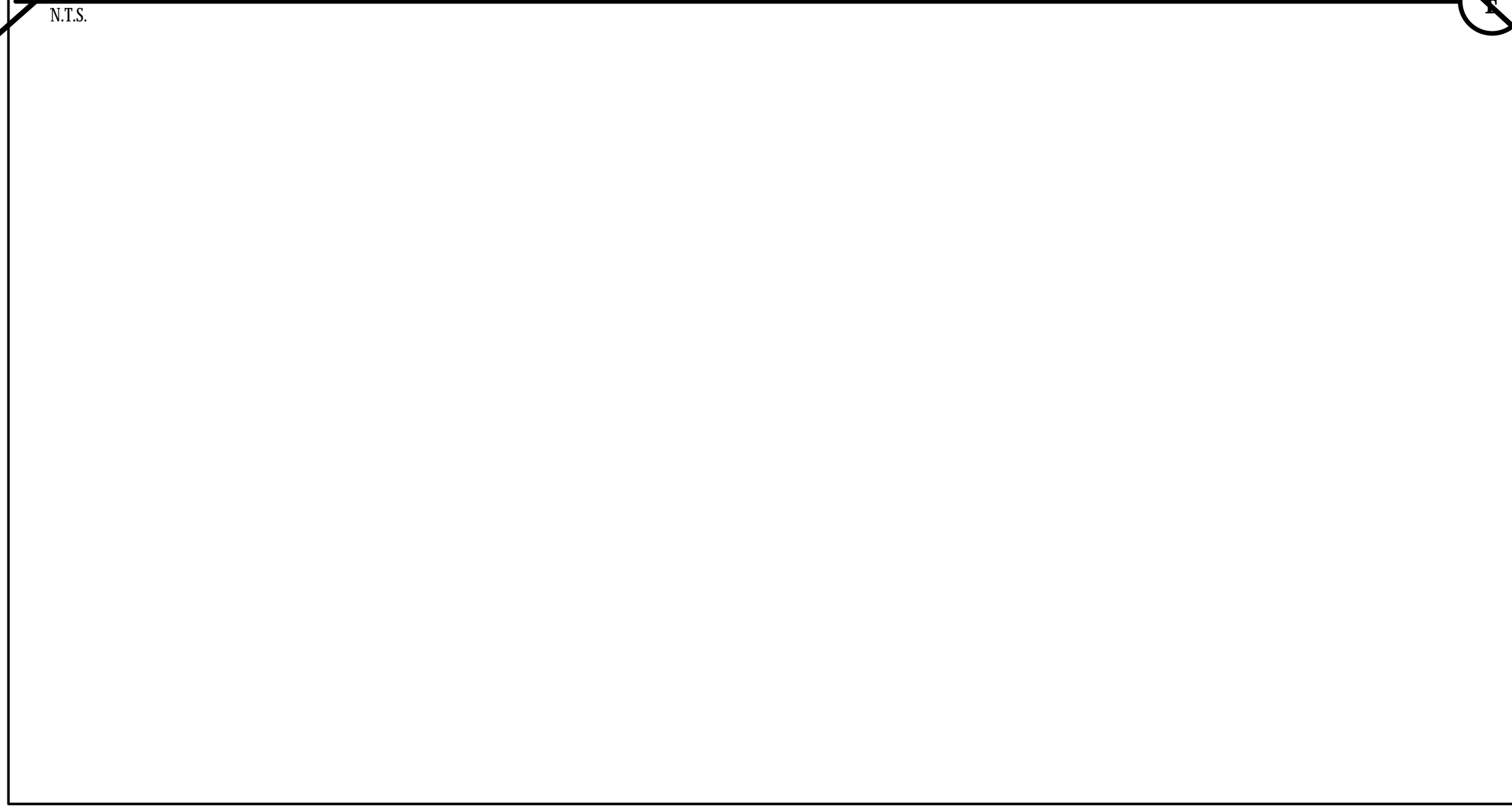
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SSG JOB #: S23109
SHEET: SB5.1



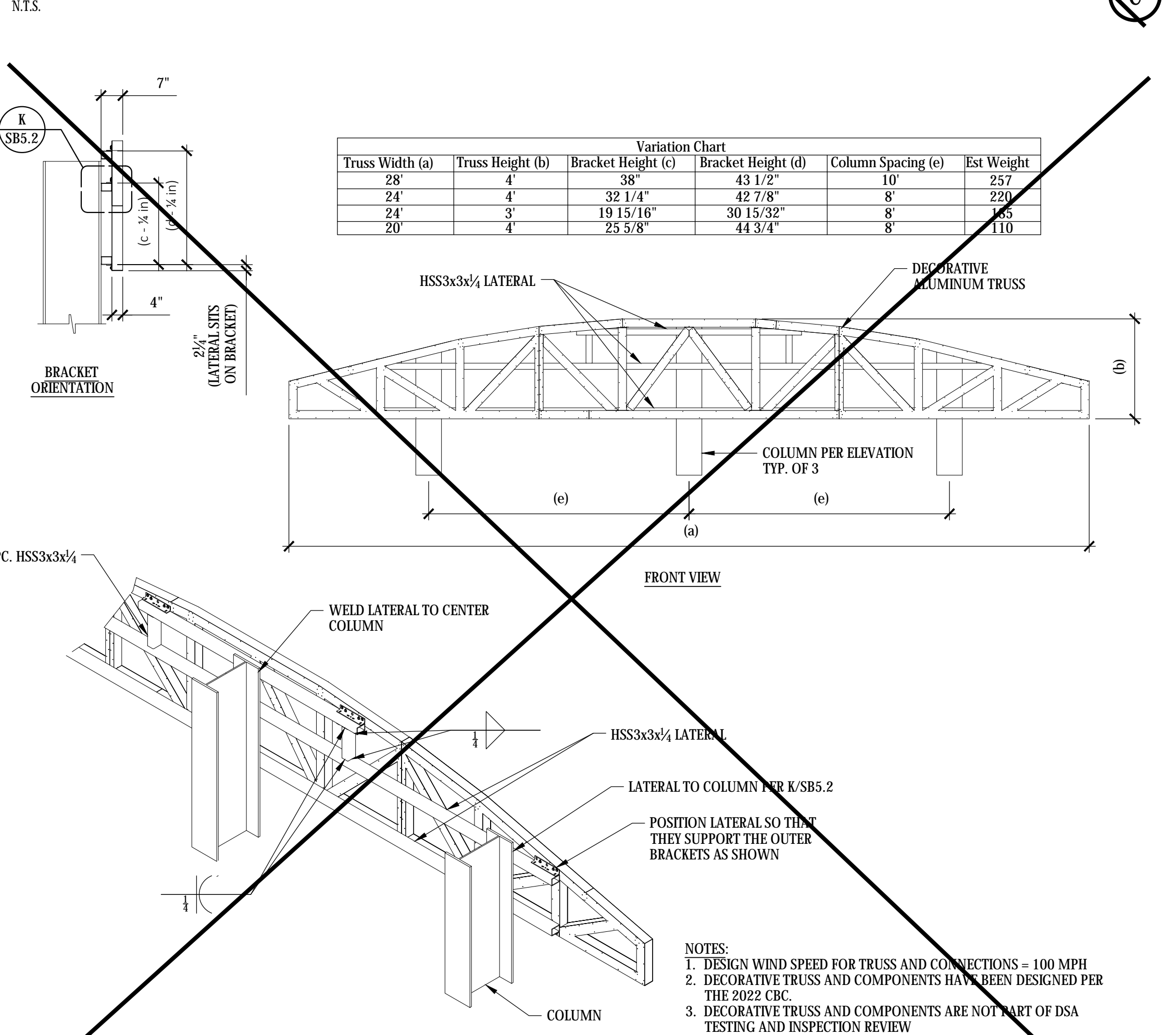
INSTALLATION OF ALUMINUM TRUSS ON FOUR COLUMNS



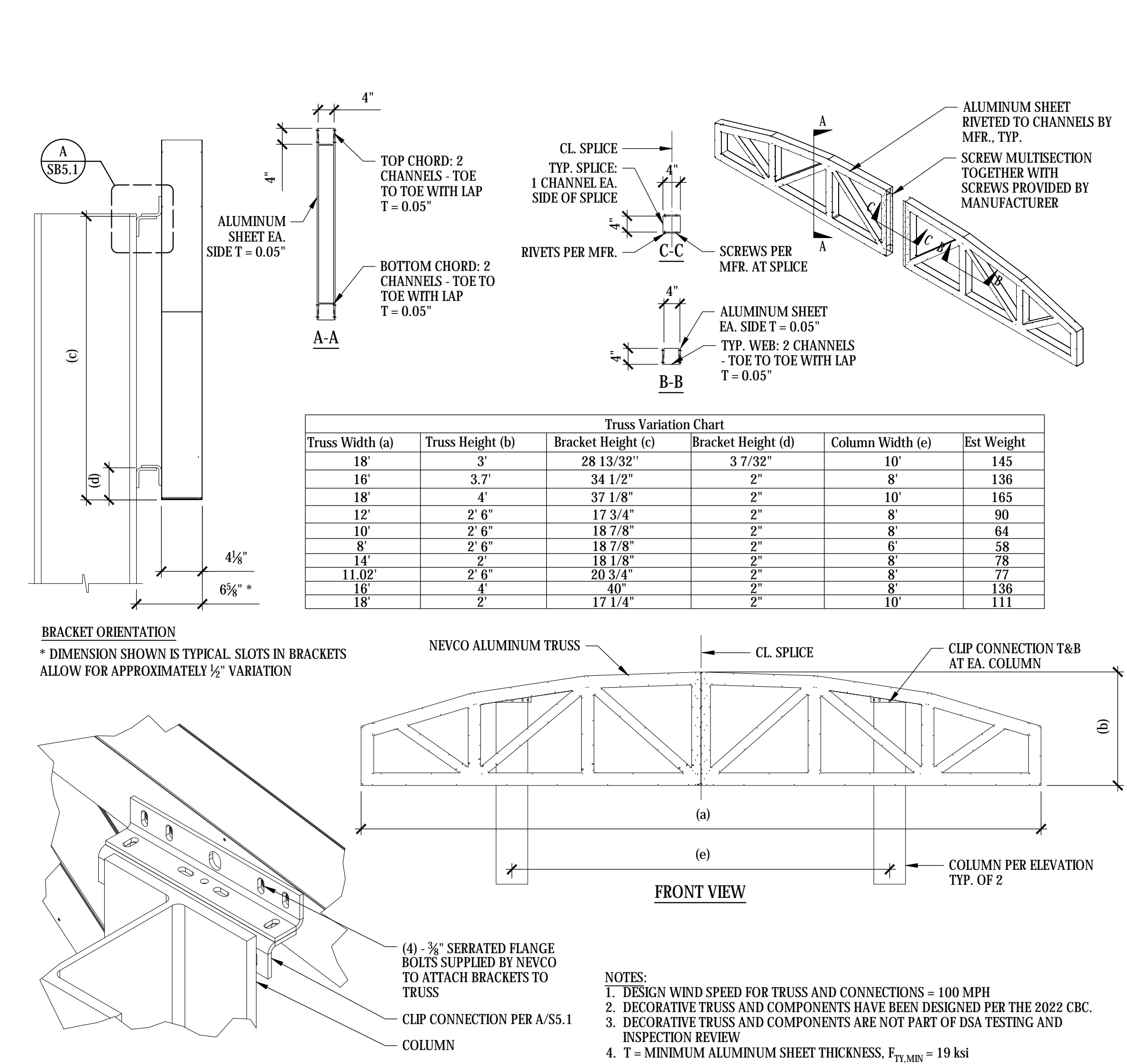
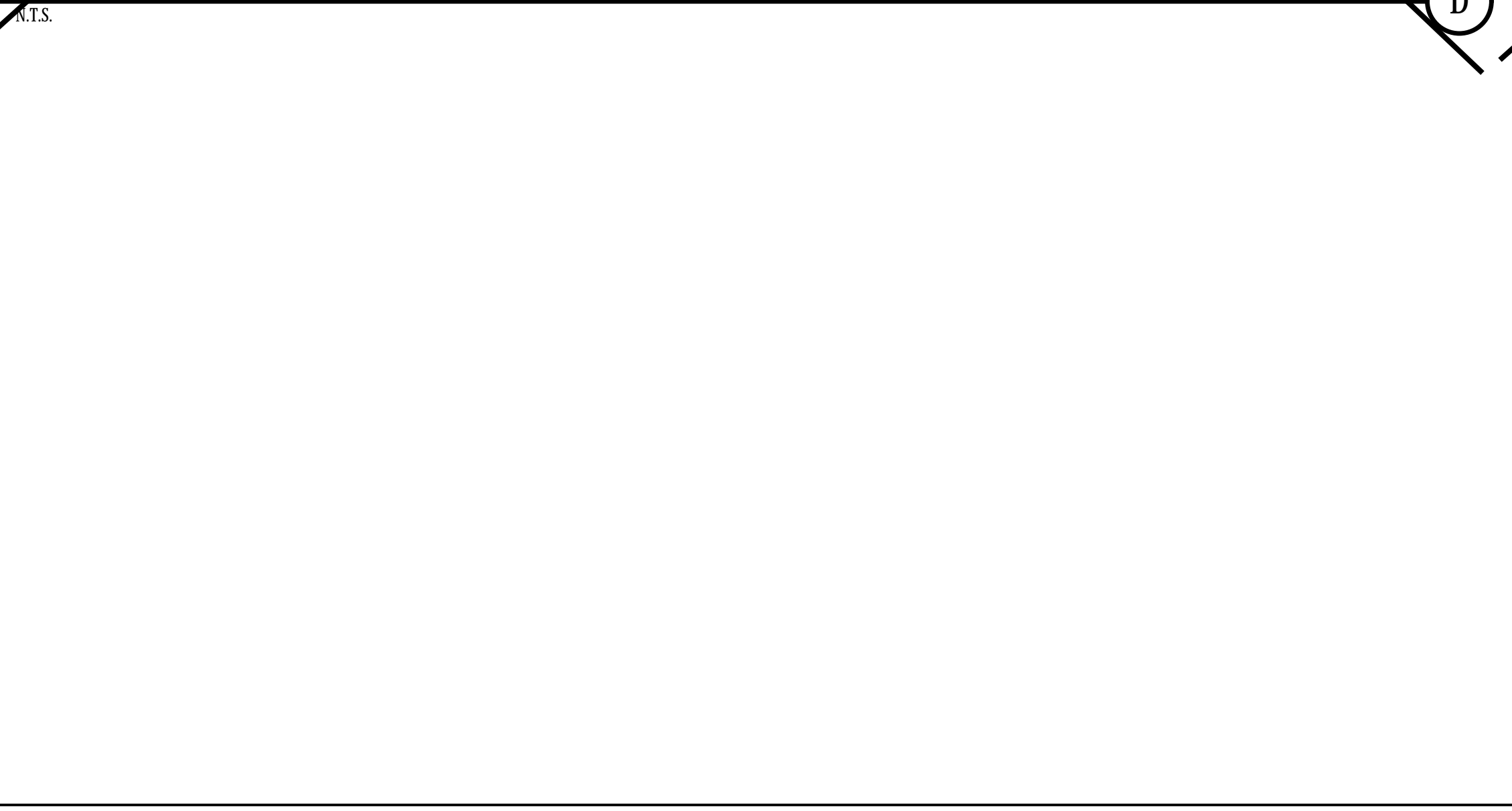
INSTALLATION OF ALUMINUM TRUSS ON FOUR COL. w/ LATERAL



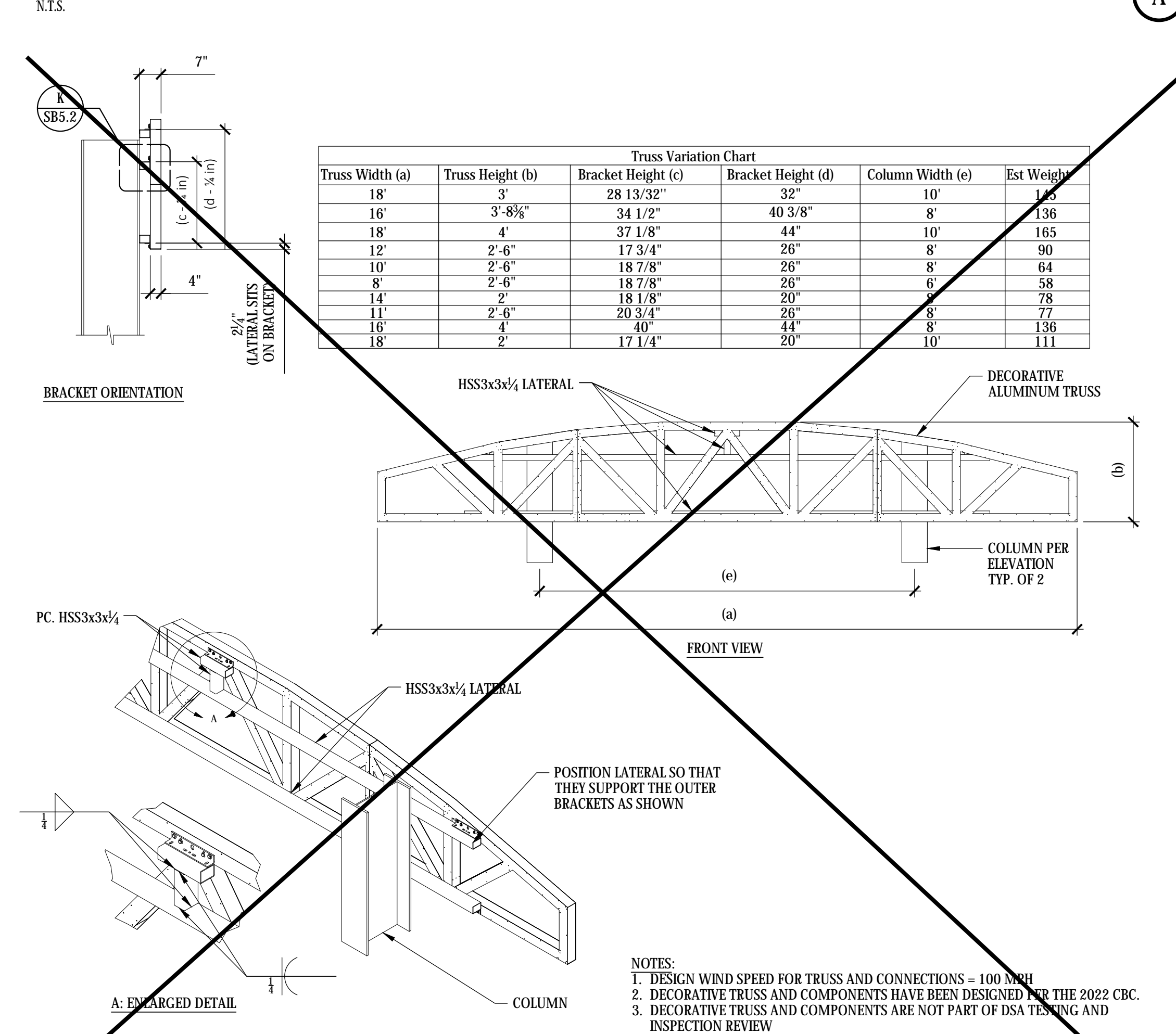
INSTALLATION OF ALUMINUM TRUSS ON THREE COLUMNS



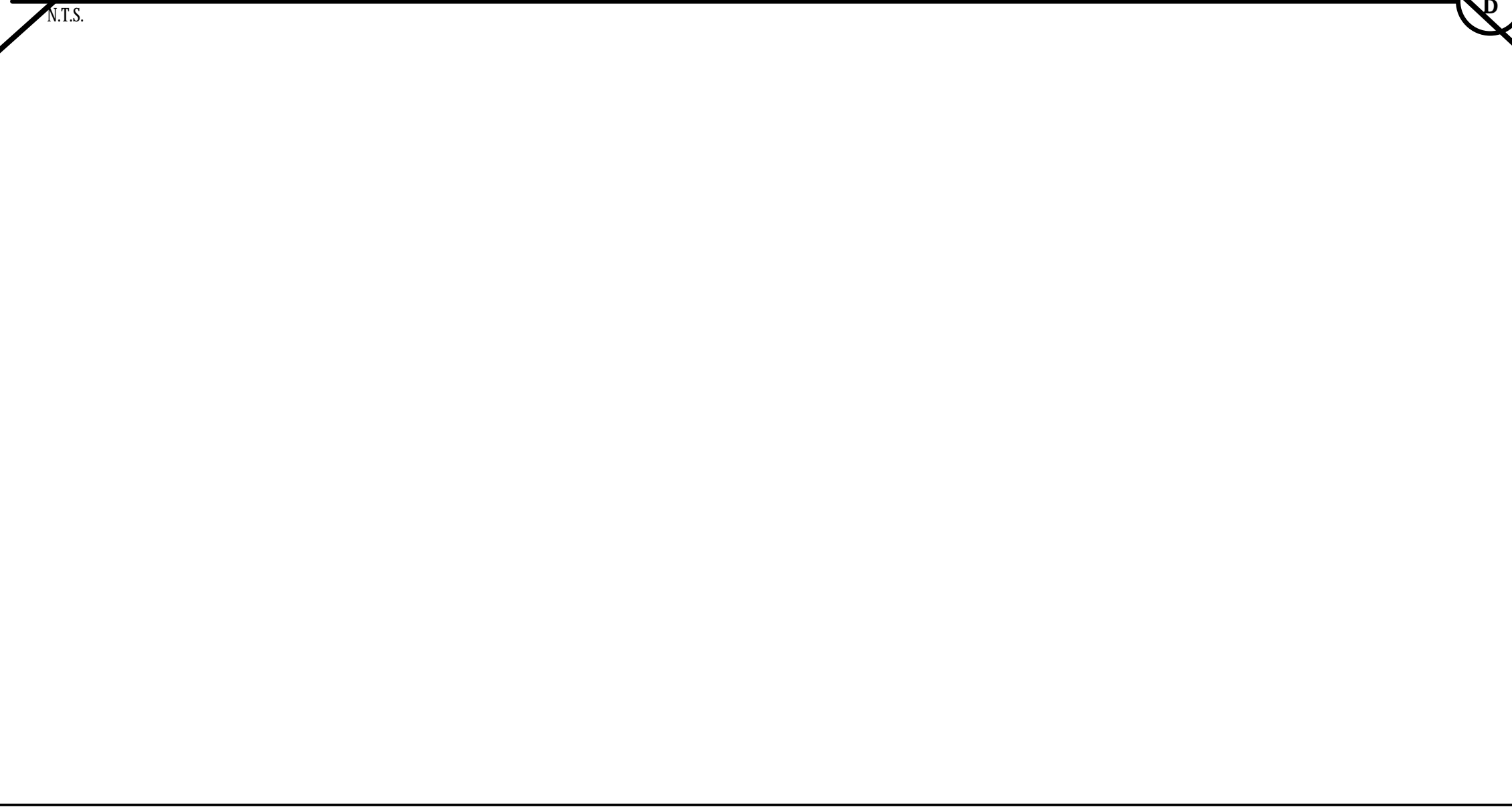
INSTALLATION OF ALUMINUM TRUSS ON THREE COL. w/ LATERAL



INSTALLATION OF ALUMINUM TRUSS ON TWO COLUMNS



INSTALLATION OF ALUMINUM TRUSS ON TWO COL. w/ LATERAL



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SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
No. 5405
STATE OF CALIFORNIA
08.09.2023

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No. 52386
EXPIRES 12/31/2024
STATE OF CALIFORNIA
09/20/2023

DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS

SHEET INFORMATION

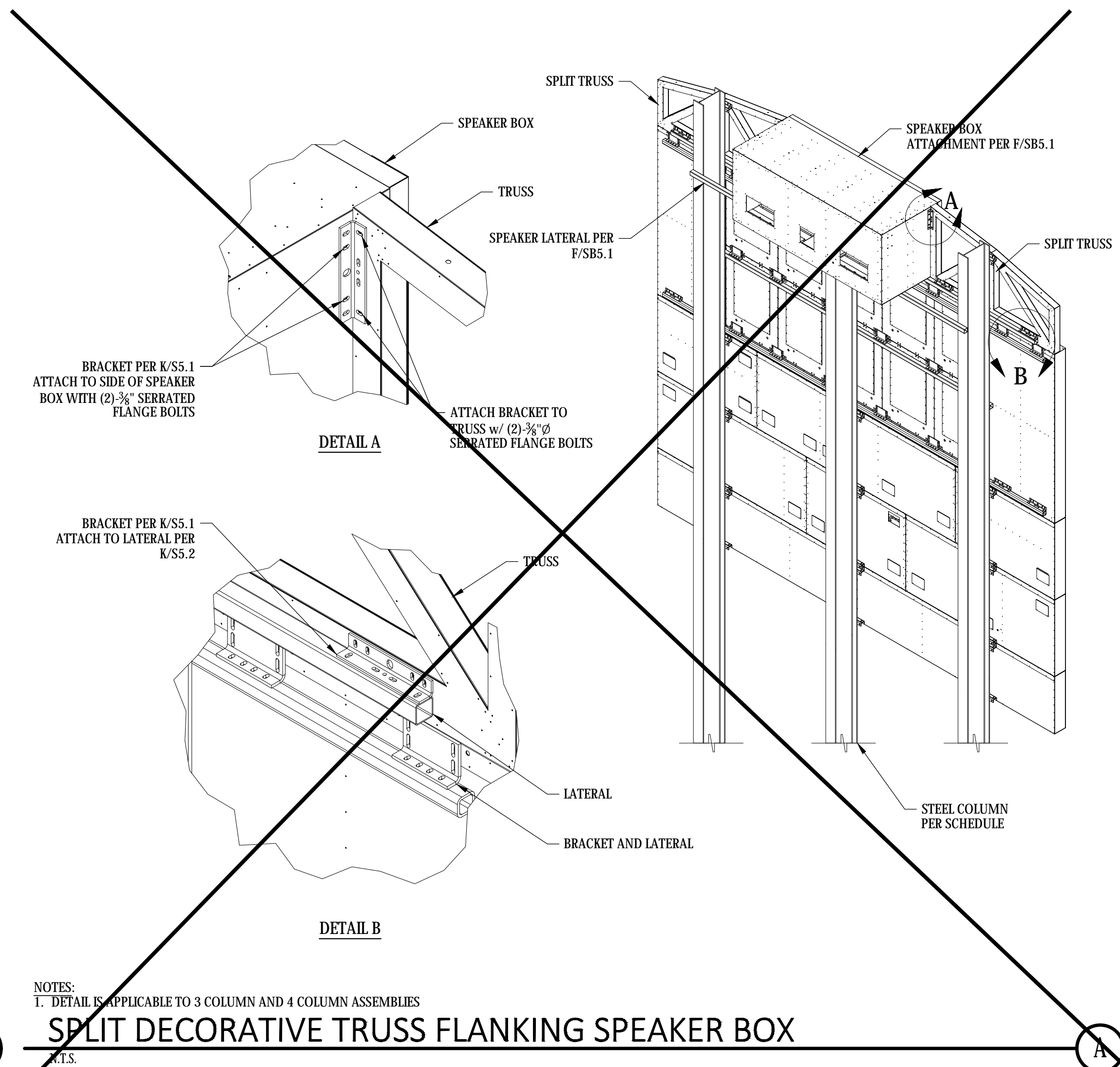
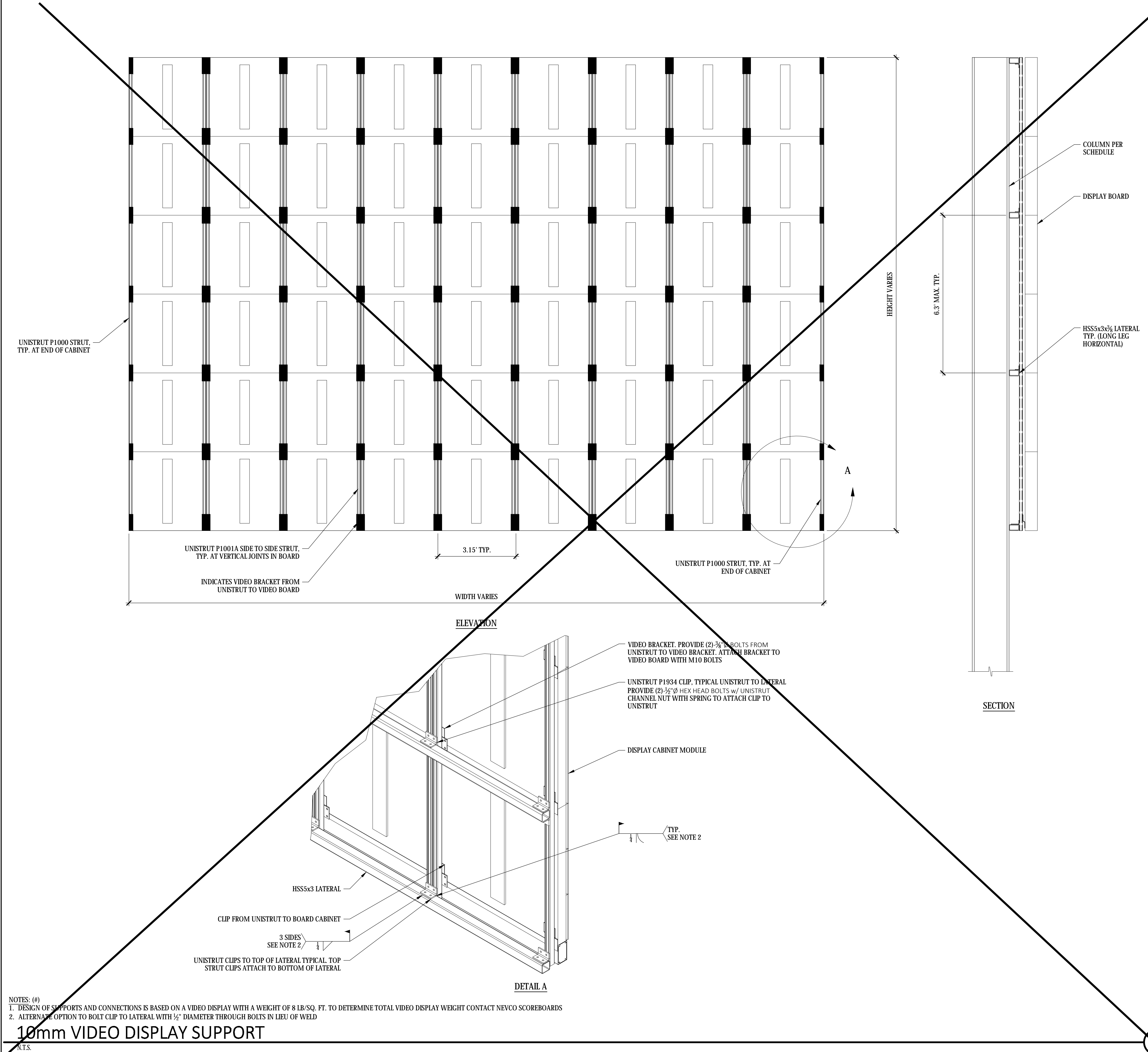
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SHEET: SB5.3



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SSG
structural engineers

REGISTERED PROFESSIONAL ENGINEER
MICHAEL E. RANNEY
No. 5485
STRUCTURAL
STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023
PC SEOR SEAL

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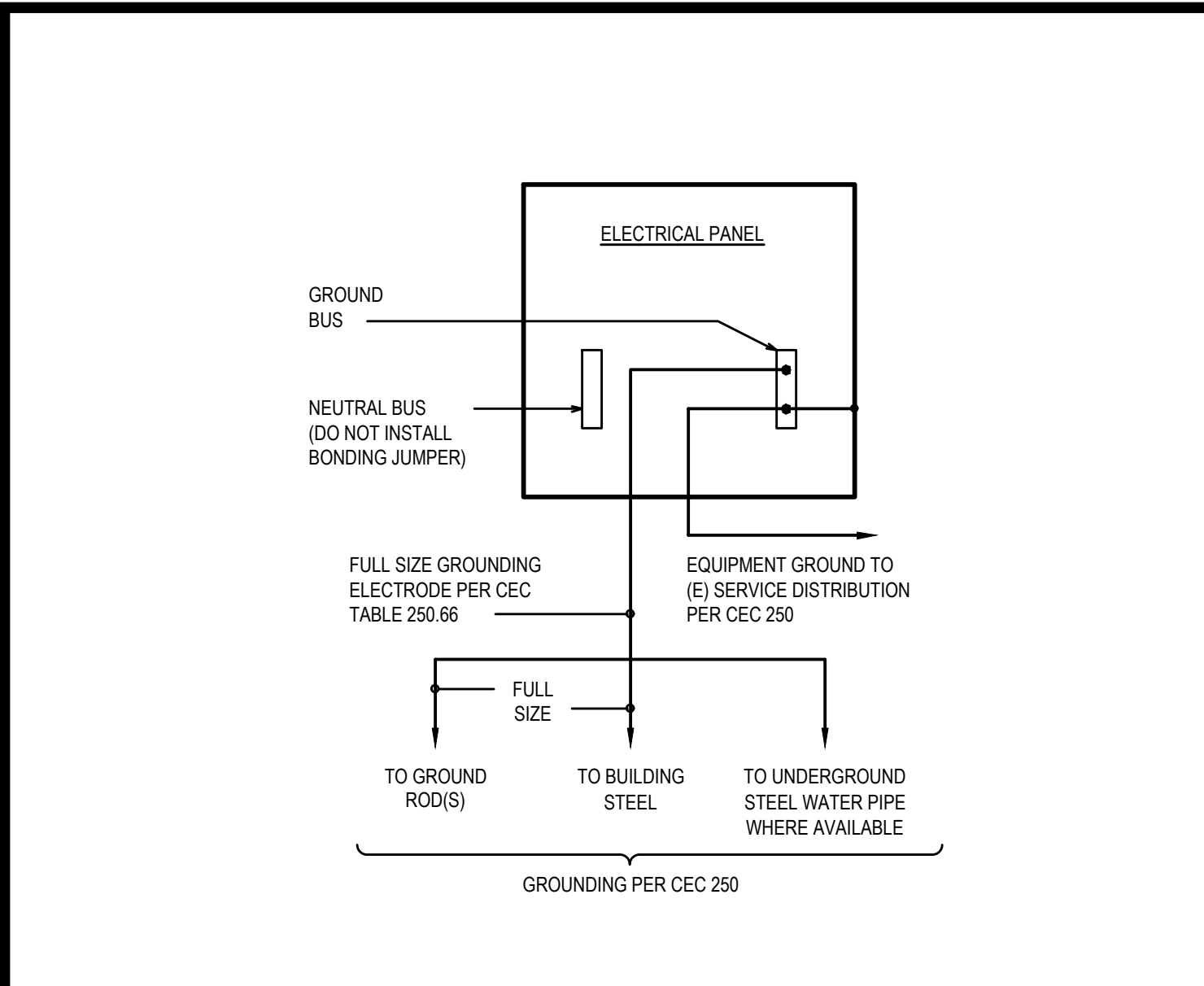
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STATE OF CALIFORNIA
DATE SIGNED: 08.09.2023

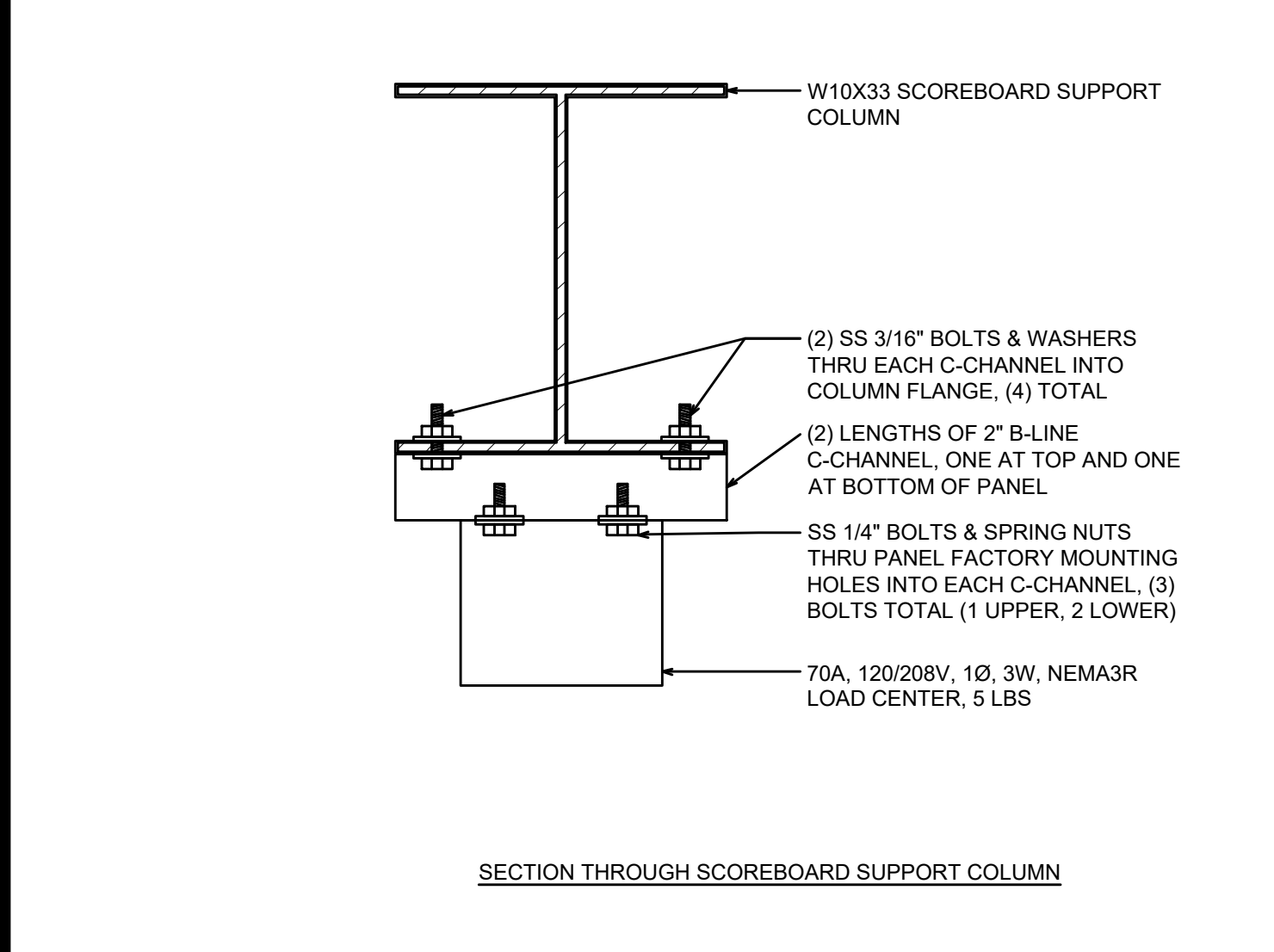
ALUMINUM TRUSS
ATTACHMENT DETAILS &
10mm VIDEO DISPLAY
SUPPORT

SHEET INFORMATION
DATE: 08.09.2023
DRAWN: JMK
CHECKED: MEP
SSG JOB #: S23109
SHEET: SB5.4

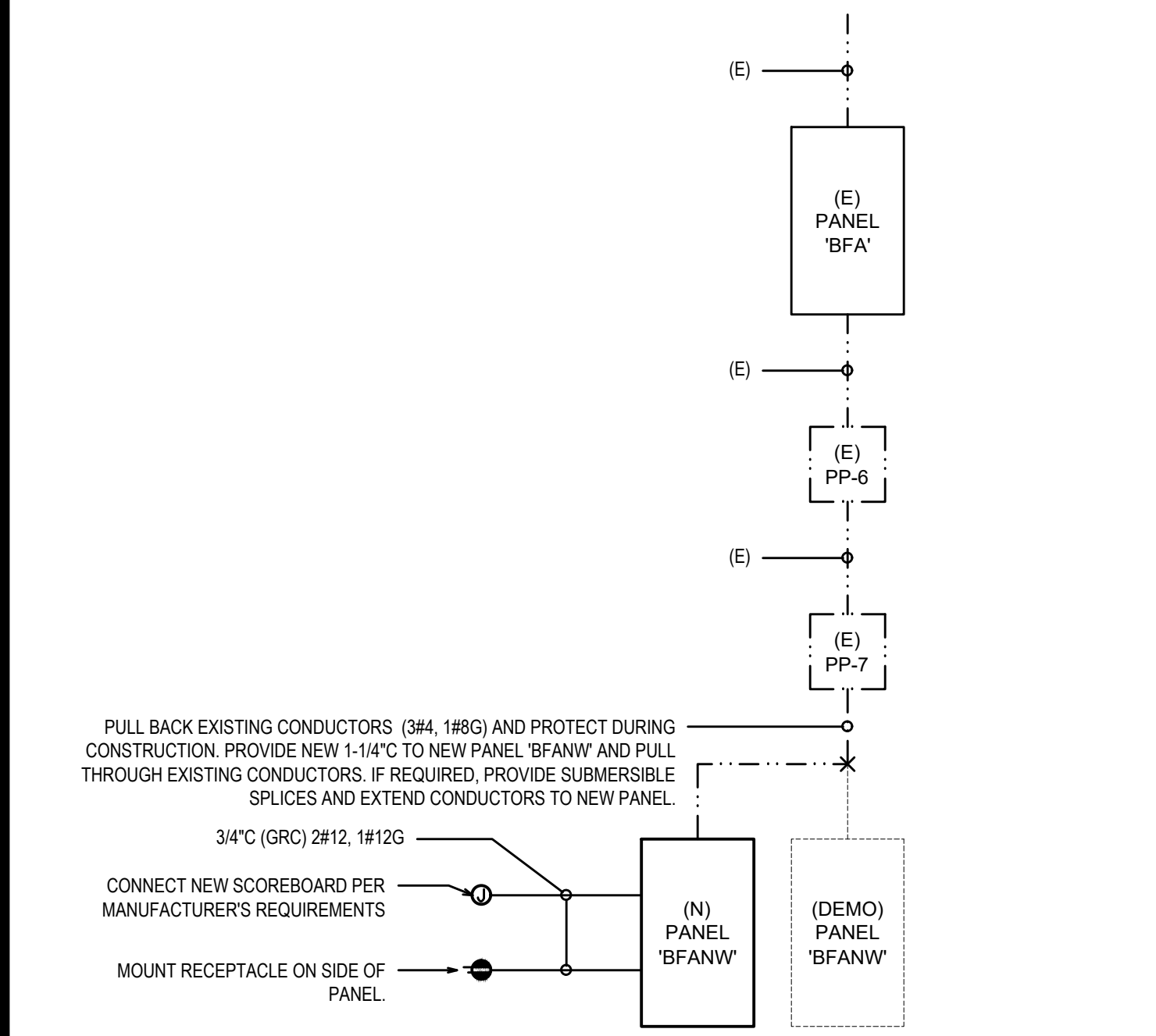
18 March 2024 9:23 AM Z:\Clients\BrooksRansom Associates\23134 - Modera USD Scoreboards (MHS, MSHS)\CAD Files\23134 - Modera South Scribble - Electrical - V.dwg
periodic may



3 Panel Grounding Detail
No Scale



2 Panel Mounting Detail
No Scale



1 Single Line Diagram
No Scale

ELECTRICAL EQUIPMENT ANCHORAGE NOTES

ALL ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. THE FOLLOWING COMPONENTS SHALL BE ANCHORED OR BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2022 CBC, SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16, CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING ELECTRICAL UTILITY SERVICE. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 110/220 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.

THE FOLLOWING ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:

- COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVING A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORTS THE COMPONENT.
- COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

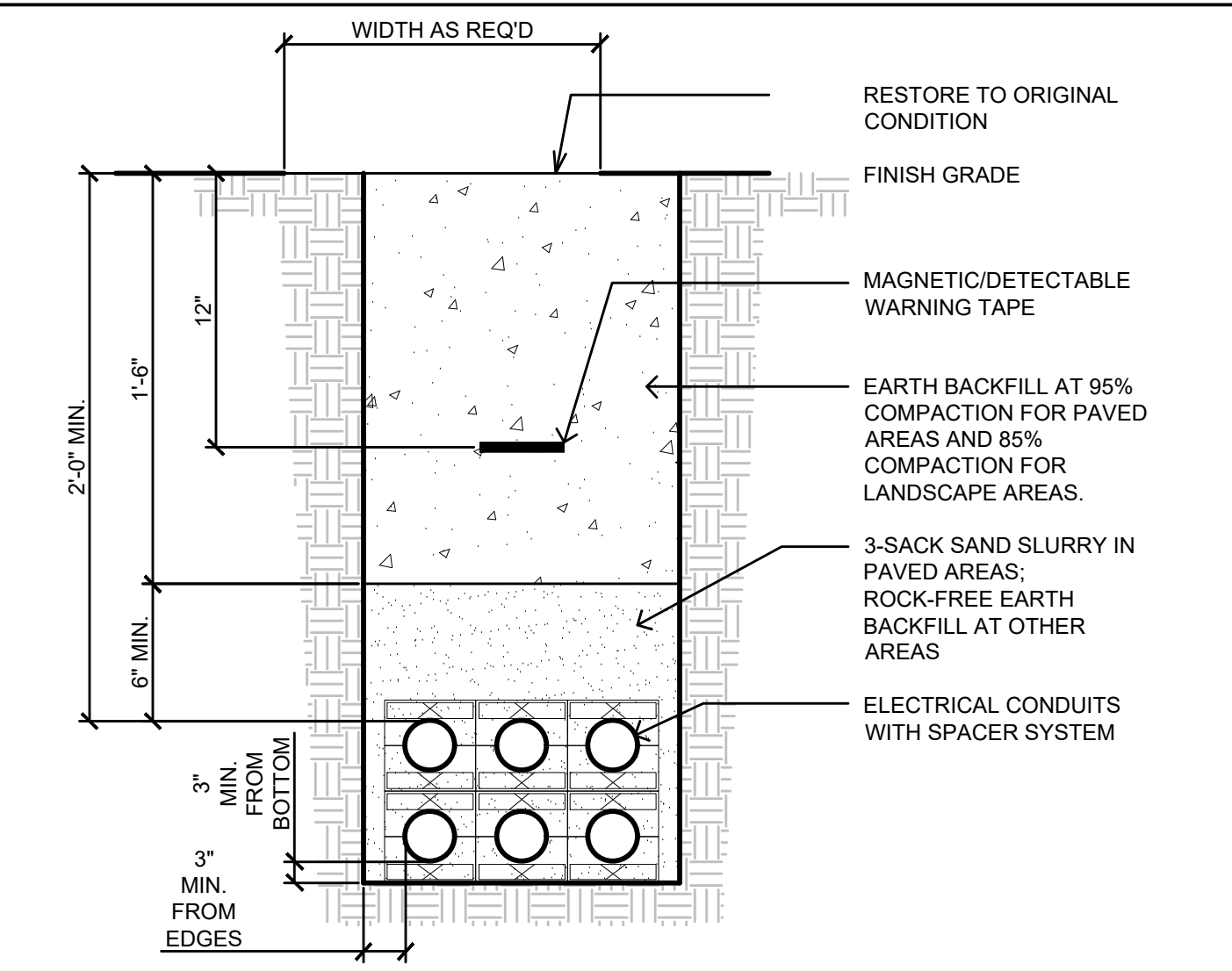
THE ANCHORAGE OF ALL ELECTRICAL COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

ELECTRICAL DISTRIBUTION BRACING NOTES

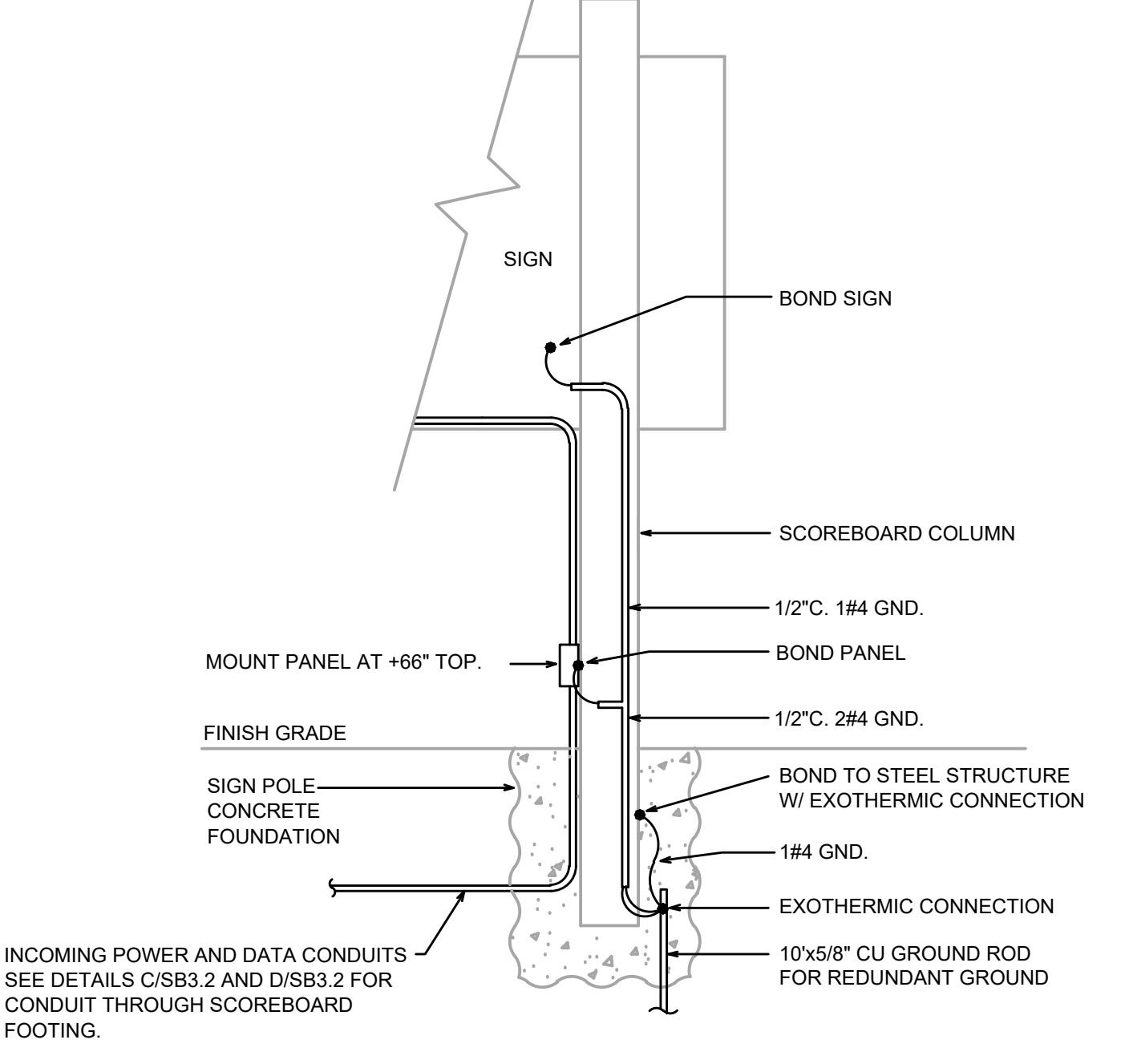
THE ELECTRICAL DISTRIBUTION SYSTEM SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16, SECTION 13.3 AS DEFINED IN ASCE 7-16, SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8, AND 2022 CBC, SECTIONS 1617A.1.24, 1617A.1.25, AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHPD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

THE ELECTRICAL DISTRIBUTION SYSTEM IS DETAILED ON THE APPROVED DRAWINGS WITH SPECIFIC NOTES AND DETAILS.



5 Trenching Detail
No Scale



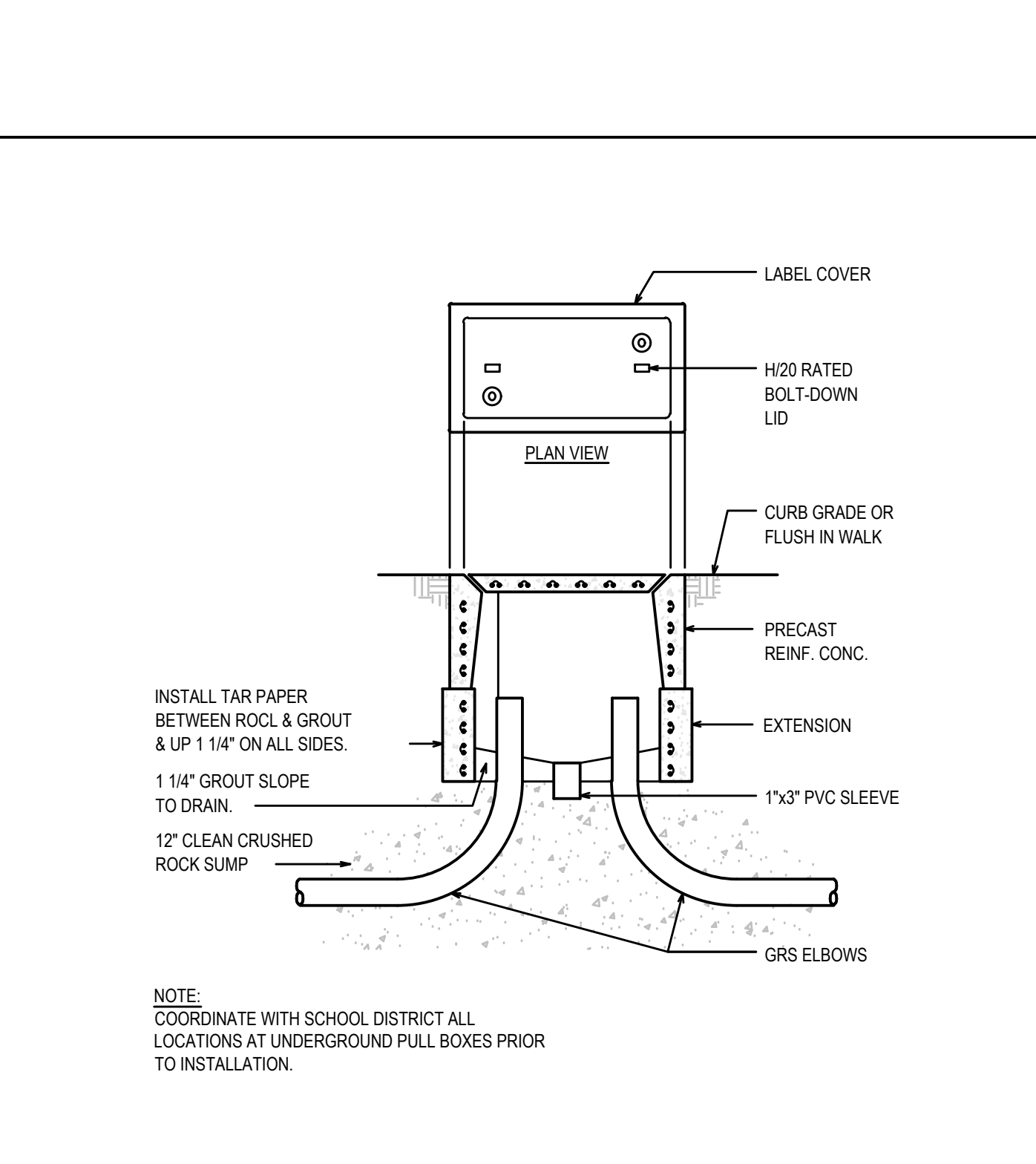
4 Scoreboard Grounding Detail
No Scale

ELECTRICAL GENERAL NOTES

- ALL WORK SHALL MEET THE LATEST ADOPTED ADDITIONS OF THE CALIFORNIA CODE OF REGULATIONS, TITLE 24 AND ALL OTHER APPLICABLE REGULATIONS, WHICH INCLUDE:

CALIFORNIA BUILDING CODE 2022
CALIFORNIA ELECTRICAL CODE 2022
NON RESIDENTIAL CEC ENERGY STANDARDS 2022

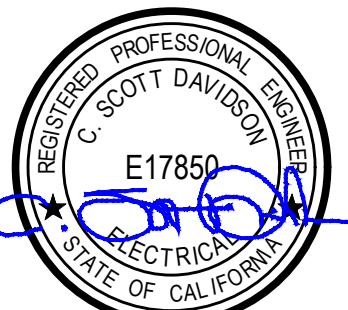
- NOTHING IN THE PLANS OR SPECIFICATIONS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES.
- IT IS THE INTENTION OF THESE PLANS AND SPECIFICATIONS TO COVER EVERYTHING REQUIRED TO PROVIDE FOR COMPLETE AND OPERATIVE SYSTEMS. THE CONTRACTOR IS TO FURNISH LABOR, MATERIAL, TRANSPORTATION, EQUIPMENT, MISCELLANEOUS SERVICES, ETC. REQUIRED TO ACCOMPLISH THIS RESULT. ANYTHING WHICH MAY BE REASONABLY CONSTRUED AS A NECESSARY PART OF THE INSTALLATION IS TO BE INCLUDED, WHETHER OR NOT SPECIFICALLY SHOWN OR MENTIONED.
- THE CONTRACTOR SHALL EXAMINE THE SITE AND EXISTING CONDITIONS AND MAKE ALLOWANCES IN THE BID FOR ANY CONDITIONS NOT SHOWN ON THE ELECTRICAL DOCUMENTS.
- THE PLANS AND SPECIFICATIONS ARE INTENDED TO BE USED AS CONSTRUCTION GUIDELINES AND ARE NOT THE TOTAL INSTRUMENT OF CONTRACT DOCUMENTS. IT IS NOT THE INTENTION OF ANY CONSTRUCTION PLANS TO DIVIDE WORK AMONG DIFFERENT TRADES. VERIFY THE SCOPE OF WORK WITH THE ARCHITECT AND THE GENERAL CONTRACTOR.
- ELECTRICAL ROUTING IS DIAGRAMMATIC ONLY. ACTUAL ROUTING & PHYSICAL CONDITIONS MAY VARY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE ACTUAL ROUTING, CONNECTIONS, & PROVISION OF ALL APPURTENANCES NECESSARY FOR A COMPLETE & OPERATING SYSTEM.
- ELECTRICAL EQUIPMENT SHALL HAVE AN APPROVED TESTING LABORATORY LABEL ATTACHED (UL, CSA ETC.) PER CEC 110.2.
- PROVIDE LABELING AND DIRECTORIES FOR ALL SWITCHBOARDS AND PANELBOARDS PER CEC 408.4.
- ELECTRICAL EQUIPMENT SHALL HAVE A SHORT CIRCUIT CURRENT RATING CAPABLE OF WITHSTANDING THE AVAILABLE SHORT CIRCUIT CURRENT PER CEC 110.9.
- PROVIDE MINIMUM 30" WIDE x 78" HIGH x 36" DEEP (42" DEEP) WORK CLEARANCES IN FRONT OF PANELS, SERVICE OR EQUIPMENT RATED AT 120/208V 30 4W [277/480V 30 4W] PER CEC 110.26.
- ELECTRICAL RECEPTACLE OUTLETS ON BRANCH CIRCUIT OF 30 AMPERES OR LESS AND COMMUNICATION SYSTEM RECEPTACLES SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), RECEPTACLES SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.
- CONTROLS OR SWITCHES INTENDED TO BE USED BY THE OCCUPANT OF A ROOM OR AREA TO CONTROL LIGHTING AND RECEPTACLE OUTLETS, APPLIANCES, OR COOLING, HEATING AND VENTILATING EQUIPMENT SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), SWITCHES AND CONTROLS SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.
- ALL WALL AND SURFACE MOUNTED FIXTURES PROTRUDING IN THE PATH OF TRAVEL (POT) OR COMMON PEDESTRIAN WAYS SHALL COMPLY WITH CBC 11B-307.2. OR SHALL BE MOUNTED LESS THAN 27" AFF OR GREATER THAN 80" AFF, OR SHALL BE PROVIDED WITH A BARRIER CONFORMING TO CBC 11B-307.4.
- WIRING FOR 120/208V AND 277/480V SYSTEMS SHALL BE MIN. #12 AWG THHN/THWN-2 COPPER.
- FEEDERS SIZE #4 AND LARGER SHALL BE MEGGER TESTED. TEST RESULTS SHALL BE SUBMITTED TO THE ENGINEER.
- COLORS/FINISHES/MATERIALS FOR ALL ELECTRICAL DEVICES, PLATES, LIGHT FIXTURES, ETC. SHALL BE CHOSEN BY THE ARCHITECT.
- EXISTING EQUIPMENT TO BE REMOVED AND/OR REPLACED SHALL BE DELIVERED TO THE DISTRICT MAINTENANCE DEPARTMENT OR DISPOSED OF, AT THE DISCRETION OF THE DISTRICT.
- CALL USA UNDERGROUND ALERT AND VERIFY WITH DISTRICT THE DESIRED ROUTING AND LOCATIONS OF UNDERGROUND CONDUITS AND STRUCTURES PRIOR TO TRENCHING.
- ALL UNDERGROUND CONDUITS SHALL HAVE MINIMUM 24" COVER. TRENCH PER DETAIL 5/E-1. INSTALL GALVANIZED RIGID STEEL RISERS & ELBOWS WHERE RISERS OCCUR. WRAP GRS BELOW GRADE OR PROVIDE PVC COATED GRS. EXPOSED CONDUIT SHALL BE GRS TO 8'-0", THEN ENT ABOVE AS APPROPRIATE. UNDER NO CIRCUMSTANCE SHALL PVC CONDUIT BE INSTALLED ABOVE GRADE.
- CONDUIT INSTALLED ABOVE GRADE SHALL BE MIN. 3/4" TRADE SIZE. CONDUIT BELOW GRADE SHALL BE MIN. 1" TRADE SIZE.
- INCLUDE ALL WORK REQUIRED TO INVESTIGATE, DEMOLISH, & RECONNECT EXISTING ITEMS.
- ALL LOW VOLTAGE EQUIPMENT SHALL BE DEENERGIZED PRIOR TO DEMO WORK. CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO LIVE EQUIPMENT.



6 Pull Box Detail
No Scale

ELECTRICAL SYMBOLS

SYMBOL	DESCRIPTION	NOTES
	POLE WITH POST TOP AREA LUMINAIRE	
	POLE WITH AREA LUMINAIRE	
	RECESSED TROFFER LIGHT FIXTURE	
	SURFACE CEILING LIGHT FIXTURE	
	RECESSED DOWN LIGHT	
	WALL LIGHT	
	EXIT SIGN, CEILING	
	EXIT SIGN, WALL	AT +80" AFF
	EMERGENCY LIGHT FIXTURE	PROVIDE UNSWITCHED HOT CONDUCTOR TO BATTERIES
	SWITCHBOARD	REFER TO POWER SINGLE LINE DIAG.
	POWER PANEL	REFER TO PANEL SCHEDULE
	TERMINAL CABINET	
	DISCONNECT SWITCH, FUSIBLE, WP	REFER TO MECH. PLANS & SPECS.
	COMBO STARTER/DISCONNECT SWITCH, WP	REFER TO MECH. PLANS & SPECS.
	JUNCTION BOX	4-11/16" SQUARE BOX & COVER PLATE MIN.
	MOTOR	REFER TO MECH. PLANS AND SPECS.
	DUPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	QUADPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	WP, GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	DATA OUTLET (RJ-45 CAT6) WITH 2 JACKS	HOMERUN CABLES TO IDF.
	(2) WAP DATA JACKS (RJ-45 CAT6A) MOUNTED IN ATTIC SPACE	HOMERUN CABLES TO IDF.
	WALL MOUNT VoIP OUTLET (RJ-45 CAT6)	HOMERUN CABLES TO IDF.
	WALL MOUNT DATA/COMM OUTLET	HOMERUN CABLES TO IDF.
	WIREMOLD 5400 SURFACE WIREWAY	
	CEILING MOUNT PA SPEAKER	MATCH EXISTING SYSTEM COMPONENTS
	WALL MOUNT PA SPEAKER IN SURFACE ENCLOSURE	MATCH EXISTING SYSTEM COMPONENTS
	AUDIO/VISUAL INPUT WITH HDMI/VGA/3.5MM AUDIO/USB JACKS AND WALL PLATE	INSTALL CABLES BETWEEN TEACHER STATION AND PROJECTOR. SEE DETAIL 6/E-2.
	ANALOG CLOCK, BATTERY POWERED	VERIFY COMPATIBILITY WITH EXISTING SYSTEM
	MAIN DISTRIBUTION FRAME (MDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	INTERMEDIATE DISTRIBUTION FRAME (IDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM TERMINAL BLOCK	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM HEAD END	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	TEL. SYSTEM TERMINAL BLOCK	WHERE EXISTING
	TEL. SYSTEM HEAD END	WHERE EXISTING
	FIBER OPTIC SPLICE LOCATION	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	CAT6 PATCH PANEL	WHERE EXISTING
	FIRE ALARM SLC & NAC TERMINAL BLOCKS	LOCATION FOR REFERENCE. SEE F.A. PLANS.
	FIRE ALARM EXPANDER PANEL	SEE F.A. PLANS
	FIRE ALARM CONTROL PANEL	SEE F.A. PLANS
	EMERGENCY VOICE/ALARM COMMUNICATION PANEL	SEE F.A. PLANS
	EXISTING WIRING TO REMAIN	
	WIRING BELOW GRADE	3/4" CONDUIT MIN.
	WIRING IN WALL OR CEILING	3/4" CONDUIT MIN.
	LOW VOLTAGE WIRING	
	CONDUIT RISER	3/4" CONDUIT MIN.
	FLEXIBLE CONDUIT	3/4" CONDUIT MIN.
	CONDUIT STUB AND CAP	3/4" CONDUIT MIN.
	HASH MARKS DENOTES QTY. OF CONDUCTORS	
	WIRE SIZE INDICATED, IF OTHER THAN #12 AWG	3/4" CONDUIT MIN.
	HOME RUN (TO PANEL 'A', CIRCUIT '15')	3/4" CONDUIT MIN.
	"EXISTING"	
	"UNLESS OTHERWISE NOTED"	
	"WEATHERPROOF" / NEMA 3R	
	"GROUND FAULT INTERRUPTER"	



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559.323.4928 fax

APPROVALS
APPLICATION #
02-122089

IDENTIFICATION STAMP
DIV. OF THE STATE ARCHITECT

APP: 02-122089 INC:
REVIEWED FOR

SS ☒ FLS ☒ ACS ☒

DATE: 4/3/2024

DATE: 12/12/2023

PARTIAL SITE PLAN
MADERA SOUTH HIGH SCHOOL
SOFTBALL SCOREBOARD
MADERA, CA 93637

REVISIONS	

BrooksRansom
ASSOCIATES
7415 N. PALM AVE. STE 100 | FRESNO, CA 93711
(559) 449-8444 OFFICE | (559) 449-8404 FAX

SHEET:
E-1
PROJECT: 23934

KEY NOTES

1. NOT USED.

2. EXISTING PANEL 'BFA' TO REMAIN. 100A, 120/208V, 3Ø, 4W, 10KAIC, NEMA 3R.

3. EXISTING PULL BOX TO REMAIN. VERIFY LOCATION IN FIELD.

4. EXISTING SCOREBOARD PANEL FEEDER TO REMAIN. 1-1/2"Ø, 3Ø4, 1#8G. EXTEND FEEDER IF REQUIRED. PROVIDE SUBMERSIBLE SPLICES.

5. EXISTING SCOREBOARD DATA/CONTROL CONDUIT. REPLACE EXISTING CONDUCTORS AS REQUIRED.

6. DEMO EXISTING PANEL 'BFANW' FROM STEEL SCOREBOARD SUPPORT COLUMN. DISCONNECT EXISTING FEEDER AND PULL BACK TO PULL BOX PP-7 AND PRESERVE FOR RECONNECTION TO NEW PANEL.

7. PROVIDE AND INSTALL NEW LOAD CENTER 'BFANW'. 70A, 4 CKT, 120/208V, 1Ø, 10KAIC, NEMA 3R. SQ.D #QO24L70RB (OR APPROVED EQUAL) PROVIDE (2) 20A/1P CIRCUIT BREAKERS. RECONNECT EXISTING CONDUCTORS. SEE DETAIL 1/E-1, 2/E-1, 3/E-1, 4/E-1, 5/E-1.

8. DISCONNECT EXISTING SCOREBOARD POWER AND DATA/CONTROL. REMOVE EXISTING CONDUCTORS AND EXPOSED CONDUIT BACK TO LAST PULL BOX.
9. POWER CONNECTION BY SCOREBOARD SUPPLIER. NEVCO MODEL #1609, 120V, 3.9A. 3/4"Ø, 2#12, 1#12G. SEE DETAIL 4/E-1.

10. PROVIDE RECEPTACLE +42", 20A, 120V, GFCI-PROTECTED. WEATHER-RESISTANT RECEPTACLE WITH WEATHERPROOF WHILE-IN-USE COVER. 3/4"Ø, 2#12, 1#12G. SEE SINGLE LINE DIAGRAM 1/E-1.

11. RELOCATE/REPLACE EXISTING PULL BOXES IF REQUIRED. SEE DETAIL 6/E-1. IF REPLACING BOXES, REPLACE WITH LIKE SIZE, H20-RATED.

APPROVALS

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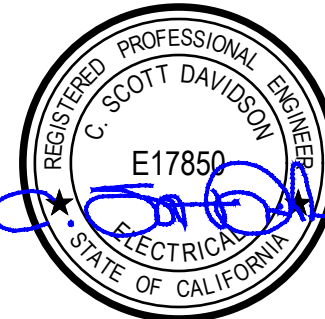
PARTIAL SITE PLAN
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1 PARTIAL ELECTRICAL SITE PLAN

SCALE: 1"= 50'-0"



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SHEET:

E-2

PROJECT: 23314